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#### PREFACE

The man who undertakes to build a book worth anything on estimating the cost of proposed buildings, is "up against" a pretty serious proposition. Not that such a book cannot be written that will be of great use to builders, but because of the ever shifting of prices of labor and materials, and the constant changing of methods and appliances. Figures that may be all right and correct for the work of to-day, may be entirely wrong and misleading to-morrow, and this is the main objection to works on estimating.

There are, however, certain rules and constants of measurements the estimator may employ when figuring up the cost of proposed buildings that may be relied upon as being correct, and in this work I have endeavored to show these rules and constants in as clear and understandable a manner as I know how, and I think my efforts have not been failures.

After all, the main factor to be employed in the make-up of an estimate is experienced judgment. No matter how much mathematics a man may be master of, if he has not experience in building matters and mature judgment to guide him, he can never become a reliable estimator. A good judgment may be born in a man, but experience can be gained only by a certain amount of labor and drudgery. As in other departments of science and art, there "is no royal road" to estimating, unless it be that which leads to guess work and financial disaster; therefore, let me press, at the outset, on the minds of all owners of this

#### PREFACE

work that an expert estimator can only become so by study and by a mastery of all the details that enter into the make-up of a building, added to a keen judgment and a comprehensive knowledge of the markets, labor, and materials employed on the proposed works.

Many an honest builder, good mechanic, and clever constructionist, has come to grief by taking contracts too low, because of his lack of knowledge in estimating, and thus not only does himself a great wrong, but he also disorganizes the whole building business in his neighborhood; for if he undertakes to do a certain job for a given price, his neighbors will expect to get similar work done for similar 'prices, and rival contractors then strain every nerve to get their estimates down to his level, and in doing so inferior materials are used, ''scamping'' is resorted to, and labor is crushed and cheapened to meet the conditions.

In the following work I have made an endeavor to place within the reach of every workman of experience an opportunity of qualifying himself to undertake the preparing of figures for work, so that he can make his tenders within the limits of reason—not absurdly high, or ridiculously low—so that only with a pen or pencil and this book he may be able to figure out and price a set of quantities in short order.

Great pains have been taken to collect such exact information as may be found useful in estimating, either in the office or on the building, with the object of forming what I believe will prove a valuable addition to building literature in other directions than that of simply being a price book.

During the last few years, materials of all kinds, raw or finished, have risen in price from 25 to 50 per cent, and labor has gone up in nearly the same proportion,

#### PREFACE

and the end is not yet, and artificial values have been created, and this continual fluctuation must always be considered when estimating, because no rules can be so devised as to be as elastic as prices and material men's quotations. This fact, or facts, only go to show that wherever prices are given in books of this sort, they should be accepted with similar to twithstanding this, however, the principles is minimating, as herein set forth, still hold good in so far as quantities and methods are concerned.

Collingwood, Ontario, May 1, 1904

FRED T. HODGSON.



# HODGSON'S ESTIMATOR AND CONTRACTOR'S GUIDE



#### INTRODUCTORY

Estimating the cost of a proposed building of any kind is not of a nature to attract the young workman, as it is a dull, dry, and methodical business and only the requirements of a sordid and money-making necessity compel the builder to wade through mazes of figures to attain the desired result.

If the writer had consulted his own pleasure and followed his inclinations he would not have written at all, or on a subject more congenial to his taste; but from long experience and observation and more or less practice, he has witnessed so much ignorance and inaptitude on the part of young men who have essayed to be builders and contractors that, with the advice of his publishers, he has undertaken to prepare this work on estimating, because it has been thought that a work of the kind may prove useful and of benefit to the young man who aspires to be a master builder or a contractor, and who may, if he chooses to go to the trouble, make himself fairly competent to arrive at the cost of any reasonable sized building. It may as well be understood at the outset, however, that there is no royal road by which eminence as an estimator can be attained. No matter what system or method may be adopted, correctness can only be reached through an avenue of labor and sound judgment. The best and most ingenious writers on the subject of estimating have never yet been able to discover or devise a method where the lost of a building may be "jumped

at" at first sight. The system of cubing is, perhaps, the easiest of all methods, but is not a system the experienced builder would care to follow altogether, unless a large margin of profits and contingencies are provided for.

While it will be impossible for me to so prepare this work as to be as entertaining as a novel, I will, to the best of my abilities, make it as easy to understand by the every-day workman as it possibly can be.

Estimating is the most difficult task the builder has to deal with, and too much care cannot be taken, even if the quantities are supplied, if a correct tender is wanted. Many who tender make up their prices in a haphazard manner, often depending on trade catalogues, price lists or newspaper quotations for data, using their judgment, whether experienced or not, and without a full or even a fair knowledge of the scientific mc.hods which underlie the proper formulating of a true estimate. Prices which enable successful contractors to calculate values for themselves are obtained by dissecting, taking asunder and examining the various elements that go to make them up, the complete result being shown in a final bill of quantities, labor and other costs.

It will be impossible to make this work a mere hand-book of builders' prices, as what may be the ruling price of labor or material to-day may be very much different to-morrow, as in these days of continual change there can be no such thing as "constants" in prices. I can give quantities, however, and describe the proper methods of obtaining them, and can convey to the student the principles upon which correct estimating is based, and offer here and there the prices of labor as *now current* in the larger cities, not to be fol-

lowed, but simply to give an idea of the cost of various kinds of work when no other data is available.

No man can be a successful contractor who does not attend strictly to his bookkeeping, so that he can tell in a moment, by reference to his books, the exact amount of profit or loss on the various jobs of work he has completed. This is important, inast such as the mistakes in estimating may be traced to their source, and thus be avoided when similar jobs ar Leing figured on; and much trouble and disappointment may be avoided by having the accounts on every job itemized and kept in proper order. I will have more to say on this subject later on.

All estimates should be retained, properly labeled, and put in some place where they can be found when required, whether the work for which they vere prepared is secured or not, for they will often prove of great service for future reference; and the estimator should make a note of each particular on which he may have priced too high or too low, if his tender is not accepted. If the work is secured, the cost of each particular item in the building should be compared with the estimated price, and a note should be made on the margin of the original estimate showing the discrepancy, if any, between estimated and actual price. A correct account of all labor, how employed, should also be kept, so that the contractor may know om actual facts exactly what a piece of work costs, he number of days or hours it required to perform such and such work, also amount and cost of materials on the same work; then, in preparing other estimates, he will have something tangible to base his figures on. It is better to estimate on days or hours for time, and on quantities for materials, because of the continual

fluctuations in price of labor and mate ials of all kinds. If it takes 2½ days, of 9 hours each, to execute a piece of work, the figuring on this is quite simple, for all we have to do is to multiply the number of hours by the price per hour for labor; suppose this to be 30 cents an hour, then we have  $22\frac{1}{2} \times 30$  675. That is, in  $2\frac{1}{2}$ days, at 9 hours per day, we get 221 hours at 30 cents perhour, which will make 675 cents, or 63 do'lars. Quantities may be figured in a similar manner. If the work requires 150 feet of material, then charge that at current rates, whatever these may be; then add cost of labor and material together, and you have the bare cost of the work. To these, of course, must be added cartage, profit and any other materials that have been employed on the work, such as nails, screws, glue, paint, or anything else. By following this course, a record of all work done and estimated for will always be at hand, and it is surprising how much the labor of estimating may be reduced by a strict adherence to this system, as a comparison with work done and work to do may be made in a few minutes, and the difference in prices of labor then and now adjusted so that no loss will occur to the contractor.

The variations in tenders for the same work are often surpr' ing. I have seen estimates, particularly in carpenter's and joiner's work, run up to as much as 50 per cent above the tenders of competing contractors, yet the lowest bidder made money. Competent estimators never make such wide errors as this, though often they do not keep close to the wind; and while mistakes will continue to be made, even by the very best estimators, by omissions, "doubling up," and using wrong dimensions, the mistakes may be narrowed down to a very small area if system, care and

judgment be exercised when the estimates are made. will be the object of this little volume to narrow this area of error to the smallest possible limits, and to show the estimator how to avoid grievous errors and make his estimates more satisfactory and reliable.

#### CATALOGUES AND PRICE LISTS

No builder's office can be well equipped except it contains the latest catalogues and price lists avai'able, for on these the estimator must, to some extent, be dependent in his figuring on the cost of most of the material that goes in the work. Bricks, stone, lime, cement, lumber, hardware, and factory-made stuff may have their prices approximated from these publications, but the shrewd estimator, while making use of these aids, does not rely upon them for serious pricing. They help considerably, as they contain a lot of condensed information regarding prices and building; but they are not always to be depended upon, as they are not always compiled in a scientific way. For example, some of the prices include trade discount, some do not, while others are merely the ordinary list prices of merchants' catalogues. The discount in itself largely varies, and there are two, and often more, discounts-a trade discount and a cash discount-and other mysterious discounts, such as 30% and 5%, which means 30 per cent off and 5 per cent off the balance; and again, the percentages are not uniform; one merchant may have one discount, another another, so in all cases it is best to get prices and discounts direct from the merchant wherever possible. The diversities in discounts are innumerable, and it is the estimator's duty to g : definite information as to

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prices and discounts as prevailing in the locality where the work is to be done.

Builders' prices are broadly made up of two divisions, labor and material, to which may be added a third, namely, profit. The cost of labor and material vary from time to time, and from place to place, and do not fluctuate similarly. Sometimes labor may be high and materials low in price, and at other imes materials may be high and labor low, so that no given rules can be formed to meet these conditions and be constant, and this fact rules price-books out of the race of accuracy for any length of time. Such things as closeness or slackness of supervision, misunderstandings as to quality of workmanship or materials, worrying by the architect, delay in furnishing detail drawings, differences in locality and site, frost and bad weather, sudden and unexpected rises and falls in the market, etc., will all help to alter the conditions of profit or loss for the contractor, and the extent of which is almost impossible to measure.

When, however, the contractor has worked out a series of prices for himself, to suit both time and locality, he must be on the alert for parallel cases to avoid the great labor involved in making calculations afresh every time a new estimate is required. In fact, he should carefully prepare a sort of price-book for himself, suitable to the conditions, and so arranged that it can be revised from time to time. Thus a consistency in pricing would result, which is of considerable importance.

As already stated, the builder will be confronted with several grades of discounts, and among them will be a *cash* discount. This may be more or less or anywhere within the limits of from 2 to 5 per cent, and it

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nted will anyd it should be the aim of the contractor to get the best discounts to be had, providing the materials or goods are up to the standard demanded by the specifications. Sometimes it may happen that on special goods or some particular make of hardware or other items, no discounts are allowed. This, however, can only happen when a dealer has the sole control of these special goods, or when there is a scarcity of them in the market, or when a sudden demand for them arises. These conditions, however, seldom or never occur, so they may hardly be considered. In the practice of a shrewd contractor, the question of discounts enters largely into the make-up of an estimate, particularly where close competition is likely to be met with.

The question of profit is one that must be well considered when estimating; 10 per cent is the least amount a builder can accept, exclusive of established charges, and this should be added to each individual price, and no provision should, under ordinary conditions, be made for any trade discounts, as these are expected to swell the profits. Some estimators when pricing bills of quantities prefer to add a lump sum as profit at the end of a bill. This, however, is not a good thing to do, as it gives no correct method of knowing what the profits are.

For wood or materials on small jobs, where both are limited, the profit should be higher, as the total expenditure in such a case is much more in proportion; therefore the percentage of profit should never be less than 15 per cent on work costing up to \$2,500, but above this amount a smaller percentage would perhaps be sufficient.

The large contractor, who may perhaps own his own brickyard, quarry or factory, with extensive premises

and rapid-working labor appliances of all kinds, can naturally turn out work cheaper and more expeditiously and at a greater profit to himself than the small contractor who possesses none of these appliances and aids. Often the latter, in order to save himself from loss, is obliged to scamp the work and use inferior materials, which he can frequently "get in" without the architect being able to detect it; he is often obliged to do this in order to keep himself afloat. My advice in cases of this kind is, that the lower contractor should confine himself to certain prices-that will pay himand if he cannot win the work for these prices he had better leave the work for the larger contractors, and thereby preserve his reputation and his money. The small contractor can always find plenty of work to do if he but gets a good name for doing his work well and according to specification.

Where there are dozens of doors made from one pattern, as many window frames and sashes, and hundreds of feet of mouldings in wood or stone of one shape and size, they can be rattled out by machinery in short or  $\therefore$  and at a comparatively low cost, and this is an item the estimator must consider, as it will aid materially in keeping down the total amount of tender; in any case, however, experience and judgment in such matters are required before a definite amount can be decided upon.

With reference to terms of payment, it is always better that the contractor gets his money often, as it enables him to push his work with greater vigor, and gives him a chance of making the best cash discounts when purchasing materials, and, on these several accounts, he will be able to make a lower bid for the work than otherwise. The reserve to be deducted

from each payment should never exceed 25 per cent, which is considered ample to cover any liens of workmen or material men and safeguard the interests of the There are certain fixed charges or provisions owner. in contracting that must not be overlooked. These consist of salaries, depreciation of plant, tools, machinery, rent of premises, lights, water, and interest in capital invested, of which the new work must pay its proportional share, and these charges should be kept separate and added to the estimate along with the percentage of profit. Such charges are commonly placed at 6 per cent interest on capital invested, and 3 per cent for depreciation of plant, etc. Sometimes they are classed in two categories: 6 per cent on work done on the building, and 8 per cent on work done in the contractor's factory or shops. These percentages, however, are somewhat arbitrary, and should be the result rather of experience and good judgment than any fixed rules, and the foregoing remarks are offered rather as reminders that some allowances must be made for each item when estimating, otherwise they might be overlooked.

The question of transportation is one also that enters largely into the cost of work. If the works are situated nearby the office and establishment of the contractor, the question will not be so formidable as when the work is some distance away, as the greater part of the material will very likely be near the ground and may only require handling and teaming once; but where the work is at a distance, the expense of getting the material on the ground will necessarily be much greater. When conditions will admit of it, it is always better and cheaper to have material shipped by boat than by rail, or long hauls by team, and the estimator

should make himself familiar with all the ways of communication to the spot where the building or buildings are to be erected, and should get a schedule of rates from all the lines running to that point. A good idea is to get a map of the district which shows all the railway and water communication; then the shortest and best routes can be chosen, providing the rates are satisfactory. As I stated before, it is much better, when it can be done, to ship by water than by land, as because of the absence of vibration, fine work will be less likely to be injured or scratched during transit, and, as a rule, rates are always lower by water than by land. The average rate for the shipment of goods in this country is about 13 cents per mile for short hauls, and something less for long hauls.\* Rates, however, vary with the different roads and at different times, the highest rates being in winter, in the north, when the waterways are frozen up. Classification, also, has something to do with regulating rates. All goods should be insured or shipped at the carrier's risk, then losses or damages will be covered. If goods have to be packed, or put up in crates or boxes, at least 15 per cent should be allowed for this work and material, and should be charged on the special goods boxed or crated only, but added to the estimates.

Goods sent at carrier's risk that get damaged, should be returned by the same carriers free of cost, and when repaired or renewed should be delivered at the point where first destined, at the cost only of the first shipment of the same goods. That is, the shipper should pay for one shipment only.

Where a quantity of goods of a similar kind is required, a special quotation should be given the con-

\*Per ton.

tractor by the dealer, and this should never be overlooked, for it is not likely that it will be given if not applied for.

Trade discounts, as a rule, are not publicly stated in trade cataiogues or circulars; they can be obtained only on private application. Their amounts greatly depend on the quantity of goods ordered, and the larger the order the larger the percentage given.

The foregoing remarks are offered as a sort of preliminary and should be well considered by the intending estimator, as they contain much that will tend to smooth the way towards accuracy in making up a tender, and, if followed attentively, will enable the estimator, along with the rules that follow, to get at a result that will be nearly correct and satisfactory.

#### SYSTEM IN ESTIMATING

The estimator should follow some well-defined system in his work, in order the he may know he has not overlooked anything, for one of the dangers is that of omission. To overlook the roof—as I have known one instance of the kind—the floors, the doors, or anything else, is a serious matter, and in order to prevent this as much as possible I have prepared a list of items which I give further on, and which may be called a "Tickler" or a "Reminder" of what will be required to consider when making an estimate of a building complete.

When erecting a structure of any kind, work should commence at the earth, so the first thing estimated, following the same rule, should be the excavations for cellar, drainage, foundations, trenches, and other similar work, then the preparing and the laying of the

foundations, whether of stone, concrete, or brick; and the same order should be followed throughout the whole building, until the whole is fully completed, from turning the first shovelful of earth until the last piece of finished work is put in place.

The following items will remind the estimator of the things to be figured on as he works his way upwards:

Inspection of site Examination of soil Note if gravel, soil, or sand Figure accordingly Get number of cubic yards The distance to be removed Where to be deposited Pumping water How drained Sewerage What depth of drains Depth of cellar Depth of foundation walls Width of footings Rock blasting Shoring banks Piling for foundations Sheet piling Excavations for piers Cesspool Cistern Trenches Cuttings for water pipes Grading Leveling cellar floor W. C. for workmen Removing fences Grubbing out tree stimps Removing surplus soil Removing debris Sodding Carriageways

**Footpaths** Driveways to rear Tamping earth Concreting foundation Openings for drain pipes Laying drain pipes Area of all tiles Weeping tiles Elbows and bends Traps of all kinds Intake water pipes Waste pipes Footings Cellar walls Furnace room Walls laid in cement Walls laid in lime mortar Walls built up of eonerete Stone walls, field stone Stone walls, quarried stone Stone walls, dimension stone Brick walls for cellar Amount of stone Amount of bricks Amount of concrete Cellar steps Cellar windows Cellar doors Cellar partitions Cellar coping stones Cellar sills and lintels Bond stones

Cellar water closet Water taps, etc. Concrete and cement floor Plank floor Earth floor tamped Wine cellar Vegetable cellar Coal storage bins **Coal chute** Ashes receiver Cellar stairs Preserve closet Shelving Plastering walls and ceilings Damp courses in walls Double sashes in windows Doors, what kind Fireplace and chimney Laundry tubs Hot and cold water supply Furnace and attachments Furnace, hot water Furnace, steam water Furnace, hot air Gas jets, how many Electric lights, how many Laundry table Clothes drying device Mangle Chimney piece Stove rings Registers Cellar finish Wardrobe hooks and pins Cupboards and drawers Tool room Wash bowl and stand Kind of hardware Ground floor Number of rooms Number of doors

Number of windows Style of doors Style of windows Sizes of doors and windows Thickness of doors and windows Kind of glass How windows are hung Hardwood or pine finish Outside walls, stone, brick or wood Thickness of walls If stone, rock face Tooled, rubbed Cross tooth chiseled Crandalled Brick wall Thickness of brick walls Common bricks Pressed bricks First, second or third quality Mixed, brick and stone Walls ornamented Walls left plain Window finish Urinals Slate slabs Exterior window finish Interior window finish Exterior door finish Interior door finish Betting courses Sailing courses Laid in cement or mortar Front steps, stone Front steps, eement or wood Hall eutrance Double floor, pine Hardwood floor Parquet floor in some rooms Tile floors Dimensions of joists

Thickness of floors Height of ceilings Stairs, straight Stairs, winding Stairs, platform Pine or hardwood Kind of hardwood Styles of newels and balusters Plain finish in rooms Ornamental Juish in rooms Fret and grill work Arches, plain or otherwise Styles of plastering Stucco cornices Styles of cornices Sliding doors Fireplaces How many Mantelpieces Mantelpieces, plain or ornamental How finished Other wood finish Pillars, columns or brackets Base and plinth Style of trimmings Style of hardware Cost of hardware Grates and tiles Mirrors Gas lighting Jcts and gasoliers Electric lighting Electroliers and brackets Piping for gas Wiring for electric lights Fitting clothes closets Fitting up den Fitting up closets Fitting up cellar stairs Fitting up dining room

Fitting up other rooms Kitchen finish Tubs, sinks, dresser Cupboards, china closet Butler's pantry General pantry Range Steam cooker Chinneys Ventilation Painting Varnishing Wainscot Panelings Washstands Marble facings for walls Double windows Sashes, weights and cords Box frames Plain frames Window stools Inside shutters Inside blinds Splay boxes **Tiled** hearths Sash locks Tiled facings Back stairs Servant's room Bay window Oriels Veranda Front porch Rear porch Stoop Back areas Front areas Iron railings Stone railings Balconies Window hoods

Door hoods Door stops Door springs Plate glass Stained glass Niches **Closet** fittings Provide for heating Conservatory Corrugated glass Skylights Handrail, oak or mahogany Bracketed stairs Anchors and tie irors Vaults Angle irons Bond timbers Carving, if any Scaffolding Temporary enclosure Iron beams Iron columns Gas pipe pillars Water on main floor Taps, nickel plated Taps, plain Glazier's work Meters, syphons Elbows, pendents Painting Paper hanging Iron pipes Lead pipes Brass pipes Washers, wastes Plugs, grating Pumps, suction pipes Wall hooks, supply pipes Cast iron work Wrought iron work Stucco work generally

Stucco friezes, enrichments Stucco pateras, panels Stucco moldings Stucco beads, straight Stucco beads over arches Stucco arrises, quirks Stucco reveals angles Stucco centerpieces General plastering Two coats Three coats Lathing Quality of laths Sand, lime and hair Plaster of Paris Clean water Sound story joists Studding for partitions Beams Trimmers for hearths Trimmers for stairs Trimmers for chimneys Strapping walls Dimensions of strapping Wooden bricks Plugging walls Nailing strips Temporary sashes Lanterns Louvres Thresholds If metal ceilings If metal cornices Metal centerpieces Bridging joists Bridging studding Dimensions of studs Double partitions for sliding doors Lining pocket of sliding doors Hanging sliding doors

Framing wooden house **Boarding** inside Boarding outside Boarding both sides Papering one or both sides Horizontal boarding Diagonal boarding Tar paper or plain paper Outriggers Towers Two-story bay windows Two-story oriels Two-story balcony Two-story porches Two-story verandas Three or more stories of same Iron railings for balconies Wood railings for same Ornamental iron column Ornamental brackets, iron Iron supports for platform Iron trusses for balconies Iron plates for piers Other iron work Siding frame buildings Half-timbered building Rough cast building Brick veneered building Wood cornice outside Metal cornice outside Shingle cornice outside Brick cornice outside Stone cornice outside Attic floor joists Rafters Collar beams Trusses for roofs Framing for dormers Framing for eye-winkers Dormer windows **Climney** stacks

Framing roof Boarding roof Mortar under shingles Mortar under slate Asbestos paper under covering Common paper under covering Shingle roof Slate roof Tile roof Composition roof Tin roof Galvanized iron roof **Roofs** painted Flashing of all kinds Tin flashings Zinc flashings Galvanized iron flashings Eave troughs **Conductor** pipes Size of conductor pipes Mansard roof Saddle roof Hip roof Flat roof Tower roof Square tower roof Conical roof Steeple roof Polygon roof Bay window roof Porch roof Roof over balcony Veranda roof Framings for veranda Chamber floors Attic floors Bedroom fittings Number of doors in bedrooms Washbasins Closets, Drawers and fitments Servants' bedrooms

Hall, sewing room Continnous stairway Bathroom and fitments Water closet, in what style Bathroom washstand Linen closet Nurserv Fireplaces Mantels Tiling for fireplaces Base, style of finish Built in seats Finish in main bedroom Finish in nursery Finish in servant's room Finish in bathroom Finish in hall Finish in closets Openings and arches Style of painting Pine finish Hardwood finish Character of finish Cost of hardware Style and cost of bath tub Style of water closet Marble washstand Tiled walls Tiled floor Marble lined walls Ventilation Air duets Register Bath trimmings Shower bath Hot and cold water Stairway to attie Attie storerooms Attic, clothes drying room Children's playroom in attie, Inside trim of dormer windows

General finish of attic Water closet and lavatory in attie Painting in attie Attic doors Heating attie Attic storeroom Children's toy room Hall in attic Railing around attie stairway Closets in attie Water in attie Plastering in attie Attic walls all boarded Matched ceiling in attie Attic hardware Chimney tops Style of chimney tops Chimney pots Finishing top of ehimney Stone tops Cement tops Metal tops Roof decks Railing for decks Rolls for ridges Cresting for ridges Wood eresting Metal crestings Terra cotta erestings Terra cotta panels Terra cotta work generally Hatehway in deek Seuttle in deck Lead work Copper work Tin work Roof pain'ing Painted or dipped shingles Stairs to roof or deek Flagpole

Halyards Wire guards Snow guards Storm sashes Storm doors Screen doo: 5 Wire screens for windows Wood gables Brick or stone gables Half-timbered gables Plastered gables Shingled gables **Deafening floors** Deafening walls Pugging floors Sub-floors **Diagonal** floors Rough floors Cellar sleepers Cedar posts Chestnut posts Spandid panels Lattice work Entrance approach Porte-cochère Stepladders Refrigerator Cold storage shelving Wine bottle racks

Folding partitions Boxed shutters boxed blinds Sliding blinds Rolling blinds Venetian blinds Dumb waiter Transom doors Transom windows Mullion windows Circular top windows Elliptical windows Double-hung windows Single-hung windows Windows, plain Windows, ornamental Pavements Slop hoppers Vestibule Vestibule partition Vestibule floor Hardwood or tile Wainscot in vestibule Wainseot up stairway Paneled stair strings Hardwood stairs Wood-shed Coal-shed

While the foregoing does not pretend to give all the items that may be required, it offers to the estimator some hints as to what is required, in a general way, for domestic buildings. For factories, stables, barns, warehouses, public buildings, churches, schools, railway stations, and similar work, a more elaborate list would be required, but the estimator should be able to find all the items in the specifications prepared for the work under consideration, and if he is thorough he will add

to the list as given above such items, with their cost, as he goes over them when figuring.

# DIFFERENT METHODS OF ESTIMATING

It is said there are not less than five different methods of estimating. Four of these are uncertain, but answer for the purpose of getting an approximate cost of some proposed work, and are chiefly made use of by architects and engineers to give their clients an idea of cost before going into actual building operations. The fifth method, which is the only reliable method, is the taking out of exact quantities item by item.

The first of these methods is the estimating by the cost per cubic foot of similar buildings. It is the best known method, and most usually adopted because of its general convenience. The dimensions are best taken by measuring the length and breadth from out to out of walls, and the height from half foundation to halfway up roof. The cubic contents, then obtained, are multiplied by the price per foot cube of some similar building. Sometimes the height is measured from the bottom of footings to half-way up the roof. Cheaper attached structures, such as annexed stables, sheds, etc., should be kept separate and priced lower; while more ornamental portions, like towers and porches, should be valued at a higher rate than the main block. Small buildings cost more in proportion than large ones of the same type.

This cubing system is open to some objections. The lumping together of solids and voids at one rate is certainly not scientific, for the same class of buildings may be divided into many rooms with numerous internal solids in the shape of walls, etc., between;

while another may have comparatively few chambers, creating much empty space. In fact, the proportion of voids to the solid structure is not a fixed quantity, so that the price per cubic foot can never be exactly regulated. This method requires a large experience and a nicety in pricing which the estimator cannot always possess. The description and quality of materials and workmanship, too, are seldom the same; neither are the conditions of contract, and these variations are frequently overlooked when a certain rate per cubic foot is assumed.

A second method is to take out rough quantities and price the items as the estimate proceeds. In this case the quantities of materials and workmanship are ascertained from the drawings in a broad and comprehensive manner, the work being concentrated as much as possible into a few specific items and afterwards priced accordingly. Although this course is perhaps less generally used than any other for estimating purposes, yet it is one of the most reliable methods that can be adopted when time and circumstances do not admit of detailed quantities and prices. The fact that such a method is not more frequently used is probably due in a great measure to the want of a readily accessible table of prices for the different groups of materials and labor. Slightly more time is also required for this purpose than when the cost is arrived at by the cubic contents or any other methods except by detail pricing. The final result, however, is nearer the truth than it would be by cubing. In estimating by this method it will be well to add 10 per cent for contingencies.

When rough quantities are being taken for an approximate estimate, it is desirable that the various descriptions of materials and workmanship should be grouped

together so as to form as few separate items as possible; also, in all cases where it can be done, the items should be priced as per square of 100 feet superficial, for the sake of uniformity and convenience.

· walls should be classed according to their matend thickness, at the same time stating whether external or internal. Each item should include all necessary digging, footings, doors, windows, and finishings of wall surfaces, such as plastering, facings to external walls, etc., so that the item, and consequently the price, shall be inclusive of everything that appertains to the various enclosures or divisions of the building. For this purpose the superficial area of the walls should be obtained by taking the extreme length of each wall by the height from the bottom of the footings to the top of the eaves, in cases where the thickness of the wall is the same throughout. Should the wall vary in thickness, either in its length or height, each portion should be measured separately. No deductions must be made for door, window or other openings. Bay windows, chimneys and other additions of a like nature should be numbered and priced according to their materials and workmanship.

The floors may be dealt with in a manner similar to that described for the walls. The ground and upper floors must be kept separate, and classed according to the materials and finishings required. The item for wood floors on the ground floor to include sleepers, dwarf walls, joists, boarding, hearths, etc., together with a layer of concrete on brick rubbish over the whole area, and all necessary digging for same. Similarly, concrete or other floors will include all materials, labor, and finished surfaces that may be required. The upper floors to be treated in a similar manner.

The item to include all joists, boarding, hearths, ceilings, cornices, and whitening or coloring the same. The roof coverings to be measured on the slope, the item being inclusive of roof trusses, rafters, boarding, shingling, slating or other covering, leadwork, eavegutters, down pipes, etc. Ceiling joists, ceilings and whitening or coloring to ceilings will also be included in the same items here required.

Drains, gas and water mains, electric wiring, and items of a similar nature, should be taken at per foot or per yard run, according to sizes, including all necessary digging, laying, filling, and removal of surplus materials. Manholes, disconnecting pits, etc., to be numbered and priced according to size and average depth.

Staircases to be taken at per step, or per foot in height, classed according to their widths, and the nature of the materials and finishings. Gas and water fittings to be priced at per light or per tap, including all service-pieces from mains, digging, etc.

Fitments or furnishings generally, such as cupboards, baths, sink, w. c.'s, ranges, grates, mantels, etc., are numbered and priced according to the class of fitments, material and finishings required.

A series of average items and approximate prices adapted to this method of estimating, may be found in this work in some of the tables, rules and memoranda that follow.

The third method of estimating is by the square of 100 feet, which, under some circumstances, is quite convenient for obtaining approximate cost. Its use is principally confined to one-story buildings, such as sheds, stores, schools, churches, chapels, stables, railway stations, bungalows and similar buildings. It may,

however, be used for buildings two or more stories in height; but a considerable amount of discrimination and care must be exercised in order that the final result may be relied upon.

The superficial area is obtained by taking the dimensions from out to out of walls at the ground level, so as to include any projection of the plinth or other offset which frequently occurs at the base of a building. The result is commonly called the *plinth area* of the building. Where the materials, workmanship, or height of building or floor varies, each description or height must be kept distinct in order that they may be separately priced.

In case of one-story buildings, the price per square includes foundations, walls, floor, roof, and all finishings. Occasionally data is at hand by which buildings comprising two or more stories, such as warehouses, etc., may be priced in the same way, the price per square of "plinth area" including foundations, walls, ground and upper floors, roof, etc., all complete.

For general purposes, however, it is more convenient to separate the different floors of buildings of more than one story in height and price each floor accordingly.

When this course is adopted for two or more stories, the ground floor is taken to include foundations, floor, walls, ceiling, and all finishings. Upper floor includes floor-joists, flooring, walls, ceilings, finishings, etc., whilst the top floor includes the roof covering in addition.

Sometimes two-story buildings have both floors priced all the same rate, as it is found that the average cost of the ground floor, including the foundations, is
about the same as that of the first floor, which includes the roof covering.

It is also useful to remember that the floor area of a certain description of buildings affords some indication of the amount of accommodation provided. For class rooms in schools, the floor area accommodates from seven to ten scholars per square, being an allowance of fourteen to ten superficial feet per child.

Ordinary churches accommodate from nine to twelve persons per square, corresponding to a total floor area of eleven to eight feet superficial per sitting respectively. In mission churches, etc., the floor space frequently averages about seven feet per sitting, or at the rate of fourteen persons per square. These figures include the floor area which is necessarily absorbed by aisles, pulpit, choir, vestry, sanctuary, etc.

The actual amount of floor space required per person for seating accommodation in churches is from  $4\frac{1}{2}$  feet to  $5\frac{1}{2}$  feet, superficial.

Pews, or sittings, in churches are usually spaced from 34 to 36 inches apart (measuring from back to back of seats), whilst the average length of seat required per person is from 20 to 22 inches.

A fourth method of estimating is by unit of accommodation, and in practice it is found that for certain descriptions of buildings or works, constructed under normal conditions, the cost of such buildings or works varies (within certain limits) in a direct ratio to some known unit of accommodation or requirements.

For such buildings as hospitals, schools, churches, factories, etc., the cost can be approximately given, if the number of putients, children, etc., required to be accommodated is known. On occasions when time will not admit of even a sketch of the proposal being

made, this method affords oftentimes the only ready means of ascertaining the approximate cost. Similarly, for certain minor accessories where the cost of materials and construction varies but slightly for units of the same class, as in a range of latrines, etc., the approximate cost can be easily determined in the same way. Data for this method of estimating will be found in the rules I give in this work.

The fifth, and most correct, method of estimating is by taking out accurate quantities of materials and items of all kinds and pricing them as the figures are obtained, and then adding the cost of labor to each item. This may be called a "detailed bill of quantities." This method, because of its entailing so much labor, should be adopted only when it is intended to carry out the work and when a tender is sent in or submitted for work about to be gone on with. very laborious, and necessitates great skill and a thorough knowledge of building construction, and particularly of the work to be tendered for, so that the subject is somewhat difficult for young hands to deal with. The sys em should be divided into three parts or processes, namely, "Taking off," "Abstracting," and "Billing," the last portion showing the prices. In this method a full set of drawings of the work and copious specifications are necessary, so that the estimator can take the dimensions from one and quality of material and character of work from the other. The cost of the various descriptions of material and workmanship are then priced in accordance with the current rates obtained in the locality where the work is to be carried out. This method takes time and much labor, but it has the advantage of being correct, or nearly so, if the work is honestly and faithfully performed. In

fact, it is the only method a young contractor should use when commencing business. After years of experience and observation as a builder and contractor, cubing, or one or other of the quick methods, *may* be made use of under certain conditions, where the contractor knows what he is about. My advice, however, is to stick to the old and reliable method of estimating by items. It takes time, but the time and labor are well invested.

The young estimator must necessarily have a fair knowledge of arithmetic, particularly that branch of it termed mensuration, before he can hope to become an expert; indeed, it will be impossible for him to become an expert unless he is good at figures and has some knowledge of geometry. In order to put him in a position to be able to wrestle with problems that are sure to crop up in estimating, I deem it expedient to arm him with rules and methods for obtaining areas, dimensions, and contents of all sorts of figures or solids he may meet with.

It is but just to say that these rules and methods can be found in many works, but it has been thought expedient to reproduce them here, so that the student may have them at hand when making use of this work for study or for practical estimating. The rules and problems are selected chiefly from educational works, and the tables have been prepared by competent authorities, and have been examined and corrected, where necessary, and made suitable to the work in hand.

It is presumed, at the outset, that the reader has some knowledge of arithmetic and is therefore able to follow without difficulty the problems that follow, which, after all, should offer no serious obstruction to a thorough knowledge of their qualities.

# MENSURATION OF SUPERFICIES

Mensuration is that branch of mathematics by which we ascertain the contents or superficial areas, and the extension, solidities, and capacities of bodies.

The *area*, or superficial contents of any figure, is the measure of its surface, or the space contained within the bounds of that surface, without any regard to thickness.

In calculating the area, or the contents of any plane figure, some particular portion of surface is fixed upon as the *measuring unit*, with which the figure is to be compared.

This is commonly a *square*, the side of which is the unit of length, being an *inch*, or a *foet*, or a *yard*, or any other fixed quantity, according to the measure peculiar to different artists; and the area or contents of any figure is computed by the number of those squares contained in that figure.

For the same reason, determining the quantity of surface in a figure is called *sq. rring it*; that is, determining the square or number of squares to which it is equal.

In order to form correct estimates of the extent of surfaces and solids, various rules have been adopted, most of which, the most valuable and useful in practice, will be found accompanying their respective problems in the following treatise, and with which the mechanic may speedily perform all the calculations that ordinarily occur in the practical details of his business.

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#### DEFINITIONS

The following definitions, which are similar in substance to those found in Euclid, are here inserted for the convenience of reference.

I. *Four-sided* figures are variously named, according to their-relative position and length of their sides.

1. A *line* is length, without breadth or thickness.

2. *Parallel lines* are always at the same perpendicular distance and they never meet, though ever so far produced.

3. An *angle* is the inclination or opening of two lines, having different directions, and meeting in a point.

4. A parallelogram has its opposite sides parallel and equal.

5. A *rectangle*, or *right parallelogram*, has its opposite sides equal, and all its augles right augles.

6. A source is a figure whose sides are of equal length, and all its angles right angles.

7. A *rhomboid* has its opposite sides equal, and its angles oblique.

8. A *rhombus* is an equilateral rhomboid, having all its sides equal, but its angles oblique.

9. A *trapczoid* is a quadrilateral figure, having only two of its sides parallel.

10. A *trapezium* is an irregular figure, of four unequal sides and angles.

II. When figures have more than four sides, they are classed under the head of *Polygons*.

These again are either regular or irregular, according as their sides and angles are equal or unequal, and they are named from their number of sides or angles. Thus, a regular polygon has all its sides and angles equal.

A pentagon	has	five	sides
A hexagon	6.	six	44
A heptagon	4.6	seven	66
An octagon	6.6	eight	6.6
A nonagon	4.4	nine	6.6
A decagon	66	ten	6.6
An undecagon	66	eleven	6.6
A dodeeagon	6.6	twelve	44

III. A figure of three sides and angles is called a *triangle*, and receives particular denominations from the relations of its sides and angles.

1. An *equilateral triangle* is that whose three sides are equal.

2. The *height* of a triangle is the length of a perpendicular drawn from one of the angles to the opposite side.

3. An isosceles triangle is that which has only two sides equal.

4. The *height* of a four-sided figure is the perpendicular distance between two of its parallel sides.

#### OF FCUR-SIDED FIGURES

**Problem I.**—To find the area of a four-sided figure, whether it be a parallelogram, square, rhombus, or rhomboid.

*Rule.*—Multiply the length by the breadth or perpendicular height, and the product will be the area.



*Example.*—What is the area of a parallelogram,  $a \ b \ c \ d$ , whose length,  $c \ d$ , is 12 feet 3 inches, and whose breadth,  $a \ c$ , is 8 feet 6 inches?

BY DECIMALS.	BY DUODECIMALS.
Feet.	Feet.
12.25	12.31
8.50	8.6'
61250	6 1' 57
9800	98. 0'
104.1250 feet. A	ns. 104. 1' 6". Ans

NOTE. The fundamental problem, in the mensuration of superficies, is the very simple one of determining the area of a *right parallelogram*. The contents of other figures may readily be obtained by finding parallelograms which are equal to them.

Take any parallelogram, a b c d, and divide each of its sides, respectively, into as many equal parts as are expressed by the number of times they contain the linear measuring unit, and let all the opposite points of



division be connected by right lines. Then it is evident that these lines divide the parallelogram into a number of squares, each equal to the superficial measuring unit, and that the number of these squares, or the area of the figure,

is equal to the number of linear measuring units in the length, repeated as often as there are linear measuring units in the breadth or height; that is, equal to the length multiplied by the height, *which is the rule*.

#### OF TRIANGLES

Problem II.—To find the area of a triangle.

*Rule.*—Multiply the length of one of the sides by the perpendicular falling upon it, and half the product will be the area Or multiply half the side by the perpendicular.



*Example.*—What is the area of a triangle whose base, a b, is 18 feet 4 inches, and height, c d, 11 feet 10 inches?

#### 18.4×11.10+2=108 feet 53 inches.

*Example 2.*—How many square rods of land are there in a lot which is laid out in a right-angled triangle, the base measuring 19 rods, and the perpendicular breadth 15 rods? Ans. 142.5.

**Case II**.—To find the area of a triangle from the length of its sides.

Rule.-1. Add together the lengths of the three sides, and take half their sum.

2. From this half sun subtract each side separately.

3. Multiply together the half sum and each of the three remainders, and extract the square root of the product; the quotient will be the required area of the triangle.

*Example.*—If the sides of a triangle are 134,108 and 80 rods, what is the area?

134 108	161 134	161	161	
$\frac{80}{322 \div 2 = 161}$	27 1s <sup>4</sup> rem. half sum.	53 2d rem.	$\frac{80}{81}$ 3d	rem.

Then, to obtain the products, we have  $161 \times 27 \times 53 \times 81 =$  18661671: from which we find area= $\sqrt{18661671}=4319$  square rods.

To find the hypotenuse of a right-angled triangle, when the base and perpendicular are known.

1. Square each of the sides separately.

2. Add together these squares.

3. Extract the square root of the sum, which will be the hypotenuse.

Example.-The wall of a building, b c, on the bank of a river, a b, is 120 feet high, and the breadth of the river 210 feet: what is the length of a line, a c, which will reach from the top of the wall to the opposite bank of the river?

# $\overline{120}^2 \times \overline{210}^2 = 58500$ and $\sqrt{58500} = 241.86$ ft Ans.

To find one of the legs when the hypotenuse and the other leg are known.

Rule.-Subtract the square of the leg whose length is known, from the square of the hypotenuse, and the square root of their difference will be the answer.

Example.-The hypotenuse, a c, of a triangle is 53 yards, and the perpendicular, b c, 45 yards: what is the length of the base, a b?

 $\overline{53^2}$ - $\overline{45^2}$ =784 and  $\sqrt{784}$ =28 yds. Ans. 28 yds.

#### OF TRAPEZIUMS AND TRAPEZOIDS

Problem III.—To find the area of a trapezium.

Rule.-Divide the trapezium into triangles by drawing diagonals; and the sum of the areas of these tri-



Example.—What is the area of a trapezium whose diagonal, a c,

is 42 feet, and the two perpendiculars, d c and b f, 18 and 16 feet?

$$42 \times 9 = 378$$
  
 $42 \times 8 = 336$  = 714 sq. ft. Ans.

Problem IV.-To find the area of a trapezoid.

*Rule.*—Multiply the sum of the two parallel sides by the perpendicular distance between them, and half the product will be the area.

Example 1.—Required the area of the trapezoid, a b c d, having given  $a \ b = 321.51$ feet,  $d \ c = 214.24$  feet, and whose height is 171.16 feet.

321.51+214.24=535.75=the sum of the parallel sides. Then,  $535.75\times171.16=91698.97$ . And,  $91698.97 \div 2=45849.485$ . Ans.

#### OF REGULAR POLYGONS

**Problem V**.—To find the area of a regular polygon, or any regular figure.

Rule r.—Multiply one of its sides into half its perpendicular distance from the center, and this product into the number of sides.

It is evident, on inspection, that a regular polygon contains as many equal triangles as the figure has sides.

Thus, the adjoining hexagon has six triangles, each equal to a b c. Now, the area of a b c is equal to the product of the side a b into  $\frac{1}{2}$  of c d. The area of the



whole, therefore, is equal to this product multiplied into the *number* of sides. *Example.*—1. Required the area of a regular hexagon, each of whose sides, a o, etc., is 45 feet, and the perpendicular, c d, 24 feet.

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We first multiply one side by  $\frac{1}{2}$  of the per endicular, c d, and that product by the number of sides: this gives the area.

#### 48×12×6=3240 ft. Ans.

To facilitate the measurement of polygons, the following table is constructed, showing the multipliers of the ten regular polygons, when the sides of each are equal to 1:

No. of sides.	Name of Polygon.	Angle.	Angle of Polygon.	Area of Multipliers	A	в	С
<b>3</b> 4 5 6 7 8 9 .0 11 12	Triangle Square Pentagon . Hexagon . Octagon Nonagon Decagon Undecagon Dodecagon	$120 \\90 \\72 \\60 \\513 \\45 \\40 \\36 \\323 \\11 \\30$	$\begin{array}{c} 60^{\circ}\\ 90\\ 108\\ 120\\ 128\frac{1}{35}\\ 135\\ 140\\ 144\\ 147\frac{1}{4}\\ 150\end{array}$	$\begin{array}{c} 0.433012\\ 1.\\ 1.720477\\ 2.598076\\ 3.633912\\ 4.828427\\ 6.181824\\ 7.694208\\ 9.365640\\ 1.196152\\ 1\end{array}$	2. 1.41 1.238 1.156 1.11 1.08 1.06 1.05 1.04 1.037	1.732 1.414 1.175 =Redius .8677 .7653 .6840 .6180 .5634 .5176	.5773 .7071 .8506 Leth of side 1.152 1.3065 1.4619 1.6180 1.7747 1.9318

Now, since the areas of similar polygons are to each other as the squares of their homologous sides, if the square of a side of a polygon be multiplied by the multiplier of the like figure, the product will be the area sought. And hence we have,

1º: tabular area : : any side squared : area.

To find the area of a regular polygon, when the side only is given.

*Rule.*—Multiply the square of the side by the multiplier opposite the name of the polygon in the above table, and the product will be the area.

*Example.*—What is the area of a regular decagon whose side is 87 feet?

87<sup>2</sup>×7.694208=58237.46. Ans.

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### ADDITIONAL USE OF THE ABOVE TABLE

The third and fourth columns of the table will greatly facilitate the construction of those figures with the aid of the sector. Thus, if it is required to describe an *octagon*, opposite to it, in the third column, is 45; then with the chord of 60 on the sector as radius, describe a circle, taking the length 45 on the same line of the sector; mark this distance off on the circumference, which, being repeated around the circle will give the points of the side.

The fourth column gives the angle which any two adjoining sides of the respective figures make with each other.

Take the length of a perpendicular drawn from the center of one of the sides of a polygon, and multiply this by the numbers in column A; the product will be the radius of the circle that contains the figure.

The radius of a circle, multiplied by the number in column B, will give the length of the side of the corresponding figure which that circle will contain. The length of the side of a polygon, multiplied by the corresponding number in the column C, will give the radius of the circumscribing circle.

#### OF IRREGULAR BODIES

To find the area of an irregular polygon.

Rule.—Draw diagonals to divide the figure into trapeziums and triangles; find the area of each separately, and the sum of the whole will give the area required.

What is the area of the adjoining polygon, a b c d cf g h?



**NOTE** The triangle, h c e, is solved by Problem II, Case II.

**Problem VI.**—To find the area of a long irregular figure, bounded on one side by a straight line.

*Rule.*—1. Measure the breadth in several places, and at equal distances from each other.

2. Add together all the different breadths, and half the sum of the two extremes.

3. Multiply this sum by the base line, and divide the product by the number of equal parts of the base.



*Example.*—1. The breadths of an irregular figure, a b c d, at five equidistant places, being 8.2, 7.4,

9.2, 10.2, 8.6, and the whole length 39, required the area.

8.2 8.6 2)16.8=sum of extremes. 8.4=mean of extremes	35.2 = sum. 39 3168 1056
7.4 9.2 10.2 35.2 sum	4) <u>1372.8</u> <u>343.2</u> . Ans.

2. The length of an irregular figure being 84, and the breadths at six equidistant places, 17.4, 20.6, 14.2, 16.5, 20.1, 24.4, what is the area? 1550.64. Ans.

E. If the perpendiculars or breadths be not at equal dis-N tances, add them together, and divide their sum by the number of them, for the mean breadth; then multiply the mean breadth by the length, and the product will be the whole area not far from

#### OF THE CIRCLE AND ITS PARTS

#### DEFINITIONS

1. A circle is a plane figure, bounded by a curved line, called the circumt rence, every part of which is equally distant from a certain point within, called the

2. A diameter of a circle is a straight line, passing through the center, and terminating at the circum-

3. A radius or semi-diameter is a straight line, extending from the center to the circumference.

4. A semi-circle is one half of the circumference.

5. A quadrant is one quarter of the circumference.

6. An arc is any portion of the circumference.

7. A chord is a straight line, which joins the two extremes of an arc.

8. A circular segment is the space contained between an arc and its chord. The chord is sometimes called the base of the segment. The height of the segment is the perpendicular from the middle of the base to the

9. A circular sector is the space contained between an arc and the two radii, drawn from the extremes of the arc.

10. A circular zone is the space contained between two parallel chords which form its bases.

11. A circular ring is the space between the circumferences of two concentric circles.

19. A lune or crescent is the space between two circular arcs, which intersect each other.

13. An ellipse or oval is a curve line, which returns into itself like a circle, but has two diameters of unequal length, the longest of which is called the transverse, and the shortest the conjugate axis.

Problem I.-To find the circumference of a circle when the diameter is given.

Rule .- Multiply the diameter by 3.1416, and the product will be the circumference. Or, multiply the diameter by 22, and divide the product by 7. Or, multiply the diameter by 355, and divide the product by 113.

NOTE.-The latter rule is a little more accurate than any other expressed in small numbers.



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Example.-1. What is the circumference of a circle whose diameter, a b, is 40 feet?

40×3.1416=125.66. Ans.

Example .-- 2. Required the circumference of a circle whose diameter is 734.

Ans. 231,6922.

NOTE.-See Table of Circumferences of Circles.

Problem II.-To find the diameter of a circle when the circumference is given.

Rule .- Divide the circumference by 3.1416, and the quotient will be the diameter. Or, multiply the circumference by 7, and divide the product by 22.

Example.-The circumference of a circle is 69.115 yards: what is the diameter?

69.115+3.1416=22 yards.

The same result may be obtained more conveniently. by exchanging the divisor, 3.1416, for a multiplier,

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which will give the same answer, for, in the proportion 3.1416:1:: Cire. : Diam., the fourth term may be directly found by dividing the second by the first, and multiplying the quotient into the third. Thus, 1+3.1416=0.31831. Therefore, if the circumference of any circle be *multiplied* by the decimal .31831, the product will be the diameter.

In many cases there will be a decided saving of labor by exchanging the *divisor* for a *multiplier*, as will be seen in the following example:

*Example*.—What is the diameter of a circle whose circumference is 50?

· 1831=15.91550.

NOTE.—As multiplication is more easily performed than division, this last method is decidedly the more preferable.

**Problem III.**—To find the area of a circle when the diameter and circumference are both known.

Rule.—Multiply the square of the diameter by .7854. Or, the square of the circumference by .07958. Or, multiply the circumference by the diameter, and divide the product by 4; in either case the product will be the area.

*Example.*—1. Required the number of square inches in a piston whose diameter is  $12\frac{1}{2}$  inches.

 $12\frac{1}{2}^{2}=12.5\times12.5=156.25$ , and  $156.25\times.7854=122.71$  sq. in. Ans. 2. The piston of the mill

2. The piston of the railroad engine Boston is 15 inches diameter: how many square inches does it contain? 176.71. Ans,

Note.—The reason of this rule will appear by considering that if the circumference of a circle be 1, the diameter will=0.31831(Prob. II), and  $\frac{1}{2}$  of this diameter into the circumference is 0.7958=area. (See Table of Areas of Circles.)

**Problem IV.**—I. To find the length of an arc of a

circle, when either the number of degrees which it contains, or the radius, chord, and height are given.

Rule.—Multiply the number of degrees in the arc by the decimal .01745, and that product by the radius of the circle. Or, from 8 times the chord of half the arc, subtract the chord of the whole arc, and  $\frac{1}{3}$  of the remainder will be the length of the arc, nearly. Or, as 3 is to the number of degrees in the arc, so is .05236 times the radius to its length.

*Example.*—1. What is the length of an arc of 40 degrees, in a circle whose radius, a c, is 12 feet?

.0745×40×12=8.376=length of the are.

2. What is the length of an arc whose chord, a b, is 120, and whose height, p d, is 45?

 $\begin{array}{r} 120+2=60=\frac{1}{2} \text{ ehord of the are.} \\ \text{And } \overline{60^2}=3600 \\ \text{``} \quad \overline{45^2}=2025 \\ \hline 5625=\text{sum of the squares.} \\ \text{Then } \sqrt{5625}=75=\text{chord of } \frac{1}{2} \text{ the are.} \\ \text{And } 75\times8-120+3=160. \\ \text{Ans.} \\ \end{array}$ 



NOTE.—The chord of half the arc is equal to the square root of the sum of the squares of the height and half the chord of the whole arc.

II. When the chord of the arc and the chord of half the arc are given.

Rule.—From the square of the chord of half the arc subtract the square of half the chord of the entire arc; the remainder will be the square of the versed sine. Then proceed as before.

NOTE — The square root of the sum of the squares of the versed sine or height, and half the chord of the entire are is equal to the chord of half the arc.

III. When the diameter and the versed sine of half the arc are given.

*Rulc.*—From 60 times the diameter subtract 27 times the versed sine, and *reserve* the number. Multiply the diameter by the versed sine, and the square root of the product will be the *chord* of half the arc. Multiply twice the chord of half the arc by 10 times the versed sine, divide the product by the *reserved number*, and add the quotient to twice the chord of half the arc; the sum will be the length of the arc, very nearly.

TABLE OF THE RELATIVE PROPORTIONS OF THE CIRCLE, ITS EQUAL AND INSCRIBED SQUARES

-	100				
1.	The	diameter of a	ainala v	00.00	
2.	4.4	Circumforence	circle X	.8862	
3	44	diametence		.2821	==side of an equal square
4		ulameter	" X	7071	i anoquare,
4.		circumference	- 4 - O	0051	=side of an incention
5.		arc		.2231	side of all inscribed sq.
6.	4.4	side of ine 11	··· X	.6366=	=contents of incontinuit
7	4.6	side of inscribed	square×1	.4142-	diam sincer and an arribed sq.
<u>.</u>		side of inscribed	Sollare	449	diam. circumserib'g cir.
8.	••	side of a square	~quarc / 1		ircum, circumscrib'g cir
9.	44	side of a guare	XI	.128 = 0	liam, of an equal similar
		side of a square	$\times 3$	.545 = c	ircum of an equal circle.
					incum, or an equal sq.

**Problem V**.—To find the side of a square inscribed in a circle, from its circumference or diameter.

Rule.—Multiply the diameter by .7071=the side of the inscribed square. Or, multiply the circumference by .2251=side of  $D = \frac{d}{c} c$ 

the inscribed square.

*Example.*—1. The circumference of a circle is 68 inches: what **a** is the side of the inscribed square?

68×.2251=15.30 inches. Ans.



2. The diameter of a tree is  $37\frac{1}{2}$  inches at the small end: what is the measure of the side of the greatest square which can be sawed from it?

37.5×.7071=26.51 inches Ans.

NOTE.—The area of a circle is to the area of the circumscribed square as .7854 is to 1, and to that of the inscribed square as .7854 is to  $\frac{1}{2}$ . If the reader will examine the above figure, he will see that the square, A B C D, which is circumscribed about the circle, is equal to the square of the diameter of the circle, since the diameter, a c, equals the side A B, and A B squared gives the area of the square A B C D; also, that the inscribed square, abcd, is just  $\frac{1}{2}$  of the circumscribed square. Since each of the triangles into which the inscribed square is divided is precisely half of each of the four squares into which the circumscribed square, A B C D, is divided. That is, the inscribed square contains only 4 right-angled triangles, while the circumscribed square contains 8. Consequently, the square described within a circle is precisely half of the square described without it.

**Problem VI**.—To find the area of a sector of a circle. Rule.—1. Find the length of the arc by problem vii.

2. Multiply the length of the arc thus found, by half the length of the radius, and the product will be the area.

Or, as 360 degrees is to the number of degrees in the arc of the sector, so is the area of the circle to the area of the sector.

NOTE.—If the diameter of radius is not given, add the square of nalf the chord of the arc to the square of the versed sine of half the arc, and divide the sum by the versed sine; the quotient will be the diameter.

It is manifest that the area of the sector has the same ratio to the area of the circle which the number of *degrees* in the arc has to the number of degrees in the whole circumference; and the rule for finding the area



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of the sector, is the same as that for finding the area of the whole circle.

*Example.*—What is the area of a sector of a circle, a c b, in which the radius, a c, is 25 and the arc of 26 degrees?

By problem vii. Rule 3.

As,  $3:26::25 \times .05236:11.344$ ; and  $11.344 \times 12\frac{1}{2}=141.8$ . Ans.

Problem VII.--- To find the area of the segment of a circle.

Rule.—1. To the chord of the whole arc add  $\frac{4}{3}$  of the chord of half the arc.

2. Then multiply the sum by the versed sine, or height of the segment, and  $\frac{4}{10}$  of the product will be the area of the segment, very nearly.

3. Divide the height or versed sine by the diameter of the circle, and find the quotient in the column of versed sines. (See table.) Then take out the corresponding area in the next column on the right hand, and multiply it by the square of the

diameter for the answer.

Example.-1. Required the area of a circular segment whose chord, a b, = 24, and whose radius, c a, = 20 feet?



 $\overline{c a^{2}} - \overline{a p^{2}} = \overline{c p^{2}} = \sqrt{400 - 144} = 16 = c p.$ cd-cp=dp=20-16=4=height of segment.  $\overline{a p^2 + p} d^2 = \overline{c} d^2 = \sqrt{144 + 16} = 12.64911 = \text{chord } a d.$ =the chord of the segment. 12.649'1=ehord of 1 the segment.  $4.216^{6}7=\frac{1}{3}$  of the ehord of  $\frac{1}{2}$  the arc. 40.86548=the height of the segment.  $163.46192 \times 4 + 10 = 65.384768 = area of the segment.$  Ans.

(See Table of Areas of the Segments of Circles.)

#### OF LUNES

Problem VIII.-To find the area of a lune or crescent. Rule.-Find the difference of the two segments which are between the arcs of the crescent and its chord for the area.



*Example.*—The chord of two segments, *a b*, is 72, and the height of the greater segment, *h d*, is 30, and of the lesser, *h b*, *c*, 20: what is the area of the crescent?

 $30^2 + 36^2 = 2196$  and  $\sqrt{2196} = 46.8 =$  chord of half the arc.

And  $46.8 \times \frac{1}{3} = 62.4$ : Then,  $62.4 + 72 \times 30 \times \frac{1}{16} = 1612.8$  = area of segment, *abd*.

Again,  $\overline{20^2 + 36^2} = 1696$  and  $\sqrt{1696} = 41.2 =$  chord of  $\frac{1}{2}$  arc.

Then,  $41.2_{b}^{i}=50.8$ , and  $50.8+72\times20\times_{10}^{4}=982.4=$  area of segment, *a b c*.

The difference of these areas is 630.4=the area of the iune or crescent.

NOTE.—If upon the three sides of a right-angled triangle, as diameters, semicircles be described, two lunes will be formed, whose united areas will be equal to the area of the triangle.

Problem IX.--- To find the area of a circular zone.

*Rulc.*—From the area of the whole circle, subtract the areas of the two segments on the sides of the zone.

If from the whole circle there be taken the two segments, a b c and d f g, there will remain the circular zone, a c f d.

Example.—1. What is the area of the zone, a c f d, if a c is 7.75, d f 6.93, and the diameter of the circle 8?

50.26=area of the whole circle. 17.23=area of the segment, a b c. 9.82=area of the segment, d f g. 27.05



And 50.26-27.05=23.21=area of the zone, acfd.

**Problem X.**—To find the area of a ring included between the circumferences of two concentric circles.

*Rule.*—1. Square the diameter of each circle, and subtract the square of the less from that of the greater.

2. Multiply the difference of the squares by the decimal .7854, and the product will be the area.

Or, multiply the product of the sum and difference of the two diameters by .7854.

*Example.*—If the diameter of the outer circle, a b, be 221, and the inner circle, d c, 106, what is the area of the ring?

First,  $221^{\circ} \times .7854 = 38359.72$ And,  $106^{\circ} \times .7854 = 8824.75$ Ans. 29534.97



NOTE.—The area of each of these circles is equal to the square of the diameter multiplied by .7854 (Prob. 3). And the difference of these squares is equal to the product of the sum and difference of the diameters. Therefore, the area of the ring is equal to the product of the sum and difference of the two diameters, multiplied by .7854.

#### OF ELLIPSES

Problem XI.-To find the area of an ellipse.

Rule.—Multiply the longer axis by the shorter, and the product, multiplied by the decimal .7854, will be the area required.

NOTE.—A common and more scientific name for the longer axis of an ellipse, is the *transverse* or *major*, and for the shorter, the *conjugate* or *minor*.



*Example.*—1. What is the area of an ellipse whose longer axis, a b, is 70 feet, and whose shorter, d c, is 50 feet?

> $a b \times d e = 70 \times 50 = 3500.$ Then,  $3500 \times .7854 = 2748.9 = area.$

2. What is the area of an ellipse whose axes are 16 and 12? 150.79. Ans.

Problem XII. --- To find the circumference of an ellipse.

Rule .-- Square the two axes, and multiply the square root of half their sum by 3.14159; the product will be the circumference, nearly.

Example.-What is the circumference of an ellipse whose transverse and conjugate axes are 16 and 18 feet?

 $\overline{16^2 + 18^2} = 580 =$  sum of the squares of the axes.

And, 290=half sum.

Then,  $\sqrt{290} \times 3.14159 = 53.498 = \text{eireumference}.$ 

Problem XIII .- To find the area of an elliptic segment, cut off by a line perpendicular to either axis.

Rule .-- Find the area of a corresponding circular segment, having the same height and the same vertical axis or diameter. Then say, as the vertical axis is to the other axis, parallel to the segment's base, so is the area of the circular segment before found, to the area of the elliptic segment sought.

Example .- The height of an elliptic segment is 10, and the axes 25 and 35 respectively: what is the area?

10+35=.2857 tabular versed sine and segment=.18452.  $.18452 \times 35^{\circ} = 249.98$ .

Then, 25:35::249.98:349.97=area.

**Problem XIV.**—To find the area of a parabola.

Rule.-Multiply the base by the height, and twothirds of the product will be the area.



Example.—What is the area of a parabola, whose base, a b, is 26 inches, and height, d c, 18 inches?

26×18=468=product of base and height.  $468 \times \frac{2}{3} = 312 =$ area in square inches. Then 312+144=24 square feet. Ans.

Problem XV.-To find the area of a frustum of a parabola, cut off by a line drawn parallel to the base.

*Rule.*--Multiply the difference of the cubes of the two ends of the frustum by twice its altitude, and divide the product by three times the difference of their squares.

*Example.*—What is the area of a frustum of a parabola whose height, c b, is 12 feet, and v its upper end, a c, 12 feet, and its base, d f, 20 feet?



#### OF HYPERBOLAS

**Problem XVI.**—To find the area of a hyperbola.

Rule.—To five-sevenths of the abscissa, v c, add the transverse diameter; multiply the sum by the abscissa, and extract the square root of the product. Then, multiply the transverse diameter, v g, by the abscissa, v c, and extract the square root of that product. Then, to 21 times the first root, add 4 times the second root; multiply the sum by double the product of the conjugate and abscissa, and divide by 75 times the transverse; this will give the area, nearly.

*Example.*—What is the area of a Hyperbola, dfv, whose transverse diameter, vg, is 80, and conjugate, df, 50, and whose abscissa, ve, is 45?



# TABLE OF THE AREAS OF THE SEGMENTS OF A CHRCLE,

# WHOSE DIAMETER IS UNITY AND SUPPOSED TO BE DIVIDED INTO 1000 EQUAL PARTS

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.005	.00021	.041	.01093	ii .079	.02880	117	05030	.134	.07674
.004	.00033	.042	.01133	0.080	020.19		.05144	.155	.07746
.005	.00047[]	.043	.01173	0.81	02040	.118	.05209	1.156	.07819
.006	.00061	.044	0121.1	0001	.02997	.119	.05273	.157	.07892
.007	.00077	045	01955	.082	.03052	120	.05338	.158	07061
.008	.00095	0.16	01200	.083	.03107	.121	.05403	159	001004
.009	00112	040	.01297	.084	.03162	.122	05468	100	.03033
010	00120	.047	.01339	.085	.03218	123	05524	1.00	-08111
011	.00132	.048	.01381)	.086	.03271	124	050004	101	.08184
.011	.00153	.049] .	.01424	.087	03330	105	.00000	.162	.08258
.012	00174	.050	.01468	088	0990	120	.05666	.163	.08332
.013	00196	.051	01511	0.80	196600	-126	.05732	.164	.08405
.014	00219	.052	01556	-000	.03444	.127	.05799	.165	08480
.015	00243	053	01600	.090	.03501	.128	.05865	.166	08554
.016	00268	05.1	01000	160	.03558	.129	.05932	167	06004
.017	00204	055	01040	.092	.03616	.130	.05999	168	00020
.018	00320	050	01091	.093	.03674	.131	.06067	160	00700
.010	00217	0001.1	01736	.094	.03732	.132	06134	170	08/78
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.032 00	755 0	70 0	2365 .]	107; .0	)4513	145 0	7039	02 .0	9707
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	.198	3 .1102	2 .24	7.1509	5 - 29	6 1045	0 94	1 .2392	0 .393	.28652
	.199	.1116	2 .24	8 .1518	1 .29	7 1951	9 214	3 9411	1 .394	.28749
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	.209	1.1190	8, .258	.16051	.307	2016	256	95071	.404	.29729
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	.211	.1207	1 - 260	16226	.309	2064	000	25069	.400	.29925
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	.213	.12234	.262	.16401	.311	.20830	360	25155	.408	.30122
	.214	.1231(	.263	.16489	.312	.20922	361	25551	410	30220
	215	.12398	.264	.16578	.313	.21015	362	256.17	.410	30318
•	210	.12481	.265	.16666	.314	.21108	363	25743	A19	20515
•	217	.12563	.266	.16754	.315	.21201	364	25830	A12	30814
•	210	.12646	.267	.16843	.316	.21294	.365	25035	41.4	20719
'	218	12728	1.268	.16931	.317	.21387	.366	26032	415	20211
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•	221 999	10077	.276	.17108	.319	.21573	.368	.26224	417	31008
*	992	12077	.271	.17197	.320	.21666	.369	.26321	.418	31106
1	220	12112	.272	.17286	.321	.21759	.370	.26417	419	31205
1	995	12007	.213	.17375	.322	.21853	.371	.26514	.420	31304
-	226	12210	0-5	.17464	.323	.21946	.372	.26611	.421	31402
ł	297	1220.1	270	.17054	.324	.22040	.373	.26707	.122	31501
ŝ	228	13178	270	.17043	.325	.22134	.374	26804	.423	31600
.2	220	13569	.211	17/33	.326	.22227	.375	26901	.424 .:	31699
2	230	13616	270	17010	.327	.22321	.376	26998	.425 .3	31798
2	31	13730	920	12001	.328	.22415	.377 .	27095	.426 .3	31897
2	32	13815	.200	10001	.329	.22509	.378 .	27192	.427 .3	1995
2	33	13899	-201	10101	.330	.22603	.379	27289	.428 .3	2094
.3	34	13984	282	10101	.331	.22697	.380 .	27386	.429 .3	2193
.2	35	14068	281	10271	.032	.22791	.381 .	27483	.430 .3	2292
2	36	14153	285	18159	.000 .	.22885	.382 .	27580	.431 .3	2391
2	37 .	14238	.286	18519	001	22980	.383 .	27677	.432 .3	2490
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-430	328871	1.1.10	9 / 1 =	1					
40.00		- <b></b>	-04178	462	25170	1.00			
437	32026	120	0.000		100413	.475	36.7.	1 400	000000
	104010	-400	34278	1 462	92270		10011	- + <b>1</b> 08	-38070
438	2200 #1	40.00		1.400	-000/3	1.476	36970.	1 100	0
	-000000	-40 H		101	DEORO		.000010	-489	.38169
- 130	99108	1 1 1		- #U#	- 50073	1 177	26070	4000	100100
10.0	166160	-4521	34477	1000	O.F. Handlin	1	.009701	.4901	38960
1.10	- 9900 All			-409	.35779	1.170	07070		.00-00
	.332841	.4531	21570	100		1.10	.310101	.4911	28200
	000.00		1040140	.400		170	0.71.00		100908
	-3338.ill		24070	1.00		1.440	-371700	.40.2	20100
4 4 4 4	0.0.1	·	-04070	.4071	-35079	1 100	0 mon all		.00409
	.334820	.1551	94555		101012	1.4001	.37270	.102	90500
4.40	0.0.0	1. 16363	.04110	.4681	-360791	40.51	0.000	+100	.00009
- 44-51	.335890	150	O to mall		-00072	1.4811	.373700	40.1	90000
		1.101	.348/50	.469	26171	400		1234	·33009
.4441	336810	157	0.00mml		111100	-4821	.374701	doel	00800
4.4.4	and and the	-4071	.34975	.470	26071	400		-4991	.38769
.4451	337911	120	13 8 13 - 1		·00271]]	.4831	-37570#	40.01	100000
	motori	1408	.3507411	1711	20271	4.1.4	.01010[]	•490	.38869
.4461	3366UH	1201	0.0		.003/11	.4841	376701	407	0000
	.ooooull	-4091	351711	.179	90.000		.01010	.4971	.38960
.4471	220701	100		171.00	·00·1711	.4851	27770	1001	0.000
	000100	-40UE	.3527411	172	9.0 mm		.01110	.4981	.39060
418	21070	40.1	THE REAL PROPERTY IN	0110	.005711	186	27070	100	000000
++101	040110	.461	353721	17.1	0.00-	• *001		.4991	39160
				. 414	.366714	187	27070IL		00100
						12018	01910	.5001	20000

USE OF THE ABOVE TABLE

To find the area of a segment of a circle.

Rule.—Divide the height, or versed sine, by the diameter of the circle, and find the quotient in the column of versed sines.

Then take out the corresponding area, in the next column on the right hand, and multiply it by the square of the diameter; this will give the ar a of the

Example.—Required the area of a segment of a circle, whose height is 34 feet, and the diameter of the circle 50 feet?

31=3.25; and 3.25+50=.065.

.065, as per table=.021659; and .021659 $\times$  50<sup>2</sup>=54.147500, the area required.

Approximating rule to find the area of a segment of a circle.

Rule.-Multiply the chord of the segment by the . versed sine, divide the product by 3, and multiply the

Cube the height, or versed sine, find how often twice the length of the chord is contained in it, and add the quotient to the former product; this will give the area of the segment, very nearly.

Example.-Required the area of the segment of a circle, the chord being 12, and the versed sine 2.

 $12 \times 2 = 24$ ; 24 + 3 = 8; and  $8 \times 2 = 16$ .  $2^3 \div 24 = .3333$ .

Hence 16+.3333=16.3333, the area of the segment, very nearly.

# TABLE OF THE AREAS OF THE ZONES OF A CIRCLE

ent

e e

t -

V'ra'di Arun of	Harren t.s.		4.1					
Sine Segment	Sine	Area of	V'rs'd	Area of	W'rs'd	Aranal	Lava	
		negment	Bine	Segment	Sine	Segment	Sine	Area of
001 00100	0.44	0.100.0					unde	aegment
.001 .00100	.044	.04394	0.087	0.08655	1 120	19070	1.00	
	045	.04494	088	OUTE A	100	-12892	173	.16948
.003 $.00300$	046	0.1502	000	.08704	1.131	.12948	.174	17049
.004 00400	0.17	01000	1059	.08852	.132	.13045	175	17195
005 00500	.041	.04093	.090	.08951	.133	131.11	170	-17133
006100	.048	.04792	091	.09040	121	1900	.170	.17229
.006.00	.049	.04892	002	00117	104	.13237	.177	.17323
.007 .00700	.050	04001	000	-09147	-135	.13334	.178	.17416
.008 00800	051	05001	.093	.09246	1.136	.13430	179	17510
000 00000	0.01	.02051	.094	.09344	.137	13596	100	17010
.009 .00,00	.052	.05190	.095	.09442	120	12000	.100	·176C3
01000. [010.	053	.052901	006	005 10	100	13022	.181]	.17696
011 $.01100$	.054	05380	007	.09340	.139	.13718	.182	17789
.012 01190	055	05 100	.097	.09638	.140	.13814	.183	17000
012 01900	000	.03489	.098	.09736	.141	13910	194	1700-
014 01299	.050	.05588]]	.099	.09835	1.19	140000	109	.17975
.014 .01399	.057	.056871	.100	00022	140	14000	.185	.18068
015 $.01499$	.058	05787	101	10000	.145	.14102	.186]	18161
.016 .01599	050	05000	101	.10030	.144	.14198	.187	18254
017 01600	000	.05550	.102[.	.10128	.145	14201	199	100 48
019 01700	.900	.05985	.103	10226	146	14200	100 .	18347
.018 .01799	.061].	.06084	.104	10324	1 17	14405	.189 .	18439
.019 .01899	.062	06184	105	10 100	.141.	14485	.190 ,	18532
.020 $.01999$	.063	06982	100	10422	.148].	14581   .	.191	18624
.021 .02099	064	06200	1001.	10520	.149],	14676	192	18717
.022 02100	065	00082	107	10617  -	.150	14771	103	19900
023 02200	.0001	00481	108].	10715	.151	14867	10.4	10000
021 00000	.000 .	06580	109	10813	159	1.1060	104	19901
024 .02399	.067].	06679   .	110	10010	150	14902 .	195 .	18993
.025 $.02499$	.068 .	06779	111	11000	100 .	15057	196].]	19085
.026 $.02598$	069	06979	110	11008	194	15153   .	197 .1	9177
.027 .02698	070	00078	112 .	11105	155	15248	198 1	0.960
028 02708	071	00977	113[.]	1203   .	156 .1	5343	100 1	0209
020 02000	071 .0	D70762 .	114 .]	1300	157 1	5420	1000 1	9301
.029 .02898	072].(	17:134.	115 1	1307	150	5500	2010 .1	9453
.030 .02998 .	073].(	)7274	116 1	1 105	100 .1	0033	201] . ]	9544
.031 .03098	074 .0	7379		1400 .	198 '1	5627	202]_1	9636
.032 .03197	075 0	7471		1592	160 .1	5722	203 1	9797
.033 03207	070 0	1411.	118[.1	1689	161 .1	5817 9	1 1.00	0010
02207	0.010	7570 <u>  </u> .]	1.  19	1786	162 1	5011		9919
.00097	077 .0	7669  1	201.1	1883	142 1	2000	1. 60	9910
.03497]].	078[.0	7768 1	21 1	1000	1.00 .1	0000 .2	06  .2	0001
.036 $.03596$ .	079 0	7967	00 .	1980 .	104 .1	61014.2	07 .2	0092
.03696	0 1020	7005	44 .1	2077	165 .1	6195   .2	08 20	0192
.038 03706		1. 6061	23 .1	2174   .1	66 .1	6289 2	00 9	0974
030 02000 .1	0. 180	8064.1	24 .1:	2271 1	67 1	6291 0	10 .2	1214
010 .03896 .(	082 .03	8163 .1	25 1	2368	60 1	170 .2	101.20	365
040 .03995 .0	83 .08	8261 1	26 1	0465	1. 60	0478 .2	11 .20	)455
041 .04095 .0	84 05	1 0.028	07 1	1. 6042	09 .16	5572 .2	12 .20	)546
042 .04195 0	85 0	steoll .	-1 . F.	2062 . 1	70 .16	366611.2	131.90	1637
043 04201 0	NG 00	1.  664	28 .11	2658 .1	71 .16	5760 9	14 90	1707
0. 11. 1. 10	201.05	\$557  .1:	29 .1:	2755 1	72 16	385.1 0	15 00	1121
					· · · · 10	1.2	19 .20	817

Thid Area of Visid Anna A	
Sine Segment Sine Segment Sine Area of Virsid Area of the	
	a Area of
.216 2000s pre prove	aer hent
<u>917</u>	
-217 $20998$ $266$ $25285$ $215$ $6657$ $-305$ $52793$ $413$	2 .35882
-218 $21088$ $-267$ $25270$ $2010$ $29270$ $361$ $32862$ $419$	2 25020
910 $9117$ $207$ $-3370$ $316$ $29347$ $45$ $39021$ $45$	000039
(-10) $(-1178)$ $(268)$ $(25454)$ $(317)$ $(90105)$ $(92.701)$ $(11)$	1 .35995
220[.21268], 269[.25520], 214[.2014.00], 329(9), 417	36051
221 21357 970 57097 318 29502 33067 114	90107
(3) 01147 $(40, 20023, 319, 90, 70, 245, 59)$	1.00104
21144 271 25707 320 Postal 500 00 01 117	1.36152
(-23, 21536), 272, 95701, 951, -9000, 309, 3 ) +18	3621
301 21698 372 072 07101 021 29733 370 35 70 110	2007
20575 $0.171$ $(-4.6)$ $(-20875)$ $(-322)$ $(-29809)$ $(-271)$ $(-2900)$ $(-1)$	
$\sim \sim 0^{-21}$ 21715 .274 .25959 393 56. [1] $\sim 1^{-1}$ .33357 120	36326
(3, 21805 - 975 - 96049)	10250
91864 577 (20042) (324) (20052) 373 32176 (05)	1000
-1001 - 70 - 26126 - 325 - 37 - 5000 - 122 - 122	-134
$-1.21983 - 277 \cdot 26200 + 298 + 56 + 10 + 974 + 35037 + 423$	. 187
2201 22072 276 3600 200 137 37 33(1)3 191	26.5.11
$230 \rightarrow 101$ $-0.92$ $320$ $300$ $370$ $33660$ $40^{-1}$	1312 C + 1
2011 20	-36593
(-2) $(22)$ $(3)$ $(28)$ $(26158)$ $(290)$ $(3)$ $(31)$ $(33730)$ $(426)$	.36646
-232 22035 21 96541 227 30511 378 33801 197	2000 0
223 59412	.000; 3
291	.36750
-234 $-2251$ $-283$ $-26706$ $-299$ $-975$ $-1$ $-550$ $-33931$ $-429$	.36801
.235 .2260. 224 2. 70. 333 .30.00 .381 .33996 430	20020
236 22601 2018 20188 2 3, 30641 382 31060 101	+000000°
207[-2091] $280$ $26871$ 4 $30715$ $2.0$	36904
(22780   .22780   .286   .26953   (10.100	36954
.238 .22868 987 97095 300 37 (89 .384 34189 133	7001
239 99055 900 - 4000 - 336 30864 385 1952 41	21119-8
910 258 .27171 337 .30937 950 1017 334	
-240 $-23143$ $-289$ $-27198$ $-2101$ $-380$ $+317$ $-435$	3. 1
2411.23131 - 901 - 7900 - 7900 - 31011 - 387 - 34380 - 436	37
.242 93916 5 5 5 5 5 31085 388 31112 197	07 ) 0~ .
212 22301 -2 27361 340 31158 280 2 200	·5+ [
-270 $-23800$ $-292$ $-27442$ $-211$ $-21001$ $-369$ $-34506$ $-138$	3700
$-244 .23393  .293 \rightarrow 7592 $	2- 445
-245 23480 904 $-3653$ $-31305$ 391 34631 110	2 . 4 . 1
216 22 20 -37 -004 -13 31377 209 26 01	21.240
-2300.8 ")5 $.27685$ 34.1 21450 $$ 31 11	\$7.93
-247 $-23655$ $-96$ $-97766$ $-9766$ $-9766$ $-9766$ $-9766$ $-97766$ $-97766$ $-97766$ $-$	.7 LIA
-248 $-237$ 11 $-207$ $-276$ $-310$ $-31023$ $-391$ $-34817$ $-12$	2 50
240 $22400$ $11 - 24840$ $316$ $31595$ $11 - 6$	01 1
270 200 20 18 7927 1.317 31667	1333
$239[23915]$ $299$ $\times 007$ $248$ $318$ $317$ $41$	
-251 $-24001$ $-300$ $-500$ $-1000$ $-1000$ $-3100$ $-3100$ $-3100$	
259 91090 000 - 319 319 11 398 m.	
252 - 3000 301 28167 350 318 2000	. 4
(233) $(24174)$ $(302)$ $(28947)$ $(251)$ $(3107)$ $(309)$ $(12)$	1
.254 $.24260$ $.303$ $.2520$ $.351$ $.31953$ $.400$	
.255 94216 204 -3520 -3521 520240 401 5000	
256 24	
-290 $-2443$ $-305$ $-2848$ $-351$ $-29160$ $-10$ $-301$ $-4$	
2571.24518 306 9	
258 2460 (1 207 200 300 2237 101 5120 475	
250 91000 .28642 .356 32307 105 -170 .153	
	· · - ·
200 .24775 .309 28801	411.1
261 24860 210 308 32447 407 35 150 36	11) I''E
269 940 40 -385, 359 3251 - 468 9 430 3	1055
-02 -24946 311 .289, 360 205, 300 .3 3 .457, 4	096
203 25021 312 900 38 409 3. 11 458	126
2041.25116" 212 001	170
	176
	216

c	tr sof	Visid Sine	Vrea of Segment	V'rs'c Sine	trea of sment	Sinsid ane	Area of Segment	V'rs'd Sine	Area of Segment
104 467 46. 16 16 16 16	.382.55 .38293 .38331 .38331 .38369 .38406 .41 .478 .4178 .478 .411	.469 .470 .471 .472 .473	.38549 38583 38617 38650 .38683 38715 38746 .38777	$\begin{array}{c} .477\\ .478\\ .478\\ .479\\ .484\\ .481\\ .482\\ .483\\ .484\\ .484\end{array}$	.38508 .38837 .38837 .38866 .3 20 .3 22 .38949 .38975 .39001	.485 .486 .487 .488 .489 .490 .490 .491 192	.39026 .39050 .39073 .39095 .39116 .39137 .39156 .39174	.493 .494 .495 .496 .497 .498 .499 .500	$\begin{array}{r} .39120\\ .39208\\ .39222\\ .39236\\ .39248\\ .39248\\ .39258\\ .39265\\ .39265\\ .39260\end{array}$

### USE OF THE ABOVE BLE

rea e a circular zoi.

divie.

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1 1 zone is less than a semicircle, ght 1 he longest chord, and seek the toti in the colum , of versed sines. Take out the arre anding area, in the next column on the right nand, and multiply it by the square of the longest chord; the product will be the are of the zone.

Example.-Required the arc zone, whose longest chord is 50, and height 15.

15+50=.300; and .300, as 1 Hence, .28087×50<sup>2</sup>=702.19, (b

-.28087. the zone.

"Ile 2.—When the zone is greater to emicircle, take te height on each side of the dia actor of the circle, at 1 find, by Rule 1, their respective areas; the areas of these two portions added together, will be the area of the zone.

Example.-Required the area of a zone, the diameter of the circle being 50, and the height of the zone on each side of the line which passes through the diameter of the circle 20 and 15, respectively.

 $20 \pm 50 = .400$ ; .400, as per table=.35182; and .35182×50<sup>2</sup>= 879.56.

 $15 \div 50 = .300$ ; .300, as per table=.28087; and .28087 $\times 50^{2}$ = 702.19.

Hence, 879.56+702.19=1581.75.

### TABLE OF THE PROPORTIONS OF THE LENGTHS OF CIRCULAR ARCS

Hight   Length	H'ght I omath	Il en e					
of Are of Are	of Arc of Arc	H'ght	Length	Highel	Langet	1	
		of Are	of Åre	of Are	DrAm	Hight Lengt	h
.100 1.098#	1111	11			or niç	or Are of Ar	c
101 1 0000	144 1.0544	1.188	1 0017	000			
.101 1.0270	145 1.0559	100	00014	.2.52	1 1379	.276 1 109	21
-102[1.0275]	146 1 0550	1.100	1.0927	.233	1.1390	977 1 100	
.103 1 0281	147 1.0009	1.190	1.09361	.234	1 1 100		54
1011 1 0000	147 1.0567	1.191	0016	008	1.1402	.278 1.194	18
104 1.0286	.148   1.0574	109 1	0050	.2.50	1.1414	.279 1.196	1
-105 1.0291	.149 1 0580	100 1	.0950	.236	1.1425	280 1 1	
.106 1.0297	150 1.0002	.193 1	.0965	.237	11426		4
107 1 0200	.100 1.0590	.194 1	.0975	000	1.1.1.00	.281 1 .	1
100 1.0303	.151 1.0597	195 1	DOVE	-600	1448	.282 1.200	1
.108 1.0308	.152 1.0605	1001	00000	.239]]	.1460	.283 1 201	Ē
.109[1.0314]	153 1 0010	-130 T	.0995	.240 1	1471	994 1 900	
.110 1 0320	151 1.0013	.197 1.	.1005	241 1	1 100	.204 1.2028	5
1111 1 0905	104 1.0621	.198 1.	1015	910	140.5	.285 1.2042	2
111 1.0320	.155   1.0629	100 1	1007	-242 1	.14954	.286 1.2056	3
112 1.0331	156 1 0627	.100 1.	1025	.243 1	.1507	987 1 9070	<b>.</b>
.113 1.0337	157 1 0017	.200 1.	1035	244 1	1510	000 1.2070	
114 1 02 15	10/ 1.0645	.201 1.	1045	215 1	15010	288 1.2083	1
115 1 00 10	158 1.0653	.202 1	1055	410 I.	1031	289 1.2097	,
.110 1.0349	159 1.0661	202 1	1000	246 1.	1543	290 1 2120	
.116[1.0355]	160 1 0000	-203 1.	1065   .	247 1.	1555	201 1 2120	
.117 1 0361	101 1.0009	204[1.]	1075	218 1	1507	401 1.2124	
118 1 0007	101 1.0678	205 1.1	1085	510 1	1007	292 1.2138	
.110 1.0307 .	162 1.0686	206 1 1	000	440 1.	1579	293 1.2152	
.119 1.0373	163 1.0604	907 1	1030	250 1.	15911.9	204 1 2160	
.120 1.0380	164 1 0700	207 1.1	006 ] .	251 1.	1602	07 1.2100	
.121 1 0286	101 1.0703	208[1.1]	117 .	259 1	letel :	295 1.2179	
199 1 0000	100 1.0711	209 1.1	197 0		.010	296 1.2193	
122 1.0392	166 1.0719	210 1 1	107	03 1.	1628	297 1.2206	
-123 1.0399	67 1.0720		137 .2	254 1.1	640 9	08 1 9990	
.124 1.0405	69 1 0707	211 1.1	148 .2	55 1 1	652 0	00 1.2220	
125 1 0.110	00 1.0/3/	212 1.1	158 9	50 1	COP .	00 1.2235	
196 1 0 1 0	09 1.0745 .2	213 1.1	160 9		005 .3	00  1.2250	
.120 1.0418 .1	70 1.0754	11 11	100 .2	24 1.1	677 .3	01 1.2264	
[127] 1.0425 [].1	71 1 0769		180 .2	58 1.1	690 3	02 1 9970	
.128 1.0431 1	79 1 0771	19 1.1	[90][.2]	59 1.1	709 2	02 1.2218	
.120 1 0428 1	2 1.0771 .2	16 1.1:	2011.90	60 11	715 0	03 1.2292	
120 1.0108 .1	131.0780.2	17 1.19	019 90	21 1.1	15 .30	04 1.2306	
100 1.0445 .1	74 1.0789 2	18 1 19	12 .20	1 1.1	728  .30	05 1.2321	
-101 1.0452 1	75 1.0798 9	10 1 15		2 1.1	740   .30	06 1 2225	
132  1.0458   .11	76 1 0807 3	19 1.12	33  .20	3 1.1;	753 30	17 1 99 10	
133 1.0465 1	27 1 00101 .2	20 1.12	45 .26	4 1 1	66 30	1 1.4349	
134 1 0170	1.0816 .2	21 1.12	56 26	5 1 1		8 1.2364	
125 1 0472	8 1.0825 .21	22 1 12	66 90		18 .30	9 1.2378	
100 1.04/9 .17	9 1.0834 29	22 1 1 2	77 .20	0 1.17	91  .31	0 1.2393	
-130 1.0486 .18	0 1.0812 99		11 .26	7 1.18	04 .31	1 1 2407	
.137 2.0493 18	1 1 0070 00	4 1.128	.26	8 1.18	16 21	9 1 0 100	
.138 1.0500 10	9 1.0662 .22	5 1.130	001.269	0 1 10	20	4 1.2422	
130 1 0500 18	4 1.0861 .22	6 1.131	1 971	1 1 10	-0 .31	5 1.2436	
110 1 0518 .18	3 1.0870 .22	7 1 1 2 9	00 07	11.18	13 .31.	4 1.2451	
1.0515 .18	4 1.0880 29	0 1 100		1.18	56 .31/	5 1.2465	
141 1.0522 18.	5 1 08801 55	1.133	3 .272	2 1.18	39 310	1 1 9100	
.142 1.0529 19	8 1 0000 -22	1.134	4 .273	1.18	29 21-	1.2480	
143 1 0527 10	1.0898 .236	1.135	6 .27.1	1 104		1.2495	
1.00011.18	11.0908 .231	1.136	7 975	1.100	-318	1.2510	
			10 - 210	1.190	<u>×   .319</u>	1.2524	

б2

H'gh:   Length	H'wht Land	11			
of Are of Are	of Are of Are	of Arc of	ngth H'ght Are of Are	Length	Hight   Length
.320 $1.2539$	.357 1 3119	200 -			of Are of Are
.321 $1.2554$	358 1 312	.093 1.3	3711 .429	1.4349	465 1 502
.322   1.2569	.359 1.3144	205 1 0	728 .430	1.4367	.466 1.504
.323 1.2584	.360 1.3160	208 1.3	746 .431	1.4386	.467 1.5061
.324 $1.2599$	.361 1.3176	207 1 2	763 .432	1.4404	.468 1.5080
.325 $1.2614$	.362 1.3192	308 1.3	780 .433	1.4422	.469 1.5000
.326 1.2629	.363 1.3209	300 1.3	797 .434	1.4441	.470 1.5119
327 1.2644	.364 1.3225	400 1.3	815 .435	1.4459	.471 1.5138
.328 1.2659	.365 1.3241	.401 1.3	832 .436	1.4477	.472 1.5157
.329 1.2674	.366 1.3258	.402 1 20	500 .437	1.4496	.473 1.5176
.330 1.2689	.367 1.3274	.403 1 20	285 400	1.4514	.474 1.5196
229 1.0700	.368 1.3291	.404 1 30	09 440	1.4533	.475 1.5215
222 1.2720	.369 1.3307	.405 1 30		1.4551	476 1.5235
334 1 0750	.370 1.3323	406 1.30	37 449	.4570	477 1.5254
335 1 9700	371 1.3340	407 1.39	55 442	.4588	478 1.5274
336 1 2700	372 1.3356	408 1.39	72 4.1.1	.4007	479 1.5293
337 1 2798	3/3 1.3373	409 1.39	90 445 1	.4026	480 1 5313
338 1 2919	$\frac{374}{275}$ 1.3390 .	410 1.40	08 446 1	4044	481 1.5332
339 2897	276 1 9400	411 1.40	25 .447 1	4603	182 1.5352
340 1.2843	$\frac{1.3423}{277}$	412 1.404	13 .448 1	4700	183 1.5371
341 1.2858	$\frac{1.3440}{279}$	413 1.400	31 .449 1	4710	184 1.5391
.342 1.2874	270 1.3401 .4	114 1.407	9.450 1	4720	185 1.5411
.343 1.2890 3	80 1 2 100 .4	115 1.409	7 .451 1.	4757	00 1.5430
.344 1.2905 .3	81 1 2507	16 1.411	5 .452 1.	4775	0/ 1.5450
.345 1.2921 .3	82 1 2594	17 1.413	2 .453 1.	4794	80 1 5470
.346 1.2937 .3	83 1 35.11 4	18 1.415	0 .454 1.	4813 4	00 1 5500
.347 1.2952 .3	84 1 3558 4	19 1.416	8 .455 1.4	4832 4	01 1 5500
.348 1.2968 .3	85 1.3574 4	20 1.418	6 .456 1.4	4851 .4	02 1 5540
.349 1.2984 .3	86 1.3591 4	$\frac{21}{99}$ 1.420.	4 .457 1.4	1870 .4	93 1 5560
350 1.3000 .39	87 1.3608 4		458 1.4	889 4	14 1 5595
.351 1.3016 .38	88 1.3625 4		.459 1.4	908 .49	)5 1 5608
.352 1.3032 .38	9 1.3643 4	25 1 4970	.460 1.4	927 .49	6 1.5628
.555 1.3047 .39	0 1.3660 4	26 1 4200	.461 1.4	946 .49	7 1.5648
-304 1.3063 .39	1 1.3677 49	7 1 1295	.462 1.4	965 .49	8 1.5668
356 1 2007 .39	2 1.3694 .42	8 1 4321	403 1.4	984 .49	9 1.5688
	1	1.4001	.404 1.5	003  .50	0 1.5708

To find the length of an arc of a circle by the fore-

Rule.—Divide the height by the base, and the quotient will be the height of an arc, of which the base is unity. Seek in the table for a number corresponding to the quotient, and take the length of that height from the next right-hand column. Multiply the number, thus found, by the base of the arc, and the product will be the length of the arc or curve required.

OF

the 2148614 J158260370482

*Example.*—The profiles of the intradoses of the arches of a bridge are each a semi-ellipse; the span of the middle arch is 150 feet, and the height 38 feet: required the length of the curve.

38+150=.253, and .253, as per table=1.1628. Hence  $1.1628 \times 150=174.4200$ , the length required.

#### TABLE OF THE PROPORTIONS OF THE LENGTHS OF SEMI-ELLIPTIC ARCS

H'ght	Length	Hight	Langeb	direction of	1.0				
or Are	of Äre	of Arc	of Are	of Are	Length	Hght	Length	H'ght	Length
100	10410					OI Are	of Are	of Are	of Arc
101	1.0416	.265	1.2306	.450	1.4931	635	1 7050	000	
101	1.0426	270	1.2371	.455	1.5008	6.10	1.789()	.820	2.0971
102	1.0436	.275	1.2436	.460	1.5084	6.15	1.7931	.825	2.1060
103	1.0446	.280	1.2501	.465	1.5161	650	1.8013	.830	2.1148
.104	1.0456	.285	1.2567	.470	1.5238	655	1.8094	.835	2.1237
.105	1.0466	.290	1.2634	.475	1.5316	000	1.8176	.840	2.1326
.110	1.0516	.295	1.2700	.480	1.5394	665	1.8238	.845	2.1416
.115	1.0567	<b>.30</b> 0]	1.2767	.485	1.5479	670	1.8340	.850	2.1505
.120	1.0618	.305	1.2834	.490	1.5550	675	1.8423	.855	2.1595
-125	1.0669	.310	1.2901	.495	1.5620	6010	1.8505	.860	2.1685
-130	1.0720	.315	1.2960	.500	1 5700	.080	1.8587	.865	3.1775
.135	1.0773	.320	1.3038	.505	1 5785	600.	1.8670	.870	2.1866
.140 1	1.0825	.325	.3106	.510	5863	.090	1.8753	875	2.1956
.145 1	.0879	.330 1	.3175	.515	59.11	-090	.8836	880   :	2.2047
.150 1	.0933	.335 1	.3244	.520	6010	705	.8919	885	2.2139
.155 1	.0989	.340  1	.3313	.525 1	6007	710	1.9002	890 1	2.2230
.100 1	.1045 .	345 1	.3383	.530 1	6175	716	.9085	895 :	2.2322
.165 1	.1106 .	350 1	.3454	.535 1	6252	790 1	.9169 .	900 1	2.2414
.170 1	.1157	355 1	.3525	.540 1	6331	705	.9253 .	905 2	2.2506
.175 1	.1213	360 1	.3597	545 1	6100	720 1	.9337 .9	)10   2	2.2597
.180 1.	.1270 .	365 1	.3669	550 1	6198	7951	.9422 .9	15   2	2.2689
-185 1.	.1327	370 1	.3741	555 1	6567	-130 1	.9506 .9	20   2	2.2780
.190 1.	.1384	375 1	3815	560 1	66.16	740 1	.9599 .9	25   2	.2872
.195 1.	1442	380 1.	3888	565 1	6725	750 1	.9675 .9	30   2	.2964
.200 1.	1501	385   1.	Se61 .	570 1	6804	755 1	9760 .9	35   2	.3056
.205 1.	1560	390 1.	4034	575 1	6883	760 1	9845 .9	40   2	.3148
.210 1.	1620	395 1.	4107	580 1	6063	765 9	9931 .9	45 2	.3241
.215 1.	1680 .4	100 1.	4180	585 1	7012	770 0	0010 .9	50   2	.3335
.220 1.	1741 .4	05 1.	4253	590 1	7193	775 0	0102 .9	55   2.	.3429
.225 1.	1802 .4	10 1.	4327	595 1	7203	780 0	0187 .9	60   2.	.3524
.230 1.	1864 .4	15 1.	4402 .	500 1	7282	795 0	0273 .9	65   2.	.3619
.235 1.1	1926 .4	20 1.	4476 .	305 1	7364	700 0	0360 .9	70   2.	3714
.240 1.1	1989,4	25 1	45521 .(	310 1	7.1.1.1	705 0	0446 .9	75 2.	3810
.245 1.2	2051 .4	30 1	4627 .(	515 1	7595	190 2.0 200 0	1533 .98	50   2.1	3906
200 1.2	2114 .4:	35 1.4	1702 .6	620 1	7606	05 94	1020 .98	\$5 2.	4002
$\frac{255}{260}$ 1.2	177 .4.	40 1.4	778 .0	325 1	7687		708 .99	0   2.4	4098
2011 1.2	241  .4	15 1.4	854 .6	30 1 7	768 9	15 9 6	1795	15 2.	1194
		-			- 11 m	1.1 2.1	00001.10	нип 97	1201

To find the length of the curve of a right semi-ellipse.

Rule.-The rule for circular arcs in the preceding table is equally applicable here.

The two last tables are not entirely confined to works which may be carried into practice, but are useful in estimating, to a very minute degree of accuracy, the quantity of work which is to be executed from drawings to a scale.

As the tables, however, do not afford the means of finding the lengths of the curves of elliptic arcs, which are less than half of the entire figure, the following geometrical method is given to supply the defect.

To find the length of an elliptic curve, which is less than half the figure.

Let the curve, of which the length is required to be found, be abc.



Produce the versed sine, b d, to meet the center of the curve in e. Draw the right line, c e, and from the center, e, with the distance, e b, describe an arc, b h. Bisect c h in i, and from the center, c, with the radius, e i, describe the arc, i k, meeting e b produced to k; then, ik is half the arc abc.

NOTE \*.- When the quotient is not given in the column of heights, divide the difference between the two nearest heights by .5; multiply the quotient by the excess of the height given, and the height in the table first above it, and add this sum to the tabular area of the least height.

Thus, if the height is 118, .120, per table,=1.0618 .115 =1.0567

 $.0051 + 5 = .00102 \times (118 - 115) = .00306$ which, added to 1.0567=1.05976, the length for 118.

· Haswell.

OF SOLIDS BOUNDED BY PLANE SURFACES

The mensuration of solids is divided into two parts. I. The mensuration of the surfaces of solids.

II. The mensuration of their solidities.

The measure of any solid body is the whole capacity or contents of that body, when considered under the triple dimensions of length, breadth, and thickness. A cube, whose side is one inch, one foot, or one yard, etc., is called the measuring unit; and the contents or solidity of any figure is computed by the number of those cubes contained in that figure.

#### DEFINITIONS

1. A cube is a right prism, bounded by six equal square faces, of which any two, opposite to each other, are parallel.

2. A parallelopiped is a prism bounded by six quadrilateral planes, every opposite two of which are equal and parallel.

3. A prism is a solid, whose ends are parallel, similar, and equal, and the sides connecting these are parallelograms.

4. A pyramid is a solid, whose base is any plane figure, and whose sides are triangles, having all their vertices meeting together in a point above the base, called the vertex of the pyramid.

5. A frustum or trunk of a pyramid is a portion of the solid that remains after any part has been cut off parallel to the base.

6. A wedge is a solid of five sides, two of which are rhomboidal, and meet in an edge, a rectangular base, and two triangular ends.

7. A prismoid is a solid, whose ends or bases are parallel, but not similar, and whose sides are quadrilateral.

### OF CUBES AND PARALLELOPIPEDS

Problem I.- To find the lateral surface of a prism.

Rule .-- Multiply the perimeter of the base into the altitude, and the product will be the convex or lateral surface. When the entire surface of the prism is required, add to the convex surface the area of the

Example.-Required the lateral surface of a prism whose base is a regular hexagon, and whose

sides are each 2 feet 3 inches, the height being 11 feet?

2 ft. 3 in =27 in. and  $27 \times 6$ =perimeter of the base. 11 ft. =132 inches=height. Then,  $132 \times 162 = 21384$  square inches.

21384+144=148.50 sq. ft. Ans.

Problem II.- To find the solidity of a cube or right prism.

Rule.-Multiply the area of the base by

the perpendicular height, and the product will be the solid contents.

Note.-The eapacity of a vessel, in gallons or bushels, of any given dimensions, may be readily ascertained by calculating its contents in inches, and then dividing the contents by the number of cubic inches in one gallon or bushel.



Examples.-1. Required the number of ale gallons there are in a cistern which is 6 feet 8 inches deep, and whose base is 5 feet 4 inches square?

6 ft. 8 in.=80 in.

5 ft. 4 in.=64 in. Then, 642=4096, and 4003×80=3?7680=solidity in inches. And 327680+282=1162 gal.

2. What is the solidity of a prism of granite, 9 feet 2 inches long, and 16 by 12 inches side dimension, and


what will be its weight, reckoning 169 lbs. to the cubic

9 ft. 2 in.=110 in.=length.  $16 \times 12 = 192$  in.=area of base |  $192 \times 110 = 21120 = solidity$  in in. 21120 + 1728 = 12.22 cubic ft. Ans.  $12.22 \times 169 = 2065$  lbs. Aus.

#### OF PYRAMIDS

Problem III.- To find the lateral surface of a regular pyramid.

Rule.-Multiply t' perimeter of the base by the slant height, and half the product will be the surface. If the whole surface be required, add to this the area of the base.

Example.-What is the lateral surface of a regular triangular pyramid, a b c, whose slant height, d a, is 20 feet, and the sides of whose base are each 8 feet?

 $8 \times 3 = 24 =$  perimeter of the base. 20=slant height. 2)480

240=lateral surface.

Problem IV.-To find the lateral surface of the frustum of a regular pyramid.

Rule .- Multiply the perimeters of the two ends by the slant height of the frustum, and half the product will be the surface required. To this add the surface of the two ends when the entire surface is required.

Example.-What is the lateral surface of the frustum of a regular octagonal pyramid, A B C D, whose slant height, a A, is 42 feet, and the sides of the lower base, D C, 5 feet each, and of the upper base, ab, 3 feet each?



First,  $5 \times 8 = 40 =$  perimeter of lower base.  $3 \times 8 = 24 = 1$ upper 64=sum of the two ends. Then, 64=42+2=1344=area of lateral surface.

**Problem V**.—To find the solidity of a pyramid.

Rule.—Find the area of the base -1 multiply that area by  $\frac{1}{2}$  of the height.

NOTE.—This rule follows from that  $c_1$  , prism, because any pyramid is  $\frac{1}{2}$  of a prism of the same base and altitude. It is manifest, therefore, that the solidity of a pyramid, whether right or oblique, is equal to the product of the area of the base into  $\frac{1}{2}$  of the perpendicular height.

*Example.*—What is the solidity of a square pyramid,  $a \ b \ c \ d$ , the sides of whose base are each 30 feet, and its perpendicular height,  $e \ f$ , 25 feet?

First,  $30 \times 30 = 900$  = area of the base.



#### 7500=solidity.

**Problem VI.**—To find the solidity of the frustum of a pyramid.

Rule.—To the areas of the two ends of the frustum, add the square root of their product; and this sum,

С

multiplied by  $\frac{1}{3}$  of the perpendicular height, will give the solid contents.

Note.—This rule holds equally true to a pyramid of any form. For the solidities of pyramids are equal when they have equal heights and bases, whatever be the figure of their bases.

*Example.*—What is the cubic or solid contents of the frustum of a marble pyramid, whose lower base,  $a \ b \ c \ d$ , is 20 inches

inches square, and upper base, e f, 14 inches, and whose height, hg, is 8 feet 4 inches? And what will be its weight, reckoning 169 lbs. to the cubic foot?

69

d

NOTE.—By this rule, marble cutters can easily determine the solidity and weight of any piece of marble, such as shafts of monuments, slabs, etc., by reference to the Table of Specific Gravities, for a multiplier for the weight of a cubic foot or inch.

### OF WEDGES AND PRISMOIDS

**Problem VII**.—To find the solidity of a wedge.

Rule.—To the length of the edge of the wedge add twice the length of the base.

Then multiply this sum by the height of the wedge and the breadth of the base, and  $\frac{1}{6}$  of the product will be the solid contents.



*Example.*—Required the solidity of a wedge whose base, a b, is 27 feet, b d, 8 feet, and whose edge, c b, is 36 feet, and the perpendicular height 22 feet?

First, 36=length of edge.

54=twice the length of the base.  $\overline{90\times22\times8+6}$ =2660 cubic ft.

**Problem VIII**.—To find the solidity of a rectangular prismoid.

Rule.—To the sum of the areas of the two ends, a b c, d e f, add four times the area of a section, g h, parallel to and equally distant from the parallel ends, and this sum, multiplied by  $\frac{1}{2}$  of the height, will give the solidity.

*Example.*—What is the solidity of a rectangular prismoid,  $a \ b \ c \ d$ , the length and breadth of one end being 14 by 12 inches and the other 6 by 4 inches, and the perpendicular 30 feet 6 inches?



First,  $14 \times 12 = 168 = \text{area of lower base.}$  $6 \times 4 = \frac{24}{100} = "$  upper "

14+6+2=10 $12+4+2=8$ $80$ $102$	$     \begin{array}{r}       192 \\       320 \\       \overline{512}     \end{array} $
$\frac{4}{320}$	61=1 height
520=area of 4 times middle section.	512
And 31232+1728=18.074 cubic ft Ann	3072
to cable it. Alls.	31232

### OF THE CYLINDER, CONE, AND SPHERE

#### DEFINITIONS

1. A cylinder is a solid, having equal and parallel circles for its ends, and is described by the revolution of a rectangle about one of its sides.

2. A cone is a solid body, of a true taper from the base to a point, which is called the vertex, and has a circle for its base.

3. A *frustum* of a cone is what remains after a portion is cut off by a plane, parallel to the base.

4. A conoid is a solid, generated by the revolving of a parabola or hyperbola around its axes.

5. A spheroid is a solid, generated by the revolution of an ellipse about either of its axes.

6. A sphere is a solid, terminated by a curved surface, all the points of which are equally distant from a point within, called the center. A sphere may be described by the revolution of a semicircle about a diameter.

7. radius of a sphere is a line drawn from the center to any part of the surface; as,

8. The diameter of a sphere is a line drawn through the center, and terminated at both ends by the surface. All diameters of a sphere are equal to each other, and each is double the radius.

9. A segment of a sphere is a portion of the sphere cut off by any plane. This plane is called the base of the segment. The height of a segment is the distance from the middle of its base to the convex surface.

10. A zone is a portion of the surface of a sphere, included between two parallel planes, which form its bases. If the bases are equally distant from the center, it is called the middle zone. The height of a zone is the perpendicular distance between the two planes which form its bases.

11. A cylindrical ring is a solid, formed by bending a cylinder, as a cylindrical bar of iron, until the two ends meet each other.

12. A parabola is a section of a cone when cut by a plane parallel to its sides.

13. A hyperbola is the section of a cone when cut by a plane, making a greater angle with the base than the side of a cone makes.

14. The transverse axis is the longest straight line that can be drawn in an ellipse.

15. The conjugate axis is a line drawn through the center, at right angles to the transverse axis.

16. An abscissa is a part of any diameter contained between its vertex and an ordinate.

17. The focus is the point in the axis where the ordinate is equal to half the perimeter.

Problem I.—To find the convex surface of a cylinder.

Rule.-Multiply the circumference of the base by the

length of the cylinder, and the product will be the convex surface required. To this add the areas of the two ends when the entire surface is required.

*Example.*—What is the convex surface of a right cylinder, whose length is 23 feet, and the diameter of its base 3 feet?

#### 3×3.14159=9.42477 Then, 9.42477×23=216.76971=surface



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**Problem II.**—To find the solidity of a cylinder.

Rule.—Multiply the area of the base by the height, and the product will give the solid contents.



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h

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*Examples.*—1. What is the solidity of a cylinder, the diameter, a b, of whose base is 16 feet, and its height, e f, 28 feet?

First, find the area of the base by  $16^4 = 256$ . Then,  $256 \times .7854 = 201.0624 =$  area of the base. Then,  $201.0624 \times 28 = 5629.7472 =$  solid contents.

2. The Winchester bushel is a hollow cylinder, 18½ inches in diameter and 8

inches deep: what is its capacity?

First, the area of the base= $\overline{18.5^2} \times .7854 = 268.8025$ . Then, 268.8025×8=2150.42=capacity in cubic inches.

No —By this rule, every sealer of weights and measures may determine the exact capacity of any *measure* submitted to his inspection. And so any one may test the accuracy of any measure, whether dry or liquid, by reducing its capacity to cubic inches, and dividing by the number of cubic inches contained in such measure. The divisor for any measure may be found in the Table of Weights and Measures.

3. How many gallons of oil will a can of a cylindrical form hold, whose diameter is 28<sup>§</sup> inches, and whose height is 4 feet 3 inches?

#### Area of the base by the Tables of Areas of Circles=643.54; and 643.54×51+221.1841=48.39 gallons. 1 gallon=221.184 cubic inches.

**Problem III**.—To find the convex surface of a cone.

Rule .- Multiply the perimeter of the base by the stant height, and ½ the product will be the surface; to which add the area of the base when the entire surface is required.

Example.- The diameter of the base of a right cone, a b, is 3 feet, and the slant height, c a, is 15 feet: what is the convex surface?

> First, 3×3.14159=9.42477=circum. of base. Then, 9.42477×15+2=70.686 sq. ft.

**Problem IV**.—To find the solidity of a cone.

Rule — Multiply the area of the base by  $\frac{1}{3}$  of the height, and the product will be the solidity.

Example .--- What is the solidity of a right cone, whose perpendicular height, c d, is 101 feet, and the circumference of the base is 9 feet?



We here multiply the area of the base by  $\frac{1}{3}$  of the height, and the product is the solidity.

> First, 92=81, and 101+3=31=1 height. Now, 81×.7854=63.6174, area of base. Then, 63.6174×31=222.6609. Ans.

Problem V.-To find the surface of a frustum of a cone.

Rule .--- Add together the circumferences of the two ends, and multiply the sum by  $\frac{1}{2}$  the slant of the frustum; the product will be the convex surface: to which add the areas of the two bases when the entire surface is required.

NOTE.—This rule is precisely the same is that for a *frustum* of a pyramid, and if a cone be considered as a pyramid of an infinite number of sides, it is equally applicable to the measurement of the *frustum* of a cone.

*Example.*—What is the convex surface of the frustum of a cone, the circumference of the greater base, a b, being 30 feet, and of the smaller, c f, 10 feet, the slant height, c a, being 20 feet?

30+10=40=circum. of two ends.  $10=\frac{1}{2}$  slant height.  $40\times10=400=$ convex surface.

**Problem VI**.—To find the solidity of the frustum of a cone.

Rule.—Add to the areas of the two ends of the frustum the square root of their product. Then multiply this sum by  $\frac{1}{3}$  of the perpendicular height, and the product will be the solidity.

Not and a pyramid have equal bases and altitudes, they are a see a seir solidity. Consequently, the rule already given for a second second of a pyramid is equally applicable to the frustum of a cone.

*Example.*—How many gallons of ale are contained in a cistern in the form of a conic frustum, *a b c f.* if the larger liameter, *a b*, be 9 feet, and the smaller diameter, *c f.* 7 feet, and the depth, *c o d*, 9 feet?



 $\begin{array}{c} \overline{9}^{2} = 81 \\ 7^{2} = 49 \text{ and } \begin{cases} 81 \times \ 7854 = 63.61 = \text{area of lower base.} \\ 49 \times \ 7854 = 38.48 = \ \ \text{upper} \ \ \text{upper} \ \ \text{upper} \end{cases} \\ \hline 102.09 \\ \hline \end{array}$  Then,  $63.61 \times 38.48 = 2447.7 \ \ 102.09 + 49.46 = 151.55. \\ \sqrt{2447.71} = 49.46 \pm 151.55 \times 3 = 454.65 \text{ cubic feet.} \\ 454.65 \times 1728 = 785635 \text{ cubic inches.} \\ 785635 + 282 = 2785 \text{ gal. Ans.} \end{cases}$ 

OF SPHERES

**Problem VII.** — To find the surface of a sphere or globe.



*Rule.*—Multiply the diameter of the sphere by its circumference, and the product will be the surface. Or, multiply the square of the diameter by 3.14159.

*Example.*—What is the surface of a sphere whose diameter is 7 feet?

First,  $7 \times 3.14159 = 21.99113 =$  circumference. Then,  $21.99113 \times 7 = 153.93791$  sq. ft.=surface.

**Problem VIII**.—To find the convex surface of a spherical zone or segment.

Rule.—Multiply the height of the zone or segment by the whole circumference of the sphere of which it is a part, and the product will be the convex surface.

*Example.*—If the axis of a sphere be 42 inches, what is the convex surface of a segment or zone,  $a \ b \ d$ , whose height,  $c \ d$ , is 9 inches?

First, 42×3.J4159=131.9468=circumference. 9=height.

1187.5212=surface in square inches.

**Problem IX.**—To find the solidity of a sphere or globe.

Rule.—Multiply the cube of the diameter, ce, by the decimal .5236. Or, multiply the square of the diameter by the circumference, and  $\frac{1}{6}$  of the product will be the contents.

Example.—What is the solidity of a globe whose diameter, c c, is 12 inches?

 $\overline{12^2} \times 3.14159 = 452.38996 = \text{surface of the sphere.}$ Then,  $452.38996 \times 12 + 6 = 404.78 = \text{solidity.}$ Or thus:  $\overline{12^3} = 1728 = \text{cube of the diameter.}$ And  $1728 \times .5236 = 904.78 = \text{solid contents.}$ 

**Problem X.**—To find the solidity of a spherical seg-

Rule.—To three times the square of the radius, a b, of its base, add the square of its height, b c; then multiply the sum by the height, and the product by .5236, for the contents.

*Example.*—What is the solidity of the segment, a d c (of the sphere e c), whose height, b c, is 8 feet, and the diameter of whose base, a d, is 14 feet P







211×8=1688×.5236=883.836. Ans.

NOTE.—The solidity of a spherical segment is frequently required when the radius of its base is not given; but if the *diameter* of the sphere and the height of the segment be known,

the solidity may be easily found by the following:

*Rule.*—From three times the diameter of the sphere, subtract twice the height of the segment; then multiply the remainder by the square of the height, and the product by the decimal .5236.

#### OF SPHEROIDS

**Problem XI**.—To find the solidity of a spheroid.

Rule.—Multiply the square of the revolving axis by the fixed axis: and the product, multiplied by .5236, will give the solidity.

*Example.*—What is the solidity of an oblong spheroid, whose longer axis, a b, is 30, and the shorter, c d, 20, the revolving axis being c d?



 $20^{2} \times 30 = 12000$ Then,  $12000 \times .5236 = 6283.2$ . Ans.

NOTE.-If the generating ellipse revolves about its major axis, the spheroid is prolate or oblong; if about its minor axis, the spheroid is oblate.

OF PARABOLIC CONOIDS AND SPINDLES conoid.

Problem XII.-To find the solidity of a parabolic Rule.-Multiply the square of the diameter of the

base by the altitude, and the product by .3927 (which is 1 of .7851), and it will give the contents.



Example. -- What is the solidity of a parabolic conoid, whose height, fg, is 60, and the diameter, c d, of its base

### $\overline{100}^2 = 10000$

And 10000×60×.3927=235620. Ans.

Problem XIII. -- To find the solidity of a frustum of a paraboloid.

Rule.-Multiply the sum of the squares of the diameters of the two ends, a b and c d, by the height of the frustum, cf, and the product by .3927 (which is } of .7854), and it will give the contents.

Example.-What is the solidity of the frustum of a paraboloid, a b c d, whose diameter, cd, is 54, ab, 28, and height, fe, 18 inches?



 $54^{\circ}=2916.$  $28^2 = 784$ 3700

Then, 3700×18×.3927=26153.82. Ans.

Problem XIV .- To find the solidity of a parabolic spindle.

Rule. Multiply the square of the middle diameter, cd, by the length of the spindle, lm, and the product

jor axis, ixis, the

abolic

of the which

of a g, is base

of

mhe of by .41888 (which is  $\frac{8}{16}$  of .7854), and it will give the solidity.

*Example.*—Required the solidity of the parabolic spindle, lm, cd, whose length, lm, is 100, and lmdiameter, cd, 40.



**Problem XV.**—To find the solidity of the middle frustum of a parabolic spindle.

*Rule.*—Add together 8 times the square of the greatest diameter, c d, 3 times the square of the least diameter, f c, and 4 times the product of these two diameters; multiply the sum by the length, a b, and the product by .05236 (which is  $\frac{1}{60}$  of 3 1416); this will give the solidity.

*Example.*—What is the solidity of the frustum of a parabolic spindle, whose dimensions are as follows: *a b*, 60, *c d*, 40, *f c*, 30 inches?

đ

20300×60×.05236=63774.48. Ans.

OF HYPERBOLOIDS AND HYPERBOLIC CONOIDS

**Problem XVI.**—To find the solidity of a hyperboloid. *Rule.*—To the square of the radius of the base, *a s*, add the square of the middle diameter, *m r*; multiply this sum by the height, *s f*, and the product by .5236, and it will give the solidity.

*Example.*—What is the solidity of a hyperboloid,  $a \ b \ f$ , whose base,  $a \ b$ , is 40 inches, and height,  $s \ f$ , 30 inches; and whose middle diameter,  $m \ r$ , is 30 inches?



A. 1. 1000

300 And  $1300 \times 30 \times 5236 + 1728 = 11.817$  cubic feet.

**Problem XVII.**—To find the solidity of the frustum of a hyperbolic conoid.

(See the foregoing figure.)

Rule.—Add together the squares of the greatest and least semidiameters,  $a \ s$  and  $d \ r$ , and the square of the whole diameter,  $m \ r$ , in the middle of the two; multiply this sum by the height,  $r \ s$ , and the product by .5236, and it will give the solidity.

*Example.*—Required the solidity of the frustum of a hyperbola,  $a \ b \ d \ c$ , whose semidiameter,  $a \ s$ , is 20 inches, and  $d \ r$ , 10 inches; the middle diameter,  $m \ r$ , 30 inches, and whose height is 20 inches?

 $20^{2} = 400$   $10^{2} = 100$   $30^{3} = 900$ 1400

100 Then,  $1400 \times 20 \times .6236 + 1728 = 8.426$  cubic feet.

**Problem XVIII**.—To find the convex surface of a cylindrical ring.

Rule.—To the thickness of the ring, a c, add the inner diameter, then multiply this sum by the thickness, and the product by 9.8696 (which is the square of 3.1416), and it will give the convex surface required.

Example. - The thickness, a c, of a cylindrical ring is 4 inches, and the inner diameter, c d, is 14 inches; required the convex surface.

ac + cd = 4 + 14 = 18.

Then,  $48 \times 4 \times 9.8696 = 710.612$  square inches =convex surface.

Problem XIX.-To find the solidity of a cylindrical ring.

Rule.-To the thickness of the ring, a c, add the inner diameter, c d; then multiply the sum by the square of the thickness, and the product by 2.4674 (which is 1 of the square of 3.1416), and it will give

Example. - Required the solidity of an auchor ring, whose inner diameter is 8 inches, and thickness in

> First, 3+8=11  $3 \times 3$  =9=square of thickness. 99×2.4674=241.2726=solidity in inches.

#### GAUGING OF CASKS

Gauging is a practical art which does not admit of being treated in a very scientific manuer.

Casks are not commonly con-tructed in exact conformity with any regular mathematical figure most writers on this subject, however, they are considered as nearly coinciding with one of the following

 $\frac{1}{2}, \quad f \text{ is middle frustum } \begin{array}{c} \text{(of a spheroid,} \\ \text{(of a parabolic spindle,} \\ \end{array}$ The qual stums ( of a paraboloid,

and their contents in cabic inches may be found by the

rules in mensuration, for determining the solidity of these figures.

To find the contents of a cask by four dimensions.

*Rule.*—Add together the squares of the bung and head diameter, and the square of double the diameter, taken in the middle between the bung and head; multiply the sum by the length of the cask, and the product by .1309.

To find the contents of a cask in the form of the middle frustum of a spheroid.

*Rule.*—Add together the square of the head diameter and twice the square of the bung diameter; multiply the sum by  $\frac{1}{3}$  of the length, and the product by .00355, for a wine gallon of New York standard measure, or .0034 for old English gallons. If *D* and *d* = the two diameters, and *l* = the length, the capacity in inches =  $(2D^2 \times d^2) \times \frac{1}{3}l \times .7854$ . And by substituting .00355 for .7854, we have the capacity in wine gallons.

*Example.*—What is the capacity of a cask of the *second* form, whose length is 30 inches, its head diameter 18 inches, and its bung diameter 24?

$$\frac{18^{2}}{2\times24^{2}} = 324$$

$$2\times24^{2} = 1152$$

$$\frac{1476}{1476}$$

$$\frac{1}{3} \text{ of } 30 = 10$$

$$14760 \times .00355 = 52.39 \text{ wine gallons} \text{ Ans}$$

To find the contents of a cask in the form of two equal frustums of a cone.

Rule.—Add together the square of the head diameter, the square of the bung diameter, and the *product* of the two diameters; multiply the sum by  $\frac{1}{3}$  of the length and the product by .00355 for New York wine gallons, or .0034 for old English gallons of 231 cubic inches.

*Example.*—What is the capacity of a cask whose dimensions are as follows: 30 inches long, head diameter 18 inches, and bung diameter 24 inches?

 $\begin{array}{r} \overline{18^2} = 324 \\ 2\overline{4^2} = 576 \\ \text{Product of 2 diam.} = \underline{432} \\ 1\overline{332} \times 10 = 13320 \times .00355 = 46.286 \\ \text{Or } (D^2 + d^2 + Dd) \times \frac{1}{3} l \times .00355. \end{array}$ 

We are now in a position to commence work in earnest, and with this end in view we will start just as the workman starts, at the very beginning, which, in the case of a building, is the preparation of the site, the excavations, the drainage, the footings, the foundations, and so on, until the whole structure is finished; and I would like to remark before commencing that whatever method of estimating is started with, that method should be continued throughout the whole for that particular work. Sometimes, where there is any doubt as to the correctness of the result, it is a good way to finish up with one system, then to use another system, and if the two results are not wide apart, the estimate may be considered fairly correct. If, however, there is a big variation, the first estimate should again be gone over, and if the same result is obtained, or nearly the same, it may be considered fairly correct; it is well, however, to go over the second system again in order to find out where the discrepancy occurs. The price of accurate results is persistent effort.

In order to get at near approximation of the cost of work, the estimator, besides having a knowledge of the price of the various materials required, should be also conversant with the current price of labor, and to this end I give herewith the average price per hour of labor as now (1904) gathered from a number of labor

circles throughout the whole country. These prices, however, are only given merely as guides, for they will vary with time and with locality; but in the absence of proper local data, they may be used with confidence. I give the price per hour of labor, as law or custom has not yet made the length of a legal day's labor.

### AVERAGE RATES OF WAGES PER HOUR

General Laborer.				
Stone Mason	om 15	to	20	cents
Excavator.	om 4t)	to	50	cents
Bricklaver	om 18	10	22	cents
Carpenter.	om 35	10	45	cents
Plasterer	9m 35	to	50	cents
Slater fro	9m 35	to	40	cents
Painter	9m 40	to	45	cents
Plumber.	m 22	10	35	cents
Roofer	ui 45	to	55	cents
	$m_{-}35$	to	45	cents

Other trades run in about the same proportion, so that, knowing the number of hours the work will require for completion, a fair estimate of the whole cost of the work may be arrived at.

A few of the things necessary to know in connection with estimating on excavation are the capacities of the tools and appliances required on the work, such as I give below.

An ordinary one-horse cart 6 feet long by 31 feet wide and 21

feet deep will hold 45 enbic feet, or  $1\frac{2}{3}$  cubic varia. A regular builder's eart will hold 1 eubic yard. A tip-wagon will hold, when heaped, 3 cubic yards. A large wheelbarrow will hold 1/10 cubic yard. A small wheelbarrow will hold 1/12 enbic yard. A basket holds a bushel, or 1/21 enbic yard. 50 barrow loads make a good wagon load. A stone wagon will earry from  $2\frac{1}{2}$  to 6 tons. A double load of earth equals about 56 cubic feet.

A single load equals some 27 or 28 cubic feet. A single, generally, is about 1 cubic yard. A single, generally, is about 1 ton of stone, brick, etc. 500 bricks make a single load. 400 pressed bricks make a single load. 1,000 plain roofing tiles make a single load. 1,000 slates, counters, make a single load. 1,000 feet dressed lumber make a single load. 50 cubic feet of timber make a single load. 1 cubic yard of mixed mortar make a single load. 16 bushels of lime make a single load.

# Earth in excavations weighs about as follows:

1 enbie yard of common earth, 2,400 pounds.

1 cubic yard of top-soil earth, 2,000 pounds.

1 cubic yard of clay earth, 2,700 pounds.

1 cubic yard dry sand earth, 2,700 pounds.

1 cubic yard wet sand earth, 3,000 pounds.

1 cubic yard of sandy loam earth, 2,400 pounds.

1 cubic yard of mnd earth, 2,500 pounds.

1 cubic yard of gravel earth, 3,000 pounds.

1 cubic foot of cement concrete, 6 broken stones, 1 sand, 1 cement, weighs 130 pounds.

1 cubic foot of concrete, 6 broken bricks, 1 sand, and 1 cement, weighs 120 pounds.

1 cubic foot of concrete, 6 broken ballast, 1 sand, and 1 cement, weighs 140 pounds.

Increase in the bulk of earth, clay, etc., when excavated and thrown into a loose heap:

Earth and day	BEFORE DIGGING	WIEN DEG
Sand and gravel	•••••••• 1	11
Broken stones		11
Free stone.		11
Rock generally.		11
STOND	••••••••	11

#### STONE-WORK DRAIN TILES

125 pieces 2 feet long, 4-inch pipe, weigh 1 ton. 80 pieces 2 feet long, 6-inch pipe, weigh 1 ton. 42 pieces 2 feet long, 9-inch pipe, weigh 1 ton. 24 pieces 2 feet long, 12-inch pipe, weigh 1 ton.

### COST OF LABOR IN FRILADELFHIA, BALTIMORE, CHICAGO, AND OTHER LARGE CENTERS AT THE TIME OF COMPLIATION OF THIS WORK

This is not to be considered reliable, but will answer when exact data are not at hand.  $\frac{2}{8}$ 

DESCRIPTION Dig, throw out, and prepare for con- crete, 12 inches deep, per super- yard	T MADE GROUNT	CONNON GROU	STIFF (LAY OR
Digging and throwing out when more than 12 inches deep, including lev- cling, per cultic	8	10	12
Ditto in trepches, leveling, fixing, and removing, shoring and planking not exceeding 6 feet deep, per enbic vard	15	20	25
Add for each additional 6 feet in depth besides the price given, the sum, per cubic vard	20	26	32
Spreading and leveling in layers not exceeding 12 inches in depth, per enbic yard	10	15	20
Add to last item for well tamping, per	7	10	13
Returning earth, spreading, tamping, exclusive of earting or wheeling, per cubic word	10	10	10
Labor only for the	15	15	15
Paddling walls, filling cofferdams tamping clay in layers 8 or 9 inches thick, per cubic word	12	12	12
For labor only in about	••	<b>\$</b> 2	. 00
Clay tempered and puddled 6 or 7 inches deep, well tamped in place, per vard super.	•••	••	40
Covering slopes, terracing with good soil in layers about 6 inches deep, per super, yard	••	••	90
Sodding same and furnishing sod and leveling same, per super gord	•	10	••

### CARTING AWAY SUFERFLUOUS MATERIAL

Wheeling or carting staff from excavation in addition to the foregoing items, not exceeding twenty yards distance, including filling of wheelbarrows, carts, etc., and depositing solid contents on the ground, per cubic yard . . \$0.12 Add for wheeling or removing every additional 20 yards, up 100 yards from starting point, per cubic yard..... .06 Basketing earth or rubbish of my kind from the inside to the outside of a building, any floor, per cubic yard..... Removing to a distance, not exceeding I mile, .15 including loading carts, wagons, etc., -3 depositing same from vehicle, per cubic y. <sup>1</sup> 00 Add for every additional mile, per cubic yard... .60 Carting away rubbish and unloading, distance not to exceed 1 mile, per cubic yard ..... Add for every additional mile, per cubic yard .... L.50 .50 Loading or unloading barges, scows, or boats of any kind, alongside the stuff being delivered, within 12 yards of barge, etc., per ton ..... Removing by barges, scows, boats, etc., to a dis-.25 tanee of 1 mile or under, per ton..... Add for every additional mile, or part of a mile, .40 beyond the first..... Cost of driver, horse, and cart, per hour..... .15 .35 Cost of wheelbarrows, per hour..... .01 Cost of team, wagon, and driver, per hour..... Other appliances, cost must be ascertained before .35 putting in the tender for work.

#### CONCRETE WORK

Concrete should be composed of pure clean water, broken stones, or ballast or clean pit-gravel, with such a proportion of sharp sand as will fill the voids between the stones or gravel; and this latter should not be larger than such as will pass through a ring 1<del>1</del> inches in diameter. The proportion should never be



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less for Portland cement than one to six parts of stones and sand combined, and the concrete should be thrown into position steadily and as evenly as possible and tamped down in layers not more than twelve inches thick. The following prices include mixing, wheeling, throwing in place and tamping down. Of course something will depend on the cost of coment, and on the cost of aggregate, i.e., broken stone and sand.

# CONCRETE FOR FOUNDATIONS AND PAVING

Foundations for walls, etc., circular straight on i	2
thick pieces, per cubic yard	
Above foundations, underninging retain	\$4.60
or similar work per cubic and	
Blocks of such size and shape if	4.80
required and state D is D is a square, as may be	
included, and set in Portland cement, moulds	
Foundation of the standard s	5.80
roundations for paving on with brick or stone.	
4 mehes thick, per yard super	65
Ditto, 6 inches thick, per yard super	00
Ditto, 9 inches thick, per vard super	1.0*
Ditto, 12 inches thick, per yard super	1.05
Floating surface of concrete and bringing it to	1.20
fair face, per yard super	
Add for work if executed between 111	.25
water mark including full	
tides or strooms	
Add for every 10 f	1.00
fact of every 10 feet hoisted above the level of	
mist noor, for each eubie yard	50

100 cubic feet of solid stone, when broken so that the largest piece will pass through a ring 13 inches in diameter, will equal 189 cubic feet.

Through a 2-inch ring, will equal 182 cubic feet. Through a 24-inch ring, will equal 170 cubic feet.

#### CONCRETE FLOORS AND ROOFS

The concrete for floors, pavements, roof-gardens or roofs, should be made in the proportion of one part

Portland cement, four parts of broken bricks, slag or other porous aggregate, and should be small enough to pass through a 3-inch ring; but no sand should be used. Fine ashes from the smith's forge make the best material for this purpose, but it should not exceed in bulk one-third of the whole mass. The concrete should be laid in position gradually and continuously, until the whole work is done, and should be tamped concurrently as laid in place. Concrete under boarded floors, tile or brick pavements should be as above described, but in the proportion of one part Portland cement to five parts of aggregate, which, after being thrown in place, should be leveled off nicely and tamped down with a wooden pounder until it becomes pulpy and the "fat" or cement portion is brought to the surface, when it should be floated or finished to a fine smooth face with a wooden float.

# PRICES FOR CONCRETE FLOORS AND ROOFS

concrete noor, as before described, 4 inches thick.	
laid complete, per yard, super.	\$1 20
Concrete roofs, per yard super	Ø1.00
Add for each inch in At 1	1.00
Add if m f in the kness above 4 inches	.18
Add it surface is finished with granite siftings.	
1 inch thick	10
Add to floors or roofs when the under it	.10
exposed and rendered 6 to the under side is	
for line und rendered fair with line putty	
for innewniting	.12
Concrete bed under wooden floors, ground level	
as described, 4 inches thick	1 00
Chases left in floors or roofs for anneal	1.00
inserting battong install.	
and any including use of same, fixing	
and removing, and filling up cavity with con-	
crete, and making good surface after remov	
ing battens, per foot run	
Forming chappels in concrete 0	.08
aviou line C i i i concrete noors or roofs, not	
exceeding o-inch girth, per foot run	.11

#### Extra to forming 4-inch projection to 6-inch flat concrete roof, and throating on under side, per foot run....

To these figures add for hoisting every 10 feet in height, after the first 10 feet, per yard super . . .05

### EXCAVATING FOR TRENCHES, DRAINAGE, FOOTINGS AND SIMILAR WORK

As before stated, the prices given in this work are not to be considered good for all time. The prices given to-day will be found quite unreliable in a month or two,  $\checkmark$ r when applied to another locality. The prices, however, I do affix to the work specified may be considered moderate and fairly safe for competitive tendering, but it is always best to vary these prices by local quotations and current rates.

I have already given a few instructions to the intending contractor with reference to excavating, but it may be well, even though I may lay myself open to the crime of repeating myself, to reiterate in some measure those instructions and warnings.

The plans of the intended specifications should be well studied and specifications carefully read over, so as to thoroughly understand what the architect desires, and when things are not properly digested the architect should be consulted.

The site of the intended building should be visited, so that the nature of the soil may be known, the distance it is to be conveyed, the state of the roadway, and the distance the building materials have to be hauled. See to the levels, and ascertain as nearly as possible the amount of material to be removed. Sometimes, in digging, a very different soil reveals itself to that taken; there are sometimes loose sand, running

water, rock, and other obstacles that have not been considered, and the price per yard for digging, removal of loose material, strutting sides of trenches, pumping, and cost of carting may make a considerable difference. The builder who knows the locality or site and the sub-soil is, of course, in a better position than others who tender. On some sites sand may be found a few feet from the surface, and this may be valuable and make a difference to the price; or it may be the sand has been screened and placed again on the site and covered with loam, in which case the excavations will have to go down to the "virgin" soil.

The cost of materials should be obtained before estimating. The prices of stone, bricks, sand, lime, ballast, delivered on the side, are all-important preliminaries to correct estimating. The prices of bricks, sand, lime, etc., vary very much in different localities. To take brick work, several elements are necessary before a correct price can be affixed per rod; as, for example, the price of bricks in field, the carriage to works, if by barge or rail, the cost of loading, the freight, unloading, carting from wharf to works, the price per yard of sand delivered, and of lime, and cost of labor. If there are any terra cotta or drain pipes, the cost delivered on the site should be obtained from the maker, and the same for any iron work or other special material.

As all these elements are found to vary considerably, it is only possible to obtain an approximate price. The market prices of leading items in each trade ought to be known, and for this purpose trade lists and prices are necessary. The quotation of prices for particular items is important.

More uncertainty prevails in estimating excavator's

work than in any other of the builder's trades, owing to the various kinds of soil to be removed, if the soil is carted or wheeled a long or short way, if the excavation is deeper than 6 feet (the height a man can work), if filled in, where acposited. This item is taken according to the labor involved. It may consist simply of digging and carting, as in the excavation over the site, or of digging, filling, and ramming, as in trenches for foundation. In the latter, however, both kinds of labor the required. Thus, the "digging and carting" represents that portion of the excavation which is occupied by the wall and has to be removed, and the "filling and ramming" applies to that portion of earth which is filled in and rammed against the walls. Then it is necessary to keep such items separate, as, for example, the excavation to basements and those only on the surface, as in removing the top soil and wheeling away not exceeding say 9 or 12 inches deep. In the deeper excavations in friable soils timbering is necessary, as walling and strutting the sides of trenches, etc.

In pricing items of excavation, the depth and width of trench, the nature of the soil, and the quantity of timber. if necessary, the latter measured per foot super. on each side, must be known. Digging in gravel or stiff clay costs twice as much as in loose earth. The disposal of the stuff should be made clear. Thus, the part of the trench to receive concrete may be described as "excavation and carting away, or wheeling and spreading," the portion to receive the brick work being described as "digging to trenches, part filled in and rammed, and remainder carted away." The earth may be dug and thrown out, wheeled or basketed out, or carted away to make up other ground.

Depths of 6 feet, 12 feet, or 18 feet should be kept separate.

Wall trenches in width are regulated by the spread of the footings, usually twice the thickness of wall at base, and room enough for men to work in the trench on one or both sides, usually 6 inches beyond bottom course of footings.

Pumping and bailing out water is a speculative item, and its cost can only be approximately put down. I have shown in previous pages approximately the cost of handling loam, sand, gravel and general rubbish, and the prices given these hold good in nearly all cases, but exceptional conditions must be provided for.

For large trenches and foundation work, when the earth is filled in and rammed, it is perhaps better to make a separate item, as "excavation and returning, filling and ramming," the quantity measured from outer face of brick work to side of trench by the depth of the footings, and deduct this from total excavation.

Priced bills do not help the young estimator much. To take two or three priced bills of quantities for the same building will reveal extraordinary differences, arising from various circumstances—the position and facilities of the contractor, his nearness to the work, whether he has a large plant and staff of workmen, or is a man of small capital without resources; the prices also depend on whether the estimate is prepared with the aid of drawings or specifications, or simply from a bill of quantities, from the items of a day or measured account. A man may be an expert quantity taker who has not mastered the fundamental elements of pricing; the two processes are different. The expert in prices must be a man naturally addicted to study and com-

pare values, to analyze the composition of items; he must be able to arrive at a price by a calculation in detail. A mind so trained will be able to trace analogous conditions, will be able to generalize and compare. We should recommend the young estimator to master the contents of every trade list of materials and goods, and these shor be kept, classified and indexed, on some sys' ior easy reference. 'Γhe trade and cash discou ..., railway rates, cost prices, etc., should be collected and indexed for reference, and for this purpose an alphabetical index or commonplace book ought to be kept. A book for each trade should be kept to enter prices, data, ar information, always giving date. Note especially the one expended on every kind of labor, as, for example, the time taken by a laborer in digging a yard cube of clay or other material, how many yards he can do in a day; the time it will take a joiner to frame a door of a certain thickness per foot super., or the time it takes to do any unit of work.

Large quantities of material, like sand or ballast or bricks, can be procured at a cheaper rate than small supplies, and a difference of at least 10 per cent in the cost may be made; but in every particuar instance it is better to make inquiries and obtain quotations from reputable dealers and contractors.

The presence of sand on the site will often save much carting away, as the sand and ballast can be used for concrete and brick work, and before pricing items of excavation inquiries should be made as to the depth of the sand below the ground level. All above the sand has to be carted away; it may be half or twothirds of the whole depth excavated. When sand · occurs in the trenches and site considerable saving is

effected, and the exact quantity of this should be ascertained before pricing, so that an allowance can be made. Thus, in trenches say half full of good sand, one-half only of the quantity or of every yard would have to be carted away. The other portion will be a distinct gain. The sand should be valued at so much per yard cube, added to the saving of carting, so that there should be a great saving. It is better to provide that a certain sum shall be allowed by the contractor for every yard of sand found on site and used in the building.



At Fig. A, I show a section of a site that was supposed to be irregular, and where the cellar excavation and irregular ground

is shown to be removed and terraced in front of the house. This will give some idea of the proper method to figure on excavating of that kind and how the material may be disposed of.

Before proceeding further it may not be out of place to show a few examples of deep trenches for drainage or other purposes, cut in various sorts of ground, and



the methods employed of holding the backs or sides



of trench in place until the work is completed. Fig. 1 shows a trench, 3 feet 6 inches deep and 3 feet wide, that is prevented from caving in by the use of cross struts and planks placed at a distance of about 6 feet. This trench is supposed to be dug in good solid ground. These struts and planking will require about 10 feet of material for every 6 feet in length of the trench, and about one-half hour's time eparing stuff

in putting in place and preparing stuff.

Fig. 2 shows a "heading" for good ground. This, it will be noticed, is sheetpiled on top and two sides. These timbers must be sized to suit the size of cutting, and character of ground; so price must be gauged accordingly. Cost per running foot, about 65 cents.



Fig. 3 shows another heading. This is for very had group

Fig. 3

is for very bad ground, and is supposed to be made

-96

very strong. This is an expensive affair; but the materials for use in the framework, when carefully removed, may be used again for the same or similar purposes. This style would cost about \$1.50 per running foot, exclusive of digging and removing material.

Fig.4 shows a method of shoving a ditchortrench for loose earth. This may be built with the sheet

piling in two lengths, as shown. The cost of this style of shoving would be considerable and depends somewhat on the depth of cutting. Supposing this to be about 6 feet deep, the cost would be from 75 to 85 cents per running foot, which would, of course, include both sides and cost of plank-

ing and poles. Some allowance would have to be made for the return stuff, as most of the material could be used again for a similar purpose. The prices given do not include digging or removing the loose earth, but simply the shoving and the material used; but these prices will vary with the locality and cost of material and labor.

Fig. 4

The trench shown at Fig. 5 differs from those previously shown, inasmuch as this exhibits a trench with sloping or inclined sides. This is arranged for a

trench dug in loose or treacherous ground, and if made about 6 feet deep the labor and materials required to complete the shoving would cost, in round numbers,



for a deep cutting in bad

Fig. 6

or loose ground. This is an expensive necessity wherever it has to be undertaken, and requires two stories, as it were, of shoring and an extra widening of the trench at the top. The sheet piling is of plank two or three inches thick, as may be required, and each plank is pointed at one end and is driven into the soi' bottom of the wide trench as shown, and is d and made secure, after which the lower trench is excavated and secured with piling and struts as shown. This style of planking and securing the work is very expensive, and each case must be figured out for itself; the cost depending largely on depth and width of trench and quality of earth to be supported. I have known of such work as described costing \$6.50 per running foot for labor and materials for the purpose; the trench being about 14 feet deep on an average. This was exclusive of digging and removing the earth from the trench. Under the circumstances, it would be folly to give any stated prim for this work. An approximate cost can only be c ined by actual figuring on the particular work to be uone, and it is always the surest way, in cases like the one under notice, to make no allowince for seturned material, for, when taken out of the irench, it will have but little value for any other purpose.

A FEW THINGS WORTH KNOWING REGARDING EXCAVATING

The following items should aid the estimator in determining prices and arranging for space, etc.

Natural slopes (with horizontal line): ....

moist sand	(1) 11
Dry sand	Chalk
Vegetable conth	Rubble
Shinal 28	Well drained elses
Suingle	Wet 1 Wat 1 Wat 1
Gravel	wer clay
Comparet senti	Loose neat
compact earth	Firm post
	45

**Height of perpendicular face** which warious soils will retain for a short time without falling:

Clay Of the	10.0
Drained loam	12 II.
Dramed Joan	8 ft.
Ordinary earth	9 44
Dry soud or growt	<b>δ</b> Π.
Dry sand or gravel1 ft. to	2 ft.

In trimming banks for a permanent surface the slope should not be uniform, but flatter at the lower than the upper part. For instance, in the same soil (clay), a bank 5 feet high may stand at a slope of  $1\frac{1}{2}$  to 1; 10 feet high, 2 to 1; 20 feet high, 3 to 1, with practically the same permanency. The most economical section for a deep cutting or hillside would be a slope ranging from 3 to 1 at bottom to  $1\frac{1}{2}$  to 1 at the top.

Equivalents of slopes:

$\frac{1}{4}$ to $1=63^{\circ} 30'$ $\frac{1}{4}$ to $1=53^{\circ}$ $1$ to $1=45^{\circ}$ $1\frac{1}{4}$ to $1=31^{\circ} 40'$ $1\frac{1}{2}$ to $1=33^{\circ} 42'$	$\begin{array}{c} 1\frac{3}{4} \text{ to } 1=29^{\circ} 44'\\ 2 \text{ to } 1=26^{\circ} 44'\\ 3 \text{ to } 1=18^{\circ} 25'\\ 4 \text{ to } 1=14^{\circ} 12' \end{array}$
12 10 1-00 42	

Increased volume of earth in embankment over the same unmoved:

Sand1 more.Clay1 more.Gravel $1^T$  $1^T$ Large rocks $1^T$ Chalk $1^T$  $1^T$  $1^T$  $1^T$ 

A usual allowance for settlement is one inch for every foot of height, but the settlement is sometimes as great as 3 inches per foot.

A good excavator will dig and throw into a barrow in a day of ten hours:

In common ground......from 8 to 10 cub. yd. In stiff clay or firm gravel...... "5 to 6 " In hard ground (picking required)...... "3 to 5""

In excavating, a vertical throw is taken at 6 feet, and when a trench exceeds that depth, stages must be pro-

vided. In practice, stages are usually set at somewhat closer intervals.

Clay invariably swells on exposure of the face in an excavation, and allowance must be made for this in certain works, as in well-digging and tunneling.

In calculating the quantity of excavation in a trench which tapers in depth or width, the prismoidal formula should be used, viz., area of two ends plus four times middle area, and the total multiplied by one-sixth of the length.

For an irregular site take spot levels, join all up into triangles, then multiply the mean depth of each triangle by its area.

A run is a certain distance for wheeling excavated material. With a length of one run, two barrows can be kept going without waiting. The length of a run is commonly taken to be 20 yards, but according to some it is only 18 yards, while in some districts 22 yards is allowed, and in U. S. government work 25 yards make one run. If wheeled more than three runs, a higher proportionate price has to be paid.

#### WEIGHTS OF MATERIALS

	euble reet = 1 load, and	l contains	27	striked	husbola		01
	heaped bushels.			Strincy	Dusheis	ог	21
E 4	1.1.4						

54 cubic feet = 1 double load.

21 cubic feet of river sand (as filled into carta)	
22 euble feet of nit could change into carts)	weigh 1 ton.
and (as filled into carts).	6.6
22 euble feet of common ballast	
23 eubic foot of coordinate in the second se	44
- cubic feet of coarse gravel	64
24 eubic feet of elean shingle	
28 oubio foot of star 1	6.6
Lo cuble leet of still clay	64
28 cubic feet of marl	
20 ouble for a state of the sta	4.4
29 euble feet of chalk (in lump)	
33 euble feet of conth (mail 1)	••
able leet of earth (mould)	12
A tip eart will hold about 3 yard cube	
A wheelbarrow contains  $\frac{1}{10}$  yard cube.

A small earth wagon will hold 12 cubic yards.

A large earth wagon will hold 3 cubic yards.

1 yard cube of solid earth or gravel contains 27 striked bushels before digging, and 27 heaped bushels when dug

49 square yards =  $1 \mod \text{of surface digging in country}$ .

I have shown some of these tables in different forms in order to meet the several local customs of dealing with the same conditions; a method which, I think, will give this little work wider range than it would otherwise have.

I now offer some short rules on excavating that may sometimes be found handy:

A 10-ton locomotive steam crane excavator, fitted with a  $1\frac{1}{2}$ -yard cube digging bucket, will excavate and deliver into wagons from 800 to 1,000 cubic yards per day of 10 working hours according to the nature of the ground.

Work in trenches costs 20 to 30 per cent more than digging over areas where the labor is not cramped. The soil is merely deposited at a safe distance (of say 2 feet) from the edge of the trench, from whence it is wheeled or carted away. Take common ground, a man would here be able to manage only 8 yards cube in one day, as there is a limited space to work in and the soil has to be pitched out one "throw." Earth that is loose enough to shovel out without using the pick, and where only one "throw" is required, may be removed for about 12 cents per yard cubic, or for less, where a plow and scraper can be employed. With the aid of plow and scraper, earth may be removed anywhere less than 100 yards for about 16 cents per cubic yard. If loaded in carts or wagons, it will cost from 20 to 30 cents per yard. Very hard clay, gravel or hard-

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pan may cost from 40 cents to \$1.00 a yard to remove. Rock will cost from \$1.00 to \$5.00 to remove, depending on the kind of rock. Old foundations, when stone, brick, old timber and lath, etc., are buried in mortar and other debris, will cost from 50 cents to \$3.00 to remove a cubic yard from the ground to a distance not exceeding 100 yards. This includes digging, loading, chopping and unloading.

### SOME ROUGH APPROXIMATE 'PRICES

Digging in ordinary soils, not mere than 6 feet in depth, per cubic vard	<b>2</b> 0 10
Ditto, above 6 feet in depth, and not exceeding	50.18
10 feet, per cubie yard	- 21
Ditto, above 10 feet and not exceeding 14 feet in	. 41
The house it is it is a second	.25
In neavy soils, allow extra, per cubic yard	.05
Preparing for foundations, including filling in and	
ramming, per cubic yard	.25
Reducing the ground to the required level, the	
average depth not to exceed 18 inches, per	
yard super	19
Wheeling ground, clay, or gravel in harrows 20	.14
yards run, or less.	00
Ditto, for every other 20 yards or part of a num	.00
beyond the first 20 yards	0.5
Carting and shooting or delivering ditte	.05
exceeding 1 mile	
Ditto, for every additional mile and	.75
(Tolly if our to l	.25
Calculate wolls, not an all of the charged.)	
per foot mu	
Digging and staining of the depth.	
6 inches in the	
Ditto 5 foot 9 : 1	2.00
Ditw, 5 leet 3 menes	2.40

While the foregoing on "excavating" does not cover the whole ground, sufficient has been advanced to enable the estimator to get a good idea of the require-

ments to make a tolerably fair estimate of the cost of any excavations that he may be called upon to figure up. As I have before stated, the thing in estimating to insure fairly correct results is "sound judgment" added to experience. The rules and methods, published in this and other work, on estimating are simply the tools with which the estimator works. If he be a good workman, a man of judgment, he will make a good job; if not, no matter how good the tools may be, the work will show up bad, and the contractor will feel himself poorer when the work is finished than before he started.

# LAVING DRAIN PIPES, WEEPING TILES, ETC.

The size of drains are determined by the quantity of sewage to be conveyed and the velocity of the sewage flow. No house drain should, however, be less than 4 inches '.' diameter. They should be laid in perfectly straight lines with an even gradient from point to point, the necessary junctions or changes of direction being within convenient inspection chambers or manholes.

The velocity of the flow of sewage in ordinary house soil drains should be about 4½ feet per second (270 feet per minute) when flowing full, so that they may be self-cleansing when only a normal quantity of sewage is passing through them.

The quantity of sewage and waste water to be removed from dwellings, for all purposes, varies from 25 to 40 gallons per person per 24 hours. The drains should be large enough to remove one-half the estimated total daily volume of sewage within six hours.

*Rainfall.*—The provision for rainfall should be varied according to the district, the average annual rainfall for

which can be ascertained. Rain-water drains must be sufficiently large to conveniently remove the whole of the water which may be expected to fall during the prevalence of a heavy storm.

The average rainfall from roofs in this country may be taken at 16 inches per annum, after allowing for loss by evaporation, absorption, etc.

Provision should be made for removing rainfall per hour as follows:

From roofs (measured horizontally)	in denth
From paved surfaces	" acpuir
From gravel surfaced	66
From meadows or grass plots	44

For ordinary houses, drains having 4-inch branches and 6-inch mains are generally sufficient. Villas and large houses usually require larger mains, but pipes of the smallest size which may be considered adequate should be used, as being more self-cleansing than larger pipes.

An easy rule to reinember for the purpose of determining the gradients of drains so as to secure good, self-cleansing velocities for the sewage, is the following well-known "decimal ...le." Multiply the diameter of the pipe by 10, and the result gives the gradient for the drain, viz.:

iamet	er of Dra	ain	•															G	r	a.c	lie	nt	cf	Drain
4	inches	• •	•	•	•	• •	• •			•	•					,		 				1 in	1	40
6	6.4	• •	•	•	•												•					l in		60
9	66	• •	•	•	• •																.1	l in		90
12	**	۰.	•	•				•													.1	l in	b	20

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The maximum velocity and discharge of sewage from ordinary drain pipes (i. e., when running nearly full), as calculated by the Etyelivein formula is as follows, viz.:

Diameter of Pipe.	Fall.	Maximum Velocity per Minute,	Maxie ann Discharge per Minute.
Inches. 4	Feet. 1 in 40 1 in 50 1 in 60	Feet, 284 254 232	Gallons. 146 131 120
6	1 in 60 1 in 70 1 in 80	$\begin{array}{c} 287\\ 265\\ 249 \end{array}$	328 • 303 • 284
9	1 in 90 1 in 100 1 in 110	284 270 257	742 705 670
12	1 in 120 1 in 150 1 in 200	285 255 221	1318 1177 1021

*Flushing.*—Where self-cleansing falls cannot be obtained for the drains, periodical and, preferably, automatic flushing should be resorted to.

Rain-water drains. —Where drains are solely used for rain-water, much less fall is required than for sewage. Generally, a velocity of 2½ to 3 feet per second (150 to 180 feet per minute) is sufficient in order that the ordinary dust and dirt may be readily washed away; but the amount of water to be removed in a given time must be allowed .r. The drains should be surrounded in concrete when passing through buildings or near the roots of trees, or wherever they are likely to be disturbed.

Drains should be kept as far away as possible from buildings, so that the pipes and joints may not be injured or disturbed by any settlement of the walls. By this means the risk of sewage or sewer air penetrating within the buildings is minimized. For similar

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reasons the drains should not pass under houses except when absolutely univoidable, and in such circumstances heavy cast iron pipes with caulked lead joints should be used.

An unyielding bed on which to lay the drains is necessary to ensure sound and permanent work. A layer of concrete should therefore be provided under the pipes, unless the ground is naturally very hard and compact.

Branch drains should not join the main or collecting drains with level inverts. The junction should be effected within an inspection chamber or manhole, and the branch channels arranged to discharge over the channel of the main drain. Care should be taken that the branch channels are placed so that they do not discharge immediately opposite each other when entering the main channel.

Stable drainage should be kept separate from the house drainage in all cases where practicable.

Covers to inspection chambers should have a clear opening of 24 by 18 inches, so that a man may conveniently pass through them.

The materials of which drain-pipes are made varies considerably in different localities. Well-burnt stoneware pipes of good quality are thoroughly vitrified, and when broken present a fine close grain with a somewhat metallic appearance. Fire-clay pipes do not possess such a dense and close grain, and are more absorbent than stoneware pipes. Earthenware pipes are quite unsuited for use in house drainage.

Stoneware drain-pipes should be of the description known as "salt-glazed," so as to obtain an impervices and lasting surface. For ordinary house drainage purposes the pipes are usually made in 2-foot lengths.

Specially selected and tested stoneware pipes in 3-foot lengths may be obtained from manufacturers at a slight additional cost over ordinary pipes. "Tested" pipes should be capable of withstanding a pressure of 25 feet head of water without showing signs of sweating.

Neat Portland cement is generally used for jointing ordinary spigot and socket pipes, or cement and sand in the proportions of one part cement to one part sand.

Cement joints must be very carefully formed and wiped out as the work proceeds, so as to avoid burrs on the inside of pipes.

Greater security is obtained by adopting one of the several well-known forms of patent safety joints now made by the leading manufacturers. They are more expensive than pipes with ordinary spigot and socket joints, but the advantage of obtaining a stronger and safer connection more than counterbalances the additional cost.

Protection against fracture can best be obtained by entirely surrounding the pipes with concrete. A thickness of 6 inches of concrete is usually sufficient for this purpo\_\_\_.

The average thickness and weight of glazed stoneware drain-pipes per 2-foot length is as follows, viz.:

Diameter of Pipe.	Length of Socket.	Thickness of Stoneware.	Average Weight per 2 ft. Length of Pipe,
Inches. 4 6 9 12	Inches. 11 13 2 2 2	Inch.	Lbs. 18 32 58 90

The cost per foot of these pipes should be obtained

from the dealer, along with the extra cost of Wys, V's or other connections that may be required, before any estimate is made. If the drain-pipes are to be laid in concrete, the cost of the concrete and labor of putting it in place must also be added. The digging of trenches has been dealt with before, but in making an estimate this item of digging and removing the soil must not be overlooked. It is not possible to give a price for work of this kind unless the size of pipes, depth of trench, if or if not bedded in cement or concrete, etc., are given; then a price per foot in length may be arrived at.

Cast iron pipes are largely used in high-class drainage work. The cost is not much more than that of good glazed stoneware surrounded with 6 inches of concrete.

The advantages obtained by the use of cast iron pipes as compared with glazed stoneware are as follows:

1. The pipes are of greater strength. They are consequently not so liable to become fractured or broken.

2. Air and water-tight joints can be readily made by running with molten lead and caulking.

3. Fewer joints are required, owing to the longer lengths of the pipe.

For substantial work the iron pipes should be of similar thickness and strength as those used for ordinary water mains. They are generally laid in 9-foot lengths, with spigot and socket joints run with lead and caulked.

Whenever a drain passes under or through a wall it should be of iron, then if any settlement takes place the iron will offer a much greater resistance to the consequent pressure than glazed earthen tiles would.

Weeping tiles may be common field tiles, or they may be ordinary drain tiles of small diameter. They are made use of occasionally to drain around a foundation wall, or to drain under the concrete floors of a cellar.

When field tiles are employed they butt at the joints, which are not made tight, as water is intended to enter the pipes at every joint. The same, also, with ordinary tiles, the joints being left loose so that water may enter at every joint.

The cost of laying weeping tiles is very small, as a man will lay 30 or 40 feet per hour, but the cost of the tile themselves must be considered. There will be no excavating for these tiles, as, in the case of a cellar, the tiles are laid on about the same level as the foundation; the tiles are laid on a level, and against the footings. Of course, the tiles in both cases must lead into the main drain, and this may necessitate some extra digging.

### FOUNDATION FOOTINGS

In placing footings a special rate should be made, as much more c  $\rightarrow$  and time is required in getting good flat stones of the proper thickness, and leveling them on their beds, than in laying an ordinary wall. In my own practice I have usually charged up 50 per cent more per cord for footings than for the other porcion of stone wall, and this additional charge has been found not a bit too much in most cases. If the footings are of concrete, as is generally the case now, then this must be charged in accordance with the rules given under the head of concrete. Concrete footings may be flat or they may have a broad base and narrow top, just wide enough to take the walls, whether of brick or stone.

The three illustrations shown at Fig. 7 give an idea of both concrete and stone footings. The first is cc -



crete having a rectangular section, or it may be inclined from the outside. The second is formed of five thicknesses of dimension stones drawn in towards the top. This is intended to carry a very heavy wall. The third is formed of two thicknesses of dimension stones, but is not drawn in. All three of these are good examples for footings, but

they do not by any means cover the whole ground; another example is shown at Fig. 8. This is a section and is intended to carry a high and heavy wall. The concrete is 18 inches thick and



Fig. 8

fully 5 feet 6 inches across. In estimating for this, the concrete must be figured at so much per cubic yard, and full allowance made for wheeling and dumping. The brick or stone work above, until the level of the ground is reached, should be charged up about 10 per cent above the regular rates.

If footings are laid in with ordinary quarried stones

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without dressing, the cost will be about \$5.00 a cord of 100 cubic feet, exclusive of all materials. Cost of materials, stone and mortar, to be added, and if laid in cement the cost will be a trifle more.

Ordinary quarried stone laid in the wall, set in good lime mortar, is worth at the present time \$5.00 per cord of 100 feet in the wall for setting.

In buying rough quarried stones by the cord, which is the usual way in most of the states and Canada, the purchaser is supposed to receive 128 feet cubic in the rough, but the mason's measurement, including mortar joints, is 100 cubic feet in the wall for one cord; and when he buys he expects to pay for the 100 feet per cord and to receive pay on the same basis.

In putting in stone foundations as above, the estimator must make provision for all openings, and when cut stone or cement sills and lintels are used for doors or windows, they must be charged up extra by the running foot. All ventilators must be extra items and duly charged. Figure for all openings for drainpipes, water, gas, or other pipes entering the basement or cellar. All areas must be figured on by the yard super., if in cement or stone, according to prices given; steps, walls and copings must all be measured off and charged up according to size and material. Prices, if not found in this work, must be ascertained in the locality where the work is to be executed.

Sills and lintels, in either stone or cement, may be bought from the dealer by the foct super. or cubic foot, and price lists of same may be obtained from the manufacturers.

Footings and basement or cellar walls are sometimes specified to be made damp-proof, and the architect *sometimes* shows how the walls are to be con-

structed so as to be damp-proof. I show two method . both of which are expensive but certain in result. Fig.

9 shows a concrete footing with a section of concrete carried up the walls to the DAMP-PROOF height of top of cellar floor, which is also of concrete 4 or 6 inches thick. A damp-proof course of slate or asphalt is shown on a line with cellar floor,



and is continued on the outside wall to a point above the line of ground. This is an effective method. In this case the concrete is worth from 10 to 15 per cent



Fig. 10

more to put in place than if a simple footing as above. Damp-proof course is worth 'rom 15 to 25 cents a running foot, according to the thickness of the wall.

The footing shown at Fig. 10 is a still more expensive one. Here is a wide footing in concrete and a double wall for a portion of the height. There is a damp course of slate laid at L in the main

wall and level with the finishing coat of cement, M, on the cellar floor. The outside wall, R, is simply to hold back the soil on the outside, thus forming a 12-inch

space between the walls for air and to avoid damp. Concrete is thrown in between the walls below to a thickness of 8 or 10 inches, thus preventing any damp from attacking the main wall. The space between the two walls is covered over at the top with a stone slab, J, which prevents any rain or other water from gaining entrance.

Here we have several new items to figure on. Extra bricks in outside wall, covering slab, concrete in air space and damp course. Figure the concrete by the yard cubic, the slab by the running foot, the extra bricks in the usual manner and the damp course same as before mentioned.

We are now in a position to describe some of the methods of estimating as mentioned in previous pages, and will endeavor to do so before entering into detail estimating.

As I have stated before, there are five distinct methods of estimating, namely, by rough quantities, by the square, by the unit of accommodation, by cubing, and by itemizing details. The two latter may be considered the best methods of the five, and the last the best of all, though the most troublesome. Of the first three I will say but little, as they will be apt to lead the ordinary contractor into a maze of difficulties that will eventuate in loss of time and money; besides, a fairly correct description of them and the method of using them have been already presented. It may be well, however, to make a few remarks concerning them.

The cost of buildings is constantly changing, so it must be remembered that no matter what prices are given in this book the estimator must in every case use his own judgment and true knowledge when mak-

ing up his tender, and add or deduct whatever percentage may be necessary to suit the fluctuations in prices of labor and materials. During the last decade the cost of buildings of every kind has increased from 30 to 40 per cent; stone and the more elaborate buildings have increased in a greater proportion than the cheaper kind, owing perhaps to the greater cost of expert labor and the more luxurious fitments. Some idea of the cost of a proposed building may be derived from a study of the proportional cost of the various Of course the result will only be approxitrades. mate. For instance, in ordinary domestic buildings the brick work and masonry will represent from onethird to one-half of the total cost, unless the building is a frame one, in which case the wood work, including labor and hardware, will represent about three-fifths of the total cost. The following figures show, from actual experience, about the average ratio of costs of the various trades for the erection of brick or stone dwellings with slate roofs.

Excavator and drainage	Percentage of total cost.
Bricklayer and mason	· · · · 3.0
Slates and roofs	
Carpenter, hardware etc	4.5
Electric wiring, bells, and fitments	$\dots .34.0$
Plasterer, stueeo work etc	4.0
Plumber, heating, etc.	···· 6.0
Painter, glazier, paper hanger	8.0
, popul nanger	4.5
Total	100.0

S'milar tables may be constructed showing the average ratio of cost for each of the trades in the erection of public buildings, schools, churches, theaters, etc., and these tables will prove of great assistance to

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the estimator when figuring up for buildings of a similar nature. Having the total cost of one building of this kind, with the cost of each of the trades named, on the same building, the rest is easy, the difference in size and character of the two buildings being considered. This may be considered estimating by comparison. If the brick and stone work of one building costs, say 10 cents per every cubic foot of the building, then the wood work will cost, according to the rule given, about 8 cents and a fraction for a foot cubic of the whole building, and the other trades in proportion as laid down.

This method is rather arbitrary, and, while given here, is not supposed to be quite correct, but when properly understood will be found quite useful.

On the same lines I give another, which may sometimes be employed in determining the cost of labor where all materials are furnished. This is a rough and ready means of making a comparison, but is pretty nearly correct and may sometimes be used to advantage:

Exeavator																-	tota	l cos	t
Drainage, etc.		•••	•••	•	•••	• •	•••	• •	•	•	• •	•	•	•	•	•	••		
Bricklaver	•••	•••	•••	•	•••	•	• •	• •	•	•	• •	•	•	•	•	•	••		
Mason.	•••	•••	• •	•	•••	• •	•	• •	•	• •	• •	•	•	•	• •	•	•		
Slater or roofer	•••	•••	•••	• •	•	•••	•	•••	•	•••	•	•	•	•	• •		•		
Tiler	•••	•••	•••	• •	• •	• •	•	•••	•	; •	•	•	•	•	• •	•	•		
Carpenter.	•••	••••	•••	• •	•	•••	•	•••	•	•••	•	•	•	• •	•••	•	•		
Joiner and hardware	• •	•••	•	•••	•	•••	•	•••	•	•••	•	•	• •	• •	•	•	•		
Plasterer	•		••	•••	•	•••	•	• •		•••	•	•	• •	• •	•	•	•		
Plumber.	• • •	•••	•••	•	•	•••	•	•••	•	•••	•	•	• •	•	•	•	•		
Painter	• • •	••	••	•••	•	•••	• •	•	•	•••	•	• •	• •	•	•	•	•		
	• •	•	• •	• •	٠	•••	• •	• •	•	• •	•	• •							

Here, then, by this rule we find that if the material for the painter cost one-third of any given amount,



	ł		1								_	
Name of City.	Masons.	Brick- iayers.	Structu- ral Iron Setters.	Ornamen- tai Iron Setters.	Plasterers.	Lathers.	Holsting Engineers,	Tile Setters.	Plumbers.	Steam Fitters.	Steam Fitters'	Gas Fitte
Buffalo, N. Y Baltimore, Md Chicago, Ill Cleveland, O	45 55 60 50 to 55	50 55 60 50 to 55	50 43 <del>1</del> 56 <del>1</del> 47 <del>1</del> to 50	50 52 <del>]</del> 31 <del>]</del> to 43 <del>]</del>	50 50 56 56	\$2 p. M. 50 \$3 to 3.75	43 37 56 35	50 441 561 434	423 45 561 50	423 31 56 44	18	
Columbus, U	50	55	40		50	p. day. 45	30	55	40	35	15	271
Cincinnati, O	60	60	50	50	623	2.25 p.M.	371	50	3.50 p.d	41	25	431
Denver, Col	62 <b>}</b> to 68	621 to 681	50	2.50 to 3.50	55	31 to 41	371	55	53 <del>]</del>	53 <del>]</del>	29	53
Duluch, Minn Detroit, Mich Erie, Pa	45 53 45	60 53 50	35 35	35 30	56 <del>1</del> 50	45 43 <del>1</del>	30 30	35 50	55 44	50 44	25	55
Indianapolis, Ind. Jackson, Mich	45 to 60 50	45 to 60 50	40	40	45 40	21 to 21	35	55	30 to 35 44 33	30 to 35 45 to 50 38	20 25 16	30 to 3 30 25
Kansas City, Mo Minneapolis, Min.	50 to 62 <del>]</del> 50	50 to 62 <del>]</del> 55	50 40	50 40	62 <del>1</del> 561	per yard. 4.00 p.dy	50 42 <del>1</del>	56 <del>1</del> 50	50 50	50 50	21 25	50 50
Milwaukee, Wis New York, N. Y Newark, N. J Philadelphia, Pa.	50 56 <del>1</del> 57 <del>1</del> 60	50 65 57 <del>]</del> 60	40 56 <del>1</del> 50 50	40 56 <del>1</del> 40	40 681 571 50	30 50 55 43 <del>1</del>	35 62 <del>]</del> 40	50 62 <del>1</del> 50 45	43 <del>1</del> 56 <del>1</del> 45 40	37 + 56 - 50 37 +	33	37 + 56 + 45
Providence, R. I.	45	45			45	83 p. day	271 to 35	50	437	314	104	311
Portland, Ore	68 <del>1</del>	68 <del>1</del>	473		62 <del>]</del>		473	561	561	561	25	471
Pittsburg, Pa	50	60	50	45	52 <b>1</b>	40	371	55	50	50	25	50
St. Louis, Mo Scranton, Pa S. Francisco, Cai. St. Paul, Minn	60 33 <del>]</del> 75 45 to 50	65 50 75 50	55 45 50 35	55 46 <del>1</del> 40	75 371 561	62 <del>]</del> 37 <del>]</del> 50 47 <del>]</del>	<b>55</b> 25 	56 <del>1</del> 50 62 <del>1</del> 35 to 45	621 404 624 50	68 <del>1</del> 40 62 <del>1</del> 45	311 21	62 40 62 50
Toledo, O Topeka, Kan	55 45	55 to 621	40	40	50 50	45 3c. p. yd.	35	30 to 45 37 <del>1</del>	43 <del>1</del> 40	37 <del>]</del> 40	22	37 <del>1</del> 40
Tacoma, Wash Washington, D.C.	681 561	68 <del>1</del> 561	62 <del>]</del> 50	62 31	521 561	371	43 <del>1</del> 37 <del>1</del>	75 561	56 <del>1</del> 50	56+ 43+	20	561 431 to 5

TABLE SHOWING RATE OF WAGES PAID PER HOUR IN THE BUILDING TRADES, IN TWENTY-NINE

Nore.—New York, water proofers, 344c.; bluestone cutters, 55c.; boiler and pipe coverers, 50c.; house shorers, 344c.; ce Nore.—St. Louis, slate roofers, 624c.; foremen, 874c.; composition roofers, 50c.; granitoid finishers, 50c.; granitoid k

Steam Fitters'	Gas Fitters.	Carpenters.	Stone Cutters.	Marble Cutters.	Marble Setters.	Painters.	Sheet Metal Workers.	Electricians.	Laborers and Hod Carriers.	REMARKS.
15 28 25	40 <del>1</del> 40 56 <del>1</del> 43 <del>1</del> to 56	35 37 <del>1</del> 50 37 <del>1</del> to 40	50 40 <del>1</del> 50 50 to 561	50 37 <sup>1</sup> / <sub>3</sub> 31 <sup>1</sup> / <sub>2</sub> to 40	50 50 50	37 <del>1</del> 40 37 <del>1</del>	35 . 30 and up 50 37 <del>}</del>	374 40 564 404 p. hour	17 to 20 30 25 to 30	Slag Roofers, 25c. per hour. Planermen, 424c. per hour. Electrical Heipers, 81.50 to
15	371	35	50	31 to 50	31 <sup>1</sup> / <sub>4</sub> to 50	30	35	64 p. day. 35	281 to 31	Electrical Helpers, 15 to 20c.
25	431	371	50	\$1.50 to 2.00 per day.	\$1.50 to 2.00 Der day.	35	25	34‡	20 to 37±	per hour. Electricians asking increase in wages on Jan. 1, '04. Marble Setters, \$3.50 per
29 <b>}</b>	531	45	561	\$2.50 to 3.00 per day.	\$3.00 to 4.00	431	467	46#	31 to 37	Plumbers' Apprentices, \$1 to
25 20 25 16 <del>1</del>	55 44 30 to 35 30 25	37 <del>]</del> 35 25 to 30 30 to 37 <del>]</del> 28	50 56 <del>1</del> 40 45 50	35 40 55	35 40 55	35 32 <del>]</del> 25 35 26 to 28	37 32 25 to 30 32 to 40 22 to 30	311 to 371 35 30 35	25 25 17 to 25 31 22	83 per day. Plasterer's work, 4 hrs. Sat. Laborers, \$1.28 to 1.75 p.day.
21 <del>]</del> 25	50 50	40 37 <del>1</del>	62 <del>]</del> 50	43 <del>1</del> to 46 <del>1</del> 50	431 to 461 50	374 374	424 40and up	37 37	171 to 30 20 to 25	Plumbers' Laborers, <b>62.50</b> to 2.75 per day. Hoisting En- gineers, <b>84.00</b> for 9 hours
33	37 56 45	35 561 41	50 62 <del>1</del> 55	27 62 56	37 <del> </del> 62   56	35 50 35	35 50 45	50 50	25 to 30 25 to 40 <del>1</del> 30	work. Mason Laborers, 274c. p. hr. See note below.
30	421	40	50	50	50	371	40	40	35	Plumbers' Laborers, 35c, per
91	31‡	35	371	41	45	31‡	31	31‡	181 to 25	Tile Setters' Helpers, 25c. per
25	471	<b>#3 to 3.75</b> per day	62	621	62 <del>]</del>	\$3 to 3.50 per day.	471	371	\$3 to 3.50	nour.
25	50	43	50	50	50	421	423	50	35	Electrical Helpers, \$2 to \$4 per
	62) 40	55 30	56 <del>1</del> 40	331	561	45	50	62	421 to 45	See note below
25	62 <del>]</del> 50	50 30 to 37}	56 <del>1</del> 50	31 <del>1</del> 30	50 35	43 <del>1</del> 35	56 <del>1</del> 40	371 to 50 311	50 17 <del>1</del> to 25	Hoisting Engineers can work 59 hours per week for \$20.
22	37 <del>1</del> 40	30 to 35 30 to 37 <del>]</del>	50 45	20 to 30 45	20 to 30 45	31 <del>1</del> 35	30 to 45 35	50 30 to 371	25 to 28 18‡ to 21‡	Laborers work 10 hours. Lathers and Painters work 8 hours per day, all other
0	56 <del>1</del> 43‡ to 50	45 43 <del>1</del>	50 45	50 45	50 45	371 371	43 <del>1</del> 371 to 30	874 43	25 to 28 15 to 25	trades work 9 hours. Electrical Helpers, \$1 to \$2 per day.

ENTY-NINE CITIES OF THE UNITED STATES. COMPILED BY E. M. CRAIG, CHICAGO, ILL., JANUARY, 1904.

s, 34fc.; cement masons, 55c.; elevator constructors, 53fc.; floor layers, 57fc.; stone setters, 62fc. After May 1, 1904, 68fc. granitoid laborers, 35c.; granitoid frame setters, 40c.; granitoid block men, 40c.



the labor will cost two-thirds of the same amount to do the work; and the same method may be applied to other trades. The figures must be filled in to suit the current prices.

The average wages paid, at this writing, March, 1904, according to E. M. Craig, Secretary of the Building Contractors' Council, Chicago, Ill., in 29 leading cities in the United States, are given in the foregoing table. The rates given are in cents per hour, with a few exceptions, which are given in days of nine hours each.

This table will aid materially in determining the cost of work in and about the cities named.

In estimating by "rough quantities," the amount of materials and workmanship are first ascertained from the drawings and specifications in a broad and comprehensive manner, the work being concentrated as much as possible, and the whole dealt with as shown in the previous paragraphs as this method, which see.

Estimating by the square has been discussed before, but it may be briefly referred to again, as this method is quite common in some localities. This method is recommended by some authorities as being superior to cubing, as it gives a better idea as to the character of work and quality of materials, though, I must confess, I do not see where the advantages come in, for the expert "cuber" must take both those conditions into consideration when deciding on his "constant" for the cost per foot cubic of the building being estimated upon. In addition to what has been said on this method, I add the following: "The mode is to take the constructional shell only, pricing it at so much per square; walls, for instance, are taken according to their thickness and manner of finishing,

whether they be wood, brick or stone. This must include all excavating, concreting, plastering, painting and paperhanging. The floors must include all joists, bridging, ceilings and ornamentations of all kinds. The roofs include all that is required to complete them, as shown on plans and described in specifications, and are measured on the slope of the rafter; and all the other work, partitions, stairways, and everything in the building, must be treated in like manner, and all reduced to squares of 100 feet super. A price is then placed on each, and the whole added together. Such a system of superficial measurement certainly has some advantages, and should be fairly satisfactory, as it takes into account the materials and labor in a fairly exact manner and form. Of course, as before stated, a special list of prices must be compiled for each set of squares, the outside watls having one price per square, the floors another, and so on until the whole of the surfaces have been priced per square. It will be seen that care and discrimination are requisite for estimating by this method. or serious errors will occur.

As an example of this method of estimating I submit the following, which is for a balloon frame building put up in the usual manner, and for convenience a space in the building is taken for a basis of  $20 \times 20$  feet, making four squares. This basis may be taken for any portion of the work, i. e., walls, roofs, floors, etc. The studding employed is  $2 \times 4$  inch, sized on one side and one edge. The studding is placed I6 inches from centers and covered with dressed and matched boarding. Building paper is next laid on, and then first or second clear siding is used. Plates are included in the cost and are put on double thickness.

I 20

#### ANALYSIS OF OUTSIDE WALLS

19 pieces, 2 x 4 inch, 20 feet long-247 feet, at	
\$20 per M	\$ 4.94
466 feet dressed and matched fencing, at \$25.00	11.65
475 feet siding, at \$30	14.25
11 pounds nr	50
30 pounds p ents per pound	75
Framing and	
at \$8 per M	1.98
Laying 4 squares of flooring, at 50 cents per square	2.00
Laying 4 squares of siding, at \$1,124 per square	4 50
Laying 4 squares, at 12½ cents per square	.50
	L1 07

Dividing this sum by 4 gives the price of a single square, \$10.27.

The analysis of cost of four squares of roofing, the rafters being  $2 \times 4$  inch scantling, set 2 feet between centers, covered with dressed and matched fencing, and the best quality of cedar shingles laid  $4\frac{1}{2}$  inches to the weather, is as follows:

#### ANALYSIS OF ROOF WORK

12 scantlings, 2 x 4, 20 feet long-156 feet, at \$20	
per M	
466 matched (feet) boarding, at \$25 per M. 11 65	
31 M. shingles, at \$3 per M	
10 pounds nails, 3d	
14 pounds nails, 8d and 10d	
Framing and putting in place 156 feet 2 x 4 scant-	
ling, at \$8 per M 1 25	
4 squares of roof boarding, at 50 eents per square . 2.00	
4 squares of shingling, at \$1.25 per square 5.00	
Staging	
\$34.58	

This sum in turn, divided by 4, gives as the cost of a shingle square,  $$864\frac{1}{2}$ .

The following is an analysis of cost of four squares of flooring, laid on joists  $2 \times 8$  inches, the flooring being selected from No. 1 fencing, and the joists being placed 16 inches between centers. Allowance is made for doubling where necessary.

#### ANALYSIS OF FLOORING

17 joists, 2 x 8 inches, 20 feet long-459 feet, at	
\$20 per M	9.18
15 feet of 1 x 2 inch bridging at 2 agent.	3.98
10 pounds of 8d common nails.	.30
3 pounds spikes	.08
Framing 450 foot of joints at 50 cents per square	2.00
Bridging.	2.30
	. 50
\$2	8.64

Dividing this amount by 4, as in the previous cases, gives \$7.18½ as the cost of one square of flooring. It may be remarked in this connection that these figures are based upon present prices in Chicago.

The following is an analysis of the cost of an inside door, 2 feet 8 inches by 6 feet 10 inches, 13% inches thick, cased and finished complete except the one item of painting:

#### ANALYSIS OF COST OF DOOR

Frame, 2-set casing and stons	
18 feet of molding 21 inchos	\$2.00
1 threshold hardwood	.28
1 first quality door give a	. 15
21 incluments door, size as given above	1.95
og-men mortised lock, bronze face, bolts and strik-	
ing plate	62
Poreelain knobs, plated roses and esoutcheous	. 0.5
1 pair of 31 japan butts and some	.40
Setting frame	. 25
Casting up 2 sides	.25
Putting down the 1 by	. 40
Malling down threshold	.15
Molding I side	20
Fitting, hanging and trimming door	- 20
	. 10

\$7.41

The following is an analysis of cost of a four-light window, with sash  $14 \times 30$  inches, 136 inches thick, check-rail, the window set, cased and finished complete:

#### ANALYSIS OF COST OF WINDOW

Window frame prepared for weights	\$2.15
Sash glazed	2 10
20 feet 24-inch molding	2.10
25 feet inside case and window sill	.30
28 pounds of each moister	.75
Such comb	. 56
Sash cords	.18
Grounds for plastering and putting on	.30
Setting frame	.25
Casting up.	55
Fitting sash	15
Nails.	.10
Sash locks	.10
Putting on each looks	.25
a dreing on sash locks	. 10
-	
	34 74

This example gives the key to the method of estimating by the square, also how to estimate the cost of a door or window in place.

The prices given may not be correct for any other place but Chicago, and even then the prices may differ in each ward; so the estimator must in this, as in all other cases, be sure of his prices before closing his tender. I have known the prices for door and window frames vary as much as 30 per cent in factories not a quarter of a mile apart.

Later on I will give other examples of estimating by analysis.

### ESTIMATING BY UNITS OF ACCOMMODATION

This method of estimating does very well for certain descriptions of buildings, such as churches, schools, prisons, hospitals, asylums, stables, and buildings of a

similar kind, but apart from these it has no value, and its value in the cases mentioned is not by any means a fixed factor. The system is based on the known cost of buildings which give so much space to each scholar, patient, sitting, horse, or prisoner. Thus, if we know how much a stable costs that was built to accommodate 20 horses, it is a simple matter to estimate how much it cost per one lorse space; for if the building complete cost \$4,000, that would give the cost per horse at So, also, with schools. If we know of a \$200.00. school for 100 children that cost \$10,000, we know that each sitting cost \$100,00; therefore it is reasonable to suppose that other schools, everything being equal, will cost \$100 per sitting. It must not be forgotten, however, that conditions are not always the same, and while a "jumped" figure of this sort may be, and is approximate, it is not always correct, for no two buildings, even though they are side by side and built concurrently, can possibly be built at the same actual cost. I have seen the attempt made on several occasions, and the variations amounted to from 3 to 71/2 per cent; quite a large amount if taken from the 10 per cent profits of the work.

On occasions when time will not admit of even a sketch of the proposal being made, this method affords oftentimes the only ready means of ascertaining the approximate cost. Similarly, for certain minor accessories, when the cost of materials and construction varies but slightly for units of the same class, as in a range of latrines, etc., the approximate cost can be easily determined in this manner. In order to give the reader some basis to work on, I submit a few examples of price for units, which are as near as possible average ones for the whole of the

United States and Canada, and while they may not be correct, they may be depended upon as being approximate.

Cost of each room in tenements.	from	\$250	to \$150
Cost of each room in cottages		900	(C 980
Cost of each room in residences.		320	4 .190
Cost of each room in villas, etc.		450	4 700
Cost per patient in asylums		1 100	1 650
Cost for each soldier in barracks.	66	750	1,000
Cost of churches, plain, per sitting	- 44	45	·· 60
Cost of churches, ornamental, per sit-		30	00
ting	**	68	" 134
Cost of first-class stables per cow	44	175	" 195
Cost of first-class stables, per horse	"	200	( 225
Cost of second-class stables, per cow	"	120	" 135
Cost of second-class stables, per horse.	**	150	4 165
Cost of third-class stables, per cow	"	75	4 95
Cost of hospitals, complete, including			00
all offices, buildings, etc., per bed	"	1.500 '	(2.200
Cost of cottage hospitals, per bed	66	1.000 4	1.200
Cost of general hospitals, per bed	"	500 '	6 750
Cost of isolated hospitals, including			100
all necessary offices, buildings, and			
other conveniences, per bed	44	1.750 "	2.250
Cost of buildings put up in a hurry for			-,
temporary occupation, per l. ad.	~	90 "	100
Cost of latrines for barracks, per seat.	**	75 "	100
Cost of city and town lodging houses,			
per bed	46	275 "	375
Cost of music halls for cities and towns,			
per head	44	75 "	125
Cost of music halls for small towns,			
per head	4	35 "	75
Cost of schools, complete, large cities,			
per scholar	"	60 "	100
Lost of schools in small towns and vil-			
lages, per scholar	44	42 "	62
scholar schools in country places, per			
Schools infant and a	66	35 "	45
ochoois, mant schools, per scholar	44	25 "	35

Cost of theaters, complete, large cities,

These examples are given for brick buildings of good style. If the buildings are of stone, from 10 to 20 per cent must be added, according to the quality of the stone and amount of ornamentation. There are theaters in New York, Chicago, Philadelphia, and other large cities, that cost per seat 50 per cent more than I have placed in the foregoing list, but these are exceptions to the rule.

If the buildings are of wood, that is, frame buildings, then a deduction of from 10 to 15 per cent may be made from the figures given, which will make the figures approximately correct. Theaters or other buildings, built of bricks and stone, or of bricks, stone or terra cotta, cost more than buildings built exclusively of bricks, and provisions must be made for extra cost whenever this condition exists, and much is necessarily left to the judgment of the estimator in determining the extra assessment.

#### ESTIMATING BY CUBING

This method, while far from being exact, is, in my opinion, a more correct method than either of the others presented. At the same time the expert estimator will frequently change his constants to suit varying conditions.

The following list of the cost per cubic foot, of buildings named, which was prepared by Mr. Kidder several years ago, and published in *The American Architect*, may be of some assistance to those who desire to know the cost of similar buildings. I may say, however, that it would be safe to add at this time at least 10 per cent on the bulk, as the prices of labor and

material have advanced sufficiently to warrant that addition during the post five years.

I have add d to Mr. Kieder's list a few others, but as I have been mable to get the most prominent buildings that have been crected within the last few years, this table is not complete up to date, so far, at least, as the cost per cubic foot of the more recent buildings is not included.

TABLE	SHOWING	DATE	OF	CONSTRUCTION	AND	COST	PER
			CUB	IC FOOT			

Date		Cuble	Cost per
		contents	euble foot
1879	Central Musie Hall Raudolph and		· · · · · · · · · · · · · · · · · · ·
	State Sts.	1 949 000	1
1881	Borden Block, Offices Randolph and	1,248,000	14.4 cts
	Dearborn Sts.	810.000	110 11
1881	Brunswick & Balke Fact'y Superior St	1 210 000	14.9
-1882	Brunswick & Balke Fact'y, Superior St	1,219,200	5.4 "
1882	L. Rosenfeld Stores and Elata Weak	305,000	6.2 "
	ington and Halstod Sta	005 450	
1882	Hammond Library Ashland And	885,456	10.7 "
1883	Wright & Lowthen Oil will D-u G	183,300	19.0 "
1883	R Knisely Stores and Flats M. E.	520,000	6.8 "
1884	A Knisely, Biores and Flats, Madison St	138,320	11.2 "
1881	I W Saovillo Fustory, West Monroe St.	1,412,640	6.1 "
1885	Zion Tomple Structure, Desplaines St.	697,000	6.4 "
1887	Auditorium Dailli	478,400	7.9 "
1887	Standard Child Ing, Congress St.	9,128,744	36 0 "
1001	A Logl & D. House, Michigan Ave	916,917	12.9 "
1000	Loeb & Bro., Warehouse, Michigan St.	123,300	12.9 "
1009	Jewish Training School, Judd St.	447.854	10.0 "
1091	A. Loeb & Bro., Flats, Randolph and		
1001	Elizabeth Sta.	499.531	10.4 "
1991	Meyer Building, Store, Franklin and		
1001	Van Buren Sts.	2.099 700	96 "
1881	. W. Oakley, Warehouse, La Salle and	-,000,100	5.0
	Miehigan Sts.	1 300 313	60 (
1891 8	Schiller Building, <sup>1</sup> Randolph St.	2 133 140	20.9
1893 S	stock Exchange Building. <sup>2</sup> La Salle and	2,100,440	30.8
1	Washington Sts.	3 102 500	22.0 4
		0,490,000	33.2

NOTE: <sup>1</sup> Sometimes called the German Theatre, 17 stories, skeleton construction, faced with terra-cotta. Rich marble work. Theatre occupies about 4 stories. Offices above. <sup>2</sup> 13 stories, flat roof, skeleton construction, rich terra-cotta facing.

1996	The Real-any Ruikling Chicago III Durn	Cost cubic	t per e foot
1000	ham & Root Architecte	20	<b>at</b> a
	Monadnock Building Chiango Burnham &	ڪ ن	cts.
	Root and Hollahird & Roche Architects	491	66
	Rialto Building Chicago Burnham & Root		-
	Architects	97	"
	Masonic Temple, Chicago, Burnham & Root		
	Architects	58	"
	Chamber of Commerce Building, Boston Mass	- 29	66
	New England Life Insurance Building, Boston,	-0	
	Mass	60	6.6
	The Hemmenway Building, Boston, Mass	43	"
	Ten Story Office Building, New York City.	60	66
	Board of Trade Building, Montreal	20	66
	Ten Story Office Building, New York City	50	"
	Seven Story Office Building, New York City,	37	"
	Six Story Office Building, New York City	26	44
	A similar building, one front	24	"
	Two Four Story Office Buildings, one front,		
	New York City	47	"
	Herald Building, New York City	46	66
	Chamber of Commerce, Umeinnati	26	66
	Wainwright Building, St. Louis, Mo	243	46
	Union Trust Building, St. Louis, Mo	273	66
	Equitable Life Insurance Building, Denver,		
	Colo	42	66
	Ernst & Cramer Building, Denver, Colo	17	"
	Masonic Temple, Denver, Colo	19	"
	Crocker Building, San Francisco, Cal	63	66
	Endicott Building, St. Paul, Minn	-29	66
	Four Story Office Building, Rhode Island	38	66
	Three Story Office Building, Connecticut	50	66
	Three Story Block, Denver, Colo	81	6.6
	Fourteen Story Hotel, New York City	44	**
	Brown-Palace Hotel, Denver, Colo	30	" "
	Denver Athletie Club Building, Denver, Colo.	18	4 6
	Denver Chib Building, Denver, Colo	24	6.6
	Public Library, New London, Conn	$-36\frac{1}{2}$	66
	Howard Memorial Library, New Orleans	44	66
	Public Library, Toronto, Ont	22	66

Fire-Proof Hospital Building, New York	40	cts.
Six Story Hospital Building, New York	32	"
Hill Theelogical Seminary, St. Paul, Minn	11	"
Wingate 1, ill, Str. 9 College, Owno, Me	10	"
Grammar School Building, Denver, Colo	91	66
Grace M. E. Church, Cambridgeport, Mass.	81	"
Christ M. E. Church, Denver, Colo.	20	44
City Dwellings (of brick) in Chicago 17 to	$\frac{-0}{20}$	"
City Dwellings (of wood), Eastern towns.	11	"
First-class Stone Homes in Denver, Colo	27	"
Brick Houses, Modern Improvements.	14	"
Cheap Brick Houses, 8-roomed, about	10	"
Cheap Wooden Houses, 8-roomed, about	71	"
"Veneered" Houses, Two-story	- * 2 - S	"
Rough-cast Cottages, First Class	63	"
Rough-cast Cottages, Second Class	53	"
Rough Wooden Sheds, Barns, Stables, etc. 34 to	$5_{4}$	"

From the foregoing table the average cost of buildings of any description may be approximately determined. The highest figures shown are those for the Crocker building of San Francisco, Cal., the cost per cubic foot being 63 cents; the lowest amounts given being for rough wooden sheds, barns, etc., which are put down at from  $3\frac{1}{2}$  to 5 cents per cubic foot. These last figures seem a little large for the kind of work mentioned, but they are handed me by a builder who has had a large experience in these kinds of buildings.

While the foregoing deals altogether with the cubic foot, the same principle may be applied to yards or perches or any other fixed dimensions, and as an example I give herewith a table of miscellaneous matters that will be found very useful when estimating:

TABLE SHOWING PRICES OF WORK OF VARIOUS KINDS Spruce lumber per M. in place on roof

	or noor ,	 125 00
H.	P. per M. matched, nailed and	 240.00
	finished on roof or floor	35.00

12ĝ

H. P. per M. matched rafters and					
joists finished on roof or floor			9	30.00	
Slate roof, no boarding, per square.	from	\$7.25	to	12.50	
Slag and gravel roof, no hoarding	"	5.00		7.00	
Composition roof, no hoarding per		0.00		1.00	
square	"	2.00		5.00	
Wood shingle roof, no boarding, per		2 00		0.00	
square	6.6	3.25		5 20	
Tin roof, with boards, per square	"	9.75	"	13.00	
Corrugated iron roof, no boarding,		0.10		10.00	
per square	"	7.20	"	10.00	
Steel stamped shingles, no boarding,				10.00	
per square	"	4 50	æ	6.00	
Common brick work, per cubic foot	"	.28	"	.38	
Public r asonry, per cubic yard	"	4.00	"	7.50	
Concrete, per cubic yard	"	5.50	"	8 00	
Cut stone pier caps, per cubic foot.	"	1 75	"	2.25	
Piles driven in place, per lin. foot	"	.25	"	.30	
Earth excavation, per cubic yard	66	.50	"	.52	
Steel truss and column frame in place			48	le. per	lb
Steel beams in place and secured in pl	aee.		3	e. per	lb
Plain castings in Sit			2]	e. per	lb
Corrugated iron No. 22 gauge, in place	e, per	super	foo	t .073	
Galvanized iron flashings, per square	foot.				
Door frame and doors, finished, per s	quare	foot.		52	
Window frames and windows, per so	uare	foot.		54	
Sash, glazed and painted, per square					
tootfi	rom §	\$0.16 t	0 \$	0.23	
Gutter and conductor pipes, per lin.					
NV	"	.25	66	.30	
wood stairs, 3 feet wide, straight,					
per step	"	<b>3</b> .00 °	16	3.25	
from stairs, 3 feet wide, straight, per					
Steel glutters and	**	7.00	" 1	0.00	
Louvros, fixed par square toot.		.50 '	•	.55	
Louvres, inxea, per square foot		.45	6	.55	
Sheet iron doors and shutter	••	.70 '	•	.80	
square foot	"	05 (	,		
Skylights Joingh gloss nor agree fi		.35 '	,	.45	
	••	.20 '	•	.30	

Skylights, white glass, per square ft. from \$0.18 to \$0.20Pipe railings, per foot in length..45 ".55Ventilators, round, per foot in length..450 "10.50Metal cornice, per lineal foot.12 ".30

It may be useful to my readers to know in a general way the cost per cubic foot of a few buildings other than those already given, and to this end the following are presented:

Public abattoirs, brick, per cubic foot	.fron	\$0.14	to	\$0.16
Small cottages, brick, per cubic foot.		13	"	¢0.10
Country court houses, brick, per cubic		.10		.14
foot	"	22	"	20
Lunatic asylums, including wards, etc.		• 14		.30
per cubic foot	" "	16	"	25
Farm barns, wood, per cubic foot.	"	.10	"	- 40
Farm barns, brick, per cubic foot	"	.04		.00
Armorics, wood, per cubic foot.	"	00	"	08
Armories, brick, per cubic foot	•••	.09	"	.11
Armorics, stone, per cubic foot	"	.11	"	.14
Public baths, complete, wood per		.10		-20
cubic foot	"	14	"	1
Public baths, complete, brick, per cu-		14		.17
bic foot	44	10 (	e l	00
Public billiard rooms, wood, per cubic		.10	-	.20
foot	12	10		0.0
Public billiard rooms brick par oubio		.10	•	.20
foot	"	10 (		
Brewerics, including all pocosso my mo		.19 .		.24
chinery, tubs cellarage copport				
cooler, numps, etc.				
Wood per cubic foot	"	10 (	,	
Brick per cubic foot		.12 "		.16
Stone per cubic foot		.14 "		.18
Single span bridges brief or stars		.15 "		.19
per foot super				
Double or more summer batters		5.00 "	1	5.00
per foot super				
f in granite, par foot super		15.00 "	30	0.00
Branne, per toot super	5.5	32.00 "	50	).00

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73 1 .			
Bungalows and summer cottages,			
wood per cubic foot	from	\$0.12 to	\$0.16
Bungalows and summer cottages,			
brick, per cubic foot	" "	.17 "	.19
Plain country churches, wood, per			
cubic foot	66	.09 ''	.12
Plain country churches, brick, per			
cubic foot	4.5	.12 "	.15
Plain country churches, stone, per			
cubic foot	"	.14 "	.17
Churches for cities, stone, per cubic			
foot	66	.21 "	.40
Coach houses, brick, per cubic foot	66	.10 "	.12
Colleges, first class, complete, brick,			
per cubic foot	6.6	.20 "	.28
Colleges, first class, complete, stone,			
per cubic foot	"	.25 ''	.35
Colleges, second class, complete, brick,			
per cubie foot	"	.18 "	.22
Underground conveniences, complete,			
per cubic foot	"	.80 "	1.40
Stable for cows, wood, per cubie foot.	"	.08 "	.12
Stable for cows, brick, per cubic foot.	"	.13 "	.15
Stable for horses, wood, per cubic foot.	"	.10 "	.13
Stable for horses, brick, per cubic foot.	"	·.14 "	.17
Power plant station, brick, per eubic			
foot	66	.14 "	.18
Fire engine house, brick, per cubic ft.	44	.14 "	.17
Residential flats, brick, per cubic foot.	"	.28 ''	.36
Blacksmith shop, brick, per cubic foot	"	.10 "	.13
Cost of heating, including hot water,			
boiler, pipes, radiators, valves,			
etc., complete for each 1,000 feet			
of cubie contents—			
Churches			6.00
Hospitals, and similar building	gs	1	6.00
Factories and mills		1	0.00
Dwellings, clubs, etc		2	1.00

These amounts include everything in connection with the heating except the boiler house.

Cost per cubic foot of houses built in			
good style of pressed brick fac-	•		
ings, or fine stone, well finished			
in hardwood, oak, or birch	from	\$0.30 to	\$0.41
Brick buildings, of less pretensions.		¢0.00 t0	20.41
per cubic foot	"	.27 "	38
Brick, third class, per cubic foot	"	.20 "	30
Brick, fourth class, per cubic foot	"	.15 "	.00
Brick, fifth class, per cubic foot	**	.12 "	21
Libraries, complete in brick, per cubic			.21
foot	"	.17 "	.25
Libraries, complete in stone, per cu-			.20
bic foot	"	.19 "	.30
Mortuary chapels, complete, per cubic		-	
foot	**	.25 "	.33
Museums and similar buildings, per			.00
cubic foot	66	.23 "	.34
Opera houses, first class, per cubic ft.	"	.30 "	.40
Opera houses, second class, per cu. ft.	"	.25 "	.35
Opera houses, third class, per cubic ft.	66	.22 "	.32
Opera houses, fourth class, per cu. ft.	"	.20 "	.28
Prisons, complete, including padded			
cells, per cubic foot	"	.18 "	.20
Cost per cubic foot for tearing down		·	
old brick buildings, including.			
walls, chunneys, partitions, tak-			
ing up floors, and removing win-			
dow and door frames, sashes,			
doors and finishings, moving			
away debris, cleaning site and			
old materials and stacking up			
The whole sub-			
huilding to be measured for			
bottom of footings to helf man			
up roof, per cubic foot	"	01 ((	011
Frame skating rinks, per cubic foot		00 (	.012
Brick skating rinks, per cubic foot	**	10.4	.12
Riding schools, with track, per ou ft	"	12 4	13
Sheds, rough, in wood, per cubic foot	"	05 "	.19
Sheds, rough, in brick, per cubic foot	"	08 "	10
		.00	.10

Sheds, rough, in iron per cubic foot.f	rom	\$0.09	to	\$0.12
Stores, dry goods, wood, per cubie foot	4.6	.13	"	.15
Stores, dry goods, brick, per eubic foot	16	.15	٢	.17
Stores, dry goods, first-class finish,				
brick, per cubie foot	64	.20	"	.28
Stores, dry goods, second-class finish,				
brick, per cubic foot	6.6	.18	" "	.24
Stores, dry goods, third-class finish,				
briek, per cubic foot	"	.16	"	.20
Stores, groceries, wood, good finish,				
per cubic foot	"	.14	"	.16
Stores, groceries, brick, fine finish,				
per cubic foot	66	.16	"	.18
Stores, groceries, brick, first-class fin-				
ish, per cubic foot	"	.18	66	.22
Country or town halls, in brick or				
stone, well finished, classic style,				
with all necessary appointments				
and fittings, marble wainscot and				
other corresponding finish inside				
and out, per cubic foot	66	.32	"	.40
For country, per cubie foot	"	.30	"	.38
For cities, per cubie foot	66	.36	"	.42
For states, per cubic foot	"	.45	"	.55
For states, with towers, per eubic foot.	66	.46	"	.57
Water towers, brick, per eubic foot.	"	.16	"	.20
Water towers, iron, per cubic foot	**	.17	"	.20
Water towers, stone, per cubie foot	"	.S. +	46	.22
Model cottages, stone dressing, briek,				
per cubic loot	••	.13	**	.16
model cottages, stone dressing, sec-		10	.,	
ond etass, per cubic foot		.12		.14
City flats, briek, per cubic foot	"	.28	"	.30
City flats, stone, per eubic foot	66	.30	"	.32
City flats, stone and brick, per cubie ft	66	.29	"	.31
Street arehes for gala days, if of rough				
wood, covered with bunting,				
mottoes, evergreens, and similar				
materials, and are only tempo-				
rary, per cubic foot	66	.04	6	.08

Better-class arches, plastered, etc.,				
per cubic footf	rom	\$0.07	to	¢0 19
If made with staff and moulded, and		40.01	•••	¢0.12
have statuary, per cubic foot.	"	10		95
Permanent arches, in stone, per cu. ft.	66	.10	"	1.00
Permanent arches, first class, in mar-		.00		1.00
ble, per cubic foot	66	1.25	**	3.00
City parks-exclusive of land-walks				0.00
drives, lakes, buildings roads				
gates, walls, rustic bridges, and				
other things in connection with				
well-appointed parks, per acre-				
First class		22	00	0 00
Second class			,00	0.00
Third elass			,00	0.00
Fourth class		1	,00 00	0.00
Fifth class.		· · · · · ·	,00 60	0.00
Parks in country towns, or large villag	 109 m	horo	00	0.00
exhibition buildings, offices, and	co n l etc	hlog		
are kept, in conjunction with a rac	e-eoi	IFSO		
and the area not less than twenty-fi	ve a	nree,		
the total cost of artificial work, i	nelu	ding		
rough buildings, should not be me	ore fl	ian		
per acre		\$	57	5.00
Cost of exhibition buildings, of wood		· · · · · ·	011	
First class, per cubic foot	m s	0.00.4		0.11
Secon class, per cubic foot	66 (G	0.00 10		0.11
Third class, per cubic foot	"	05 "		.03
Fourth class, per cubic foot	"	01 "		.07
Fifth elass, per eubie foot		.03 "		.00
Exhibition buildings for pigeons				.00
cows, horses, sheep, poultry, etc.				
First class, wood, per en, ft fro	m 📽	1.08 to		10
Second class, wood, per cu.ft	6	07 "	ø	.10
Third class, wood, per eu. ft.	6	.06 "		09
Fourth class, wood, per cu.ft.	6	.05 "		07
Fifth elass, wood, per eu. ft.	6	.03 "		.05

These items cover most of the ground for cubing, and are taken from the best authorities on the subject
and from actual experience, and are quite sufficient for the ordinary purposes of the estimator who is likely to purchase this book.

As I have stated before, the cube rate cannot be relied upon for work of exceptional elaboration. The cubes generally published are intended to apply chiefly to buildings of a plain character in their several classes, and it would be of value if this circumstance were taken into account in fixing upon the rate. Precision can, however, only be attained by a generalization from extensive experience. The rates must be taken as general guides in forming an estimate of cost, and in all cases the experience of the expert estimator can alone give value to the system. There can be no comparison between a large block of stores and an elaboratel "ted up hotel. The one is comparatively simple to ... other; the decoration to the hotels in an avenue would alone increase the cost per cubic foot. The materials may be the same, brick or stone, with the same kinds of materials for finish, but the cost of labor, sizes of rooms, difference in walls, in heating, in plumbing, etc., would make a vast difference in the cost per foot, as an authority says on this subject: "I think the probabilities are that the cubing of a building 100 feet high would be higher than that of a building 50 feet high. It altogether must depend upon whether the larger building and the higher building has rooms of nearly the same size as the smaller building. No doubt the higher building would require thicker walls, but immediately you get away from comparatively small rooms into very large cubic spaces, then the difference in price is not great."

In fewer words we may say that the cost per foot cube of a building depends mainly upon the divisional

internal walls and floors; the more numerous the rooms into which the space is divided, the greater the cost. Height is certainly a factor of cost, as a high building requires thicker walls; scaffolding and labor become expensive. But if we take two buildings, one twice the superficial area of the other, but of the same height, the difference per foot would entirely depend on the interior division and elaboration of plan. But to say that the cubing of a bigger and higher building is pro rata higher than for a smaller and lower one is a proposition that does not always hold It is so only when the rooms are about the same dimensions in both cases. It would, for instance, be absurd to cube a large public halt with the usual rooms at a higher ratio than a small villa residence, because it was larger or higher. In plain English, the greater internal space and vacuities the less charge must be placed on the cube foot.

With regard to ornamental façades of wrought stone, a considerable addition per foot must be made upon the cost of a plain brick front. To cube both at the same figure would be wrong.

It may be asked, then, would any successful builder take a contract on the figures derived from cubing? We may answer that half the estimates now made by architects, in their private and public capacities, are made by cubing, and that contractors are to be found who would willingly take the risk of carrying out work in that manner. The two most perilous rocks upon which the cuber comes to grief are those of taking a figure without the verification of experience, and not making any allowance for internal elaboration of plan and decoration.

### ESTIMATING BY DETAIL QUANTITIES

We now come to the only method on which the small contractor can depend, and which is always reliable if the estimator only does his duty properly and refrains from "jumping" at the prices, a trick many estimators employ to evade a little work in figuring.

I have given, in the first pages of this work, a detailed method of estimating for excavating, ditching, rough walling, concreting, and other like matters, to which the reader is referred when he is called upon to estimate on such work, so I will now make a departure and reproduce a system, corrected and brought up to date, which I published in *The Builder and Woodworker* of New York, in February, 1879, and which, in my opinion, has never been improved. The system was quite popular and many thousand copies of it have been sold. Insurance appraisers and others have made it a "text-book" to some extent, and used it with the adjustment of prices, of course, to suit the time and locality.

The list of items given in former pages must be followed, but there will be many others that will crop up which the estimator must provide for when preparing his tender, and these he should make a note of for future reference. It would be well to copy the items I have given in a good-sized book, leaving a generous margin for any remarks or notes it may be necessary to make, and new items should be entered as they appear.

We will suppose the building to be figured on is to be a balloon frame: the total cost of it can be closely calculated when t + price of material and wages per day or hour are known.

First, mark on the plan, in plain figures, all the

dimensions and measurements in the building on which you are to estimate. Next, get the lineal measurement of all the sills, and from their size estimate the number of feet, board measure. Retain the lineal measurement, as from that the labor amount is estimated. The labor on the sills may be summed up to three kinds: First, framing without gains for joists or mortises, for studding as in common building when the studding is spiked to the sills and the joists rest on their top. Second, with mortises for studding, gains for joists, or studding without mortises. Third, with both mortises and gains.

Sills,  $6 \times 8$ , framed and placed in the building by the first, second and third processes, will cost for labor about 3, 5 and 7 cents per lineal foot. Sills,  $12 \times 16$ , double above prices. The intermediate sizes can be approximated from the above figures.

Joists are ordinarily placed 16 inches from center to center, and when so placed the number of joists on a given floor can be found by taking  $\frac{3}{4}$  of the length of the building and adding one joist where they are placed on top of the sill, and deducting one where the end sills are used in place of joist. First floor joists usually are  $2 \times 8$  to  $2 \times 14$ . Second floor  $2 \times 8$  to  $2 \times 12$ . Ceiling joists, where no floor rests thereon, are  $2 \times 6$ to  $2 \times 8$ .

Two men will frame and place in a wood building, not exceeding three stories, 600 lineal feet of joists, in size from  $2 \times 6$  to  $2 \times 14$  stuff, in one day of 8 hours.

In brick buildings not exceeding three stories, including anchoring and leveling up, 400 feet. Fourth story work, 350, and fifth story, 275 lineal feet.

The cost per lineal foot can be had from the above figures.

When joists are doubled under chimneys or partitions, the number of joists so used must be added to the result above named.

In balloon frames no braces are used. In timber frames they are made as follows:

1st. Cut off plain, spiked in, or "flat foot."

2d. With short tenons, and 3d, with long tenons and pinned. Braces vary in size from  $4 \times 4$  to  $6 \times 6$ . The cost of labor will not vary on account of difference in size. The first pieces will cost 2 cents, the second  $3\frac{1}{2}$  cents, and the third  $4\frac{1}{2}$  cents per lineal foot, framed and placed in the building.

The plates in a balloon frame are made of scantling of the same size as the studding, and are worth to get out and spike to the frame  $1\frac{1}{2}$  cents per lineal foot.

In timber frames the labor on plates is: (1) framing without braces or gains for rafters; (2) framing with braces and no gains for rafters; (3) framing with both braces and gains. An average price for labor on plates in sizes from  $4 \times 6$  to  $6 \times 10$  would be: first process,  $2\frac{1}{2}$  cents; second process, 5 cents; third process, 7 cents per lineal foot. From  $8 \times 12$  to  $12 \times 16$ , respectively, 4, 6 and 9 cents per lineal foot. This includes placing them in the building. Plates laid on walls are worth the same as plates spiked on the joists.

Posts in balloon frames are merely double-studding. The cost of placing them in position is the same as for studding.

Posts for timber frames are framed, first, with tenon top and bottom; second, the same, with one set of braces with girth or beam mortises; and third, the same, with two sets of girth or beam mortises.

By the first process posts from  $4 \times 6$  to  $8 \times 10$  would cost 4 ents. Second process, 6, and the third process, 9 cents per lineal foot to frame and place in the building.

Studding for balloon frames is usually placed 16 inches from center to center. They vary in size from  $2 \times 4$  to  $2 \times 6$ . Occasionally odd sizes are used, as  $2\frac{1}{2} \times 4$ ,  $2 \times 5$ , or  $3 \times 4$ . In an ordinary size frame building two men will lay out and raise 800 lineal feet of  $2 \times 4$  studding per day, or 750 feet of  $2 \times 6$ .

At \$3 per day, the first would cost 77 cents per 100 lineal feet. The latter, 86 cents. The labor of spiking of joists and plates being considered under their respective heads, the work on studding is simply confined to tenoning and studding on end, or spiking them to the sills.

A short rule for getting the number of pieces of outside studding, including plates, and allowing for doubling at all corners, and for windows and doors, is simply had by allowing one piece of studding for every foot of outside measurement.

This rule for buildings having many angles, where studding must be doubled, approximates very closely to the true result. In smaller buildings, without any angles, it will somewhat overrun.

The exact number of pieces of studding on the outside of building may be found by taking three-fourths of the number of feet in the outside measurement of the building; add one stud for each corner and angle, and one for each door and window. To this add for plate and gable studding.

Three-fourths of the number of lineal feet of all partitions will give the number of pieces required.

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Their length, of course, depends upon the height of the rooms.

The cost of labor is the same as for outside studding.

It frequently happens that the studding is not double for doors and windows, and occasionally the extra stud for the corners in omitted.

Ribs for studding are usually made from 1 to  $1\frac{1}{2}$  inch stuff, and will cost to lay out and nail to the studding about 1 cent per lineal foot. The purpose for these is to support the upper joist.

Three-fourths of the width of the building, less one, gives the number of pieces required for gable; the average length of each piece is the distance from the plate to the ridge of the roof, or what is termed the rise of the rafter.

Rafters are designated as main or principal rafters, hip, jack, and valley rafters, and plain rafters.

The long rafters of a hip roof are called the main or principal rafters.

The shorter ones are called jack rafters.

A plain rafter is the ordinary rafter used in straight gable roofs.

The projection of a rafter is the distance it extends beyond the plate, or the length of the look-outs.

The *rise* of a rafter is the height on a perpendicular line from the plate to the ridge of the roof.

The gain of a rafter is the difference between the run and its length.

The run of a rafter is the distance from the outer edge of the plate to a point immediately under the ridge of the roof, or one-half the width of the building.

For a common rafter, to the square of the rise, add

the square of the *run*. The square root of their sum is the length of the rafter from the outer edge of the plate to the ridge of the roof.

The *rise* of a rafter is found by multiplying the number of inches rise required by the run by one-half the width of the building.

The *rise* in one-quarter pitch is one-quarter the width of the building. In a one-third pitch, one-third the width of the building. In a one-half pitch, one-half the width of the building, etc.

A common rafter can also be found as follows: If the roof is one-quarter pitch, to the square of onequarter of the width of the building ad' the square of one-half the width of the building. The square root of the sum will be the length of rafter required. If a roof is one-third pitch square, one-third of the width of the building. If one-half pitch square, one-half the width, etc., and then proceed with the balance of the rule.

Required the length of rafters for a building 24 feet wide, gable roof, and one-quarter pitch.

One-fourth of 24 equals 6;  $\frac{1}{2}$  of 24 is 12. Squaring both gives 36 and 144, or 180; the square root of which is 13.416 feet, or length of rafter required.

Rule for estimating the length of rafters for hip roofs where they are of equal lengths:

Get the length of the main rafter by using the rule for common rafters. Then divide the length of the main rafter into one more space than the number of rafters required. The length of the space is the length of the shortest jack rafter, and the length of each studding rafter is simply the space added to the length of the preceding one.

Example.-Main rafter, 24 feet. Number of jack

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rafters required, 7. Hence the number of *spaces* would be 7+1, or 8. Dividing 24 by 8 gives 3 feet as the length of the shortest rafter. The next would be 6 feet, then 9 feet, 12 feet, 15 feet, 18 feet, 21 feet, and then comes 24, or the main rafter.

*Common* rafters on shingle roof are placed from 16 to 24 inches from center to center, according to the length and weight of roof required; generally 2 feet is the distance.

The number of rafters in a plain gable roof is found by dividing the length of the building by the distance the rafters are apart from center to center, to which add 1; the result is the number of *pairs* of rafters.

Cost of Framing Rafters.—Two men in one day will frame and place in the building 600 lineal feet of  $2 \times 4$  or  $2 \times 6$  rafters—roof, plain gable.

In a hip roof, including framing for deck, if any, 250 feet is a fair day's work.

The former would cost 75 cents per 100 lineal feet, and the latter \$1.75 per 100 lineal feet.

The contract price for framing one and a half, two, and two and a half story houses, in many of the Western states, averages 85 cents per 100 lineal feet of *all* the bill timber.

In all the framing labor thus considered, reference is had to soft wood only. If hard wood is used a fair addition to the prices would be 30 per cent.

If any of the work is circular, segment or octagonal, an addition must also be made, varying from two to four times the prices herein charged.

Lookouts for Hip Roofs.—An average length would be 20 inches. These are made of inch stuff and nailed to the rafters. They are worth, to get out, furnish material and place in position, 22 cents each.

The siding to a building is either drop siding, lap siding, dressed barn boards, or rough barn boards.

The number of feet of drop or lap siding is found by multiplying the outside measurement of the building by the height of the posts, to which add for gables, if roof is a gable roof, the product of the width of the building by the height from the plate to the ridge of the roof. This gives the number of surface feet, to which add one-fifth for lapping, and you have the number of feet board measure.

Two men will put on 700 feet in one day of drop siding when the window-casings and corner-boards are placed over the siding. Where joints are made against casings and corner-boards, 400 to 500 feet is a day's work.

Of lap siding, 650 feet. This includes putting up staging. Making the prices per square: Drop siding by the first method, 80 cents; second method, \$1.20 to \$1.50. Lap siding, 95 cents.

Two men will put on 2,000 feet of rough barn boards, or 1,500 feet of surfaced barn boards in one day, and will put on 2,000 feet of dressed battens, or 3,000 of rough battens. Hence the price would be: rough barn boards, 30 cents per 100 feet or one square; surface barn boards, 35 cents per 100 feet or one square. Dressed battens, 30 cents per 100 lineal feet. Rough battens, 18 cents per 100 lineal feet.

**Roofs**.—The area of a plain gable roof is had by multiplying the entire length of the rafters by the length of the building, including the projection of the cornice This gives one side; doubling it gives the total square feet of roof.

Hip Roofs.—Get the entire outside measurement of the building, including the projections of the cornice.

Multiply this by the length of the principal raftet and take one-half; the result is the area of the roof.

**Hip Roof with Deck.**—To the outside measurement of the deck, add the outside measurement of the building as above. Multiply this by the length of the principal rafter, and take one-half for the area of the roof.

Roof boards for plain gable roofs are worth 40 cents per square to put on the building, and for hip roofs 60 cents per square.

If roof boards are matched stuff for tin or slate roof, charge \$1.00 pcr square for gable and \$1.25 per square for hip roofs.

**Shingles.**—The average width of a shingle is 4 inches. Hence when shingles are laid 4 inches to the weather, each shingle averages 16 square inches; and 900 are required for a square of roofing.

If  $4\frac{1}{2}$  inches to one another, 800 will cover a square. If 5 inches to one another, 720 will cover a square. If  $5\frac{1}{2}$  inches to one another, 655 will cover a square. If 6 inches to one another, 600 will cover a square.

This is for common gable roofs. In hip rcofs, where the shingles are cut more or less to fit the roof, add 6 per cent to above figures.

A carpenter will carry up and lay on the roof from 1,500 to 2,000 shingles per day, or 2 to  $2\frac{1}{2}$  squares of plain gable roofing, so that an average price per square for simply laying the shingles would be \$1.40. Add 40 cents for laying the roof boards, and the labor account on a common shingle roof would be \$1.80 per square.

Tin Roofs.—A sheet of roofing tin is  $14 \times 20$  inches, and a box of tin contains 112 sheets.

Allowing the usual amount for side ribs and top and bottom laps, a box of tin will cover 182 square feet, and is worth about \$6.50 per box. 1 C. charcoal.

Laying a bux of tin will cost as follows:

1 box 1 C. charcoal tin	\$6.50
10 pounds solder, 15c	1 50
Preparing tin for roof	1.80
Laying tin, 1 1/5 days	3.20
Total	

**Valleys.**—Tin valleys for shingle roofs are generally 14 inches, and for slate roofs 20 inches wide. An average price put on the roof, including material, would be 12 cents per square foot. One man will lay  $1\frac{1}{2}$  squares per day of valleys, in plain work; when roof is steep or valleys cut up, 1 square is a day's work.

**Flashings**.—Tin flashings for chimneys and where one part of a building joins another are worth, put on, 13 cents per square foot.

### Gutters and Spouts .---

Gutters, 4-inch, are worth, put up, 12 cents per lin. foot. Gutters, 5-inch, are worth, put up, 14 cents per lin. foot. Gutters, 6-inch, are worth, put up, 17 cents per lin. foot. Down spouts, 2-inch, are worth, put up, 10 cents per lin. foot. Down spouts, 3-inch, are worth, put up, 12 cents per lin. foot. Down spouts, 4-inch, are worth, put up, 14 cents per lin. foot. Down spouts, 6-inch, are worth, put up, 30 cents per lin. foot.

**Slate Roofs.**—The prices per square for slate roofs can be had of slaters in any of our towns and cities.

They will vary from \$8 to \$11 or \$14 to \$16 per square.

The following table will be found useful to the estimator.

Names.	Size.	3 in.Lap naile	3 in. Lap ualled u bead.	lares covered	1200, First	d to cover one t 3 in. gauge.	r Square, First	Na requ P Squ	ills ilred er are,
		Gauge for in center	Gauge for 1 in. fron	No. of Squ by 1200.	Weight of Quality.	No. require Square a	Weight per Quality.	Iron.	Copper.
Singles	12 x 8	1n. 44	in. 4	3.0	ew1. 18	400	ewt.	No.	lbs.
Doubles	13 x 6	5	43	2.5	15	480	6	960	6
Ladies.	16 x 8	61	6	4.5	25	266	51	532	31
Viscountesses .	18 x 10	$7\frac{1}{2}$	7	6.2	35	192	61	384	21
Countesses.	20 x 10	$-8\frac{1}{2}$	8	-7.0	-40	170	53	340	4
Marchionesses.	$22 \times 11$	91	9	8.7	50	138	51	276	31
Duchesses	24 x 12	101	10	10.4	60	115	51	230	3
Princesses	$24 \times 14$	101	10	12.2	70	- 98	53	196	3
Empresses	$26 \times 16$	111	11	15.2	95	79	61	158	31
Imperials	30 x 24	13 <del>1</del>	-	$\frac{1}{2.5}$	-	36	8	72	3
Rags	36 x 24	$16\frac{1}{2}$		2.2	-	25	9	50	31
Queens	36 x 24	$16\frac{1}{2}$	-	2.2		25	9	50	31/2

SLATER: MEMORANDA

A .--- Squares eovered by 1 ton.

The above sizes sometimes slightly vary, according to the quarry.

Slates are classed according to their straightness, smoothness of surface, fair even thickness, presence or absence of discoloration, etc. They are generally divided into first and second qualities, and in some cases a medium quality is quoted. Slates of first quality are thinner and lighter than those of inferior quality.

Rule to find the number of slates required to cover one square: One square in inches + width of slate in inches  $\times$  gauge in inches.

The weight of slating on roofs is 8 pounds per foot super. for all sizes, except rags or queens, including a 3-inch lap and nails.

As there are two nails per slate, the number required per square will be found by doubling the number of slates. The trade "thousand," or "long tally," equals 1,200 for buying and selling.

**Nails.**—Composition nails are best for all good work, as they are stiff and tough. They are cast from an alloy of 7 copper to 4 zinc, and have a yellow, brassy appearance. Copper nails are either cast or wrought; but they are soft and dear. Malleable iron nails are frequently used, dipped while hot in boiled linseed oil to preserve them from corrosion. These can also be painted or galvanized. Cast-iron nails are only employed for temporary work. Zinc nails are very soft, and liable to bend, and as their heads come off in driving, they make a good deal of waste.

All these nails are sold by weight, and the price should lessen with the increase of length. Allow 5 per cent for waste in reckoning the number to the square.

Nails for small slates, such as Doubles, etc.,

Nails for medium slates, such as Countesses	1‡ in. long
etc., should be about	11 in. long
should be about	2 in. long

#### SLATE NAILS

Galvanized slate nails, per keg. 3d	
Galvanized slate usile nor log 41	\$5.50
Tinned slate neile	5.00
Thined state name, per keg, 3d	5.75
Tinned slate nails, per keg, 4d.	5 95
Polished steel wire nails 3d and 4d	0.20
Copper slate nails nor nound	4.00
Those prices and the state of t	.20
these prices vary with time and locality.	

Labor.—The labor in holing slates, any size, is' usually estimated at \$1.50 per thousand; but if a single

slate-holing machine is used, a smart boy, at 15 cents per hour, will be able to hole from 300 to 400 slates in an hour.

The following statement shows the labor required per square, which will be less for larger surfaces, as the slating will be performed more quickly. The difference in time for the various kinds represents the extra trouble in handling, greater areas being covered with larger slates in a given time, and the labor in holing is the same for all sizes.

A slater and	assistant wil	l lay:-			
1 square of	Doubles (w	ith two	nails each)	in 23	hours.
"	Ladies	"	"	· ' 1 Å	44
**	Countesses	**	66	"11	
66	Duchesses	**	"	"1	б ((
A slater and a	ssistant will	prepare	and lay:-		
1 square of	Doubles (w	ith two	nails each)	"4	"
"	Ladies	"	"	4 21	66
44	Countesses	"	66		**
"	Duchesses	"	<b>66</b>	"13	"
Plastering a	igainst unde	rside of	slating, per	- 4	
yard su	per	• • • • • • •	• • • • • • • • • • • •	· · · · 1	"

Cost per Square.—Taking Countess slates, 20 inches long by 10 inches wide, the gauge, if center-nailed, would be:  $\frac{\text{Length of slate} - \text{lap}}{2} = \frac{20 \text{ in.} - 3 \text{ in.}}{2} =$ 8½ inches. In estimating, therefore, the number

of slates required per square of 100 feet super., the width of the gauge in inches, multiplied by the breadth of the slate in inches, gives the margin or exposed surface of a single slate. This divided into the number of superficial inches in a square (100 feet super. by 144 square inches = 14,400 super. inches per square), will give the number of slates to a square—

e.g.,  $8\frac{1}{2}$  inches gauge by 10 inches breadth of slate = 85 square inches margin, and  $\frac{14,400 \text{ super. in. per square}}{85 \text{ sq. in. margin p.r slate}}$ = 170 Countess slates per square.

Allowing 5 per cent for waste, this would give roundly 180 slates to the square.

As there are two nails per slate, the number of nails required per square will be found by doubling the number of slates—i. e., in this case, 340 nails. Also reckoning 5 per cent waste for nails, the number for estimating would be some 360. Using 1½-inch composition nails, 144 of which go to the pound, this latter number would give exactly 2½ pounds per square, as they are sold by weight.

A slate roof is laid by first placing a course on the All courses above this one must be laid with a eaves. lap of more than one half the length of the slate or the vertical joints which are not close will not be covered. The lap of the slate is more than one-half its length, so the more lap a course is laid with, the better will be the roof. Manufacturers allow 3 inches when selling a square of slate, and architects and consumers should see that the roof is laid with that amount of lap, as a less one is a considerable gain for the dishonest roofer, which he takes advantage of to the permanent injury of the roof, because any less lap than 3 inches greatly endangers the weather-proof qualities of a slate roof. Slate, before it in laid, should be carefully sorted, the thick ones used to start the roof at the caves and the thin ones to finish with at the comb. In nailing slate do not drive the nails too tight. The top of the nail should be just even with the surface of the slate.

7055 5473 7770 9240 8470 10164 501 1107 13292 9546 12307 14769 11076 13846 16415 00221 0020 11200 00221 0000 1000 00221 0000 1000 00221 10000 1000 (9440) 11 5.40 1004au 1.254a) 69/241 20071 76/241 20071 82 511 1019 (ALMA) 194.345 1442 19200 石石石 - <u>-</u>. NOO 10867 13334 14000 OINUTS 1-000 20000 24000 NUMBER OF SLAVE IN ANY NUMBER OF SQUARES, FROM 72 UP TO 60 SQUARES 5410 7381 5570 11 4 1545 875 1000 13 4 1545 875 1000 13 7 1541 1042 13000 15 1 1554 1672 1300 1555 17 1 8751 1300 1455 17 5 1818 1300 1455 17 1 1220 1455 1800 23 INNY. 224 (P.U) Har Star 9600 12800 16000 1 3% 2012 5455 5442 6504 6514 111 1 1 1 1 1 2001 614 2014 1 2015 2045 N533 1 3449 112 1331 0141 10000 3714 18285 ₽*1*. 1-HE (H)T-(NIL) 24: 288 10 - C Real 5760 (Autro 24MS 177 13.64. t-ti-tt. サノシー 8.7 1282 #197 1710 3200 1923 1923 2164 P.T. ..... 12121 31445 121 3740 5334 (1151) 1.363 3640 Single Si 897755598 899898 5061 (NU) 5.7 (M)r-20 2017 241 241 241 245 206 2002 3170 344 3 2 240 2017 256 365 3272 3490 376 392 4 1/2 280 314 13 3665 382 449 450 450 452 3 2 200 364 75 450 460 366 5236 5 2 200 364 74 550 450 326 568 560 6 1/11 455 4 550 560 560 560 560 560 7 1543 1516 1625 1733 1542 1647 1768 1894 2021 - 517 15 1929 2066 2204 254 1. j. 2400 25.00 12.0 17.4 25 3264 3456 3266 3413 3540 3540 3299 346713 54 4000 4267 4531 4500 0 1286 1372 1458 1 1 469 1567 1565 1 1 1714 1828 1942 2 12112 上京 1 #2 ÷. 1280-1140 [nov. 1760] [0.20] 2080 2240 2400 [536-1725 [020-2112 2364 2406 2088 25 [506-1920-2133-2346 2560] 295 2330 空方 3171 1 1926 ±.ż [1086] [210] [331] [452] [573] 16 [271] [412] [552] 1694 [855] 19 [271] [412] [552] 1694 [848, 2002] 21 [356] 1540 [1694] [848, 2002] 2502 23 1515 1642 1 1515 1642 1 1653 1791 1 11151 nż. 1020 白虎 1389 31218 1309 1496 1683 1870 2057 1527 1745 1983 21×2 2400 1833 2044 2256 0 18 0840 三方 1807 2848 857 979 143 2. 141 1. 5 145 5 5 60 SU 11 ()# 67 × 1 975 1137 1240 128 o,ż 1992 33ta. 275 S66 · 7 1. 12.78 126 1929 808 9-10 1120 1152 1344 1250 1493 203 2618 000 1-222 650 1120 1310 1310 1310 = <u>;</u> 12 T T T T 10 1904 + 086 1001 1300 1163 1453 1 1309 1636 1 195 228 1991 in in 002 11)4 717 1496 1870 1999 7999 + 5 332 135 081 42 ×t() č. 207 S 333 929 8446 GEAR 12221 190 121 217 ~ 5 12288 80255 334 370 1155 374 23 281 326 8752 -3 127 28148 14 1853 C+ 64 30 123 3773 жã 7 (1+) 101 93 **E**8% 60 31 1 1910 1212 421 219 11129 01 2 ox ×1-10 201-24 x J 21 x { 2(i x { ж. 1996 ÷ -

**Cornices** At or lina y plain cornice h three members, w/z frie out and fuscia.

The next part nailed or fastene to the side of the filding

The soffit is the sart attaches to the under side of the prosing the result, or locatout.

The feature is the and of the raftthe or location if.

own molds in ling on the fascia.

the ieze at 'so a

I. estimation of terial in a even in for quarior sultiple ne entire outside something by the sum of the width of soffit zeo fascia; the result is the number o bo measure.

or ga ofs, to the lengths of the two sides of the build and the end projections and length end fters an multiply as before.

Table ibor account on cornice work.

n Doofe

N'in freet two men will put on per price of:

	VIDTH INCHE	······		
	So 1	Fascia	No. Feer	Cost Di -
1	1	4	80	71
()	1.2	-4	75	82
12	16	-4	60	10
14	20	5	48	124

The above is for gable roofs and includes cost of sectoding.

P .	100013	Se	offit	Fa	scia.	No. Feet	Cost per Foot
	11.	16-	inch.	4-i	neh.	75	g
	• •	20	6.6	41	6.6	64	91
-25		24	f t	5	**	02	12
04 94	44	-28	**	$5\frac{1}{2}$	66	40	15
01		32		6		32	20

### Cornice Mouldings .--

CIOWII I	mould	mg, nat,	-2-	mch.	-800 fe	et per da	W.Or 80c	ner 100 foot
"	"	spring	4	"	500		\$1.20	"
<b>66</b>	"	"	5	"	445	"	1 21	"
"	4.6	"	6	"	365	**	1.61	"
"	66	4.6	7	**	300	"	2 00	"
"	"	"	8	**	250	"	2.40	"

The cost of cornice moulding is ordinarily 1 cent per lineal foot less than the number of inches in work -2-inch moulding, 2 cents; 3-inch, 3 cents, etc.

Bed moulding, flat, 1½-inch, 800 feet per day, or 80 cents per 100 feet. Bed moulding, flat, 2 inch, 750 feet per day, or 84 cents per 100 feet. Bed moulding, flat, 3-inch, 700 feet per day, or 88 cents per 100 feet. Bed moulding, flat, 4-inch, 500 feet per day, or \$1.20 per 100 feet.

**Cornice Brackets.**—Price per bracket, soft wood, all well worked—cost to put on building:

rerpe	ndicu	lia <b>r</b>	Horiz	ontal	Thi	ckness	Cost Plain	Moulded	Diain	Mouldad
Size,	16-i	neh.	12-i	nch.	21	-inch.	\$0.35	\$0.42	\$0.15	
44	20	٤.	16	4.6	3	**	70	80	90.10	\$0.20 0"
**	24	"	20	6.6	4	"	70	.00	.20	.20
**	28	"	24	"	5	**	1.00	.00	.14	.20
"	30	"	28	4.6	6	"	1.00	1.20	.25	.35
	00		-0		0		1.50	1.60	.35	.45

Plain panel moulding, two men will put on 300 feet per day. Foot moulding, two men will put on 400 feet per day.

			F	LOORS				ost
Soft wood,	6	in.wide,	without	bridging,	per joist,	800	sq. ft.	\$0.80
	6	44	with	"	"	650		00
"	4	66	without	44	66	600	**	.00
"	4	""	with	44	66	500	66	1.04
"	3	**	without	"	66	400	**	1.01
14	3]	**	with	44	44	300	"	1.50

Two men will dress six squares of flooring after laying per day, or at a cost of \$1.00 per square.

If flooring is of hard wood, estimate per day twothirds of above.

The number of feet, board measure, in a given floor is had by multiplying its length by its width and adding one-fifth for lapping. For flooring not matched omit the lapping. Two men will lay 1,333 feet of plank flooring per day, or 45 cents per square, or will lay 2,000 feet of common rough flooring, 1-inch stuff, or 30 cents per square.

Outside ceiling for wood buildings, average width, including beading and scaffolding, is worth, to put up, \$1.25 per square. An average day's work for two men is five squares. Two men will dress, after laying the ceiling. five squares per day, or \$1.20 cents per square. Ceiling overhead is generally of wider stuff than outside ceiling; as ther is no beading, and the workmanship is not so particular, two men will put up the same amount as of outside ceiling, including putting up and taking down scaffolding, or five squares at 80 cents per square.

Wainscoting. — Wainscoting 2½ to 3 feet high, beaded, with ordinary capping, including dressing after putting up, is worth \$3.00 per square. Two squares is a day's work for two men.

The same, 3 feet to 4 feet high, is worth, to put up, \$2.00 per square.

The same, with shoe and heavy caps, is worth \$2.60 per square. The capping to wainscoting is ordinary moulding from  $1\frac{1}{2}$  inches by  $\frac{2}{3}$  to 2 inches by  $1\frac{1}{3}$  inches.

Panel wainscoting, mill worked, ready to put up, including capping, shoe or base, is worth, for labor, \$3.25 per square.

Hand-worked panel wainscoting is of so various a kind that definite prices of labor cannot well be given

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without specifications. In a general way, the price per square for getting out and putting up will vary from \$3.00 to \$20.00 per square.

The above prices are for soft wood. For hard wood add 40 per cent.

**Baseboards.**—Plain base, 6 to 10 inches wide, put up before plastering, is worth  $1\frac{1}{2}$  cents per lineal foot for labor. Two hundred feet is a good day's work for a man with mill-dressed lumber.

The same, put on after plastering, including putting on grounds, is worth 2 cents per lineal foot.

Plain base, after plastering, with moulding, leveling, or capping by hand—mill-dressed stuff—is worth  $2\frac{1}{2}$  cents per lineal foot to get out and place in the building.

Stairs.—The wall string is the board with which the ends of the steps are fixed next to the wall.

The face string is the board that carries the outeend of the steps and risers.

The tread is the horizontal board of the step.

The riser is the upright board of the step.

The netwel post is the upright post at the lower step to receive the hand rail.

The hand rail is the rail supported by balusters. Balusters are small columns or pillars to support the rail.

The number of risers is found by dividing the distance from floor to floor by the height of the rise.

The height of each rise is found by dividing the distance from floor to floor by the number of risers.

The number of treads is one less than the number of risers.

The width of each tread is found by dividing the risers by the number of treads and adding the projection.

Risers vary in height from 4 to 8 inches. Treads run from 8 to 14 inches.

It will be impracticable to give detail prices for all variety of stair-work on account of the diversity of designs. We simply give a few as an illustration. The labor on rough, open stairs, for cellars or stables, when no risers are used, is worth 16 cents per tread. Straight stairs between partitions, 2 feet 6 inches to 3 feet 6 inches long, with 6-inch to 9-inch tread, and 7-inch to 8-inch risers, are worth 35 cents per riser.

Winding stairs, same dimensions, 40 cents per riser. Open straight stairs, risers  $6\frac{1}{2}$  to 8 inches, treads 6 to 11 inches; housed in wall strings, mitered to face string; moulded nosing, including putting up turned balusters, and plain round or oval rail, with 6-inch to 8-inch turned newel post, are worth for labor \$1.10 to \$2:00 per riser.

The same stairs, winding, charge \$2.50 per riser for the winding steps, and \$1.25 for straight steps. Putting on brackets outside of stringer is worth from 5 to 12 cents per bracket.

The following is a list of the approximate prices of stair material:

Newel Posts.—A turned newel post of cherry or black walnut, 5 inches in diameter, with cap, is worth \$3.50; 6 inches, \$4.00; and 8 inches, \$5.50.

Octagon newel posts, walnut, oak, or cherry, with ornamental cap, 8 inches, \$8.00; 9 inches, \$8.50; and 12 inches, \$10.50.

Newel posts veneered with fancy woods, with carv-; on plinth and cap, and moulded sunk panels, will .y from \$20.00 to \$60.00 each.

**Balusters.**—Turned balusters, walnut or cherry, from 2 feet 4 inches to 3 feet, are worth, 1½ inches, 10 cents;

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2 inches, 14 cents; and  $2\frac{1}{2}$  inches, 20 cents each. Oak and ash 20 per cent less.

Fluted or octagon balusters, walnut or cherry, 2 inches, 18 cents; 2½ inches, 25 cents; 24 inches, 30 cents each. Fancy balusters for high-priced stairs may run from 40 to 60 cents each.

**Bails.**—Walnut or cherry,  $3\frac{1}{2}$ -inch, 15 cents; 4-inch, 20 cents;  $4\frac{1}{2}$ -inch, 22 cents; and 5-inch, 22 cents per lineal foot. Raised back rails, walnut or cherry, 4-inch, 25 cents; 5-inch, 30 cents;  $5\frac{1}{2}$ -inch, 36 cents; and 6-inch, 40 cents per lineal foot. Fancy raised back rails from 6 to 7 inches will vary from 50 to 70 cents per foot.

**Doors.**—The price of doors may be had from any dealer's catalogue. The labor account is as follows: A fair day's work for one man is setting 5 door frames a day, and putting on ordinary casing. He will also hang and finish 5 doors per day, or \$1.20 a door complete. The above is for 6 feet to 7 feet 6 inch doors, and 1<sup>§</sup> inch thick. From 7 feet 6 inch to 9 feet doors and 1<sup>§</sup> inch thick, a day's work of setting and casing 3 frames per day, or hanging and finishing 3 doors per day, \$2.00 per door complete.

**Moulding Door Casings.**—For 6 feet to 7 feet 6 inch doors, and 3 inch mouldings, one man will mould 6 door casings, two sides, per day, or 50 cents per door; with  $4\frac{1}{2}$ -inch mouldings, 5 doors per day, or 60 cents per door. Mouldings with two members about onehalf above number, 7 feet 6 inches to 9 feet doors, single moulding two sides, 5 openings per day. The same, with double members to moulding,  $2\frac{1}{2}$  openings per day.

Door frames when had from factory are cased both sides for inside doors, and one side for outside doors.

**Sliding Doors.**—The frames for a pair of sliding doors with double joint, including casings each side, are worth from \$3.50 to \$4.00 per frame.

The same, with segment top, will vary from \$6.00 to \$9.50; setting either one of the above frames, putting up the track, and lining the pocket is worth from \$3.50 to \$4.00 for labor. Setting, hanging, and trimming a pair of sliding doors will take a man about  $1\frac{1}{4}$  days, or \$3.75 per door.

Folding Doors.—The frame for a pair of folding doors with opening 5 feet by 8 feet 6 inches, with single joints, including casing each side, is worth from \$3.50 to \$4.25 per opening. Segment top, same size opening, \$6.00 to \$8.00. Setting the frame for a pair of folding doors will take a man three-quarters of a day, or \$2.25 per frame.

Fitting, hanging, and trimming a pair of folding doors will take one man a day and a quarter, or \$3.75 per door.

Moulding, sliding and folding door casings, square top opening 5 feet by 8 feet 6 inches on both sides, single member; a day's work is 4 openings per day, or 75 cents per door. If moulding is double member, two openings per day, or \$1.50 per door. Segment top with same size of swing, the moulding will cost \$3.00 per opening. Over the face of a square top, one man will put on the moulding with a single member in onehalf a day, or \$1.50 per opening. Double member one day, or \$3.00 per opening.

Setting door frames in brick buildings will cost the same as fcr frame buildings.

**Common Door Frames.**—Outside fram s. with casings on one side for doors, from 2 feet 6 inches by 6 feet 6 inches to 2 feet 8 inches by 6 feet 8 inches, are worth

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from \$2.25 to \$3.50 each. The same for inside doors, with casing on both sides, are worth from \$3.00 to \$4.00.

**Door Trimmings.**—Butts  $3 \times 3$  inches, for cheap trimmings, are worth 10 cents per pair, and a common mortise or rim lock, with brown knob, 30 cents each;  $3 \times 3\frac{1}{2}$  butts, 10 cents, and  $3\frac{1}{2} \times 3\frac{1}{2}$ , 10 cents each;  $4 \times 4$ , 15 cents. A good mortise lock, with brown or white knobs, brass key, face, and bolt is orth 45 cents. Outside door locks vary from 50 cents to \$2.00 a pair; average price would be \$1.00.

Sliding door locks  $4 \times 5$ , brass key and face, \$1.50 each. Iron track for door, 3 cents per foot; brass track, 25 cents. A very good rabbeted lock, without night works, \$1.50; with night works, \$2.50 to \$4.00 each.

Screws for putting on above trimmings, 30 cents a gross. The labor account for trimming doors will be found under the head of doors.

Windows.—The price of the sash, including glass and glazing for all sizes of windows, may be had from the dealers' catalogues. Window frames, factory made, simply have outside casings and jambs. One man will cut the openings and set five frames per day, of an average size, say 2 feet 6 inches by 6 feet, in a frame building, and can set the same number in a brick building, or 60 cents per opening.

As the brick-work goes up the carpenter must plumb up the frames occasionally, so that a fair estimate would be both alike.

In larger openings, setting from two to four frames per day would be fair work, or from 60 cents to \$1.25 per window.

One man will case 12 windows per day of windows 2

feet 6 inches by 6 feet, or 1½ cents per lineal foot of the casing.

Moulding window casings, same price per foot : door casing.

For we d buildings, plain rail sash, 8 or 12 lights, with outside casings, an average price would be as follows:

 $8 \times 10$ , \$1.20;  $10 \times 12$ , \$1.50;  $10 \times 14$ , \$1.80;  $10 \times 16$ , \$2.20.

With check-rail sash outside, casings:  $8 \times 10$ , \$1.80;  $10 \times 12$ , \$2.00;  $10 \times 14$ , \$2.20;  $10 \times 16$ , \$2.40;  $10 \times 18$ , \$2.60.

Plain window frames for brick buildings:  $8 \times 10$ , \$2.00;  $10 \times 12$ , \$2.20;  $10 \times 14$ , \$2.50;  $10 \times 16$ , \$2.60;  $12 \times 24$ , \$3.65.

Box window frames:  $8 \times 10$ , \$2.85;  $10 \times 12$ , \$3.00;  $10 \times 14$ , \$3.20;  $10 \times 16$ , \$3.50;  $12 \times 24$ , \$4.30.

The same frames, with segment outside and square inside, are worth 50 cents more.

**Pantries and Closets.**—In ordinary work of this kind one man will get out and put up 50 to 75 lineal feet of shelving 12 inches wide per day, or will make and put up five drawers 15 inches wide by 18 inches deep, including racks and fitting.

If the drawers are dovetailed, four is a day's work. Strips and hooks: one man can put 50 to 80 lineal feet of strips, and put on closet hooks, about 12 inches apart, in one day.

**Porches.**—These differ so widely in design that prices per foot lineal cannot be given without specifications, as they will vary from \$1.25 a foot upwards. In an ordinary porch, figure the sills and joists as in framing; also roof, labor, ceiling, and cornice the same as in other parts of the building, and charge for whatever extra work the design may call for.

Blinds.—These are made and sold by the foot, measuring height of the window on one side only; 60 to 70 cents per lineal foot, including trimming and hanging, is a fair price. Inside blinds, O. G. panel or rolling slats, ordinary width, are worth \$1.25 per foot, complete in the building. If inside blinds are of hard wood, they are worth from one and a half to double the price of pine.

**Plastering**.—The number of yards is simply the area of all the walls and ceilings.

One hundred yards of plastering will require 1,400 laths,  $4\frac{1}{2}$  bushels of lime, 18 bushels of sand, 9 pounds of hair, and 5 pounds of nails for two-coat work.

Three men and one helper will put on 450 yards, in a day's work, of two-coat work, and will put on a hard finish for 300 yards.

Retail cost of three-coat work for 100 yards of plastering:

Seven bushels of lime at 30 cents	\$2.10
Four-fifths of a load of sand at \$1.25	1.00
Nine pounds of hair at 2½ cents	.24
Five pounds of nails at 41 cents	22
Lathing, 100 yards at 21 cents	2.25
1400 laths at \$3.00 per 1000	4.20
Plastering, 2 coats, 1 man 3 of a day	2.00
Helper, <sup>1</sup> / <sub>b</sub> of a day	.33
Hard finished, one day's work	3.00
Making mortar and scaffolding	1.50

Total cost ......\$16.84 Or, say seventeen cents per yard.

**Painting.**—Painting is done by the yard, and at the present prices of lead and oil, house painting in plain colors will cost on an average:

For one coat, 8 cents per yard; two coats, 15 cents per yard; three coats, 23 cents per yard.

One coat, or priming, will take for 100 yards of painting 20 pounds of lead and 4 gallons of oil. Twocoat work, 40 pounds of lead and 4 gallons of oil. Three-coat, the same proportion; so that a fair estimate for 100 yards of three-coat work would be 60 pounds of lead and 12 gallons of oil.

A day's work on outside of a building is 100 yards of first coat, and 80 yards of either second or third coat. An ordinary door, including casings, will on both sides make 8 yards to 10 yards of painting, or say, 5 yards to a door without the casings. An ordinary window  $z\frac{1}{2}$  to 3 yards. Fifty yards of common graining is a day's work for a grainer and one man to rub in.

In measuring up outside work, use the rule for plain surfaces. In common painting run your tape-line over all the mouldings in and out, and this, with the width of the cornice multiplied by its length, will give the area. It is customary to add from one-third to onehalf for the bracket painting. In painting blinds of ordinary size, twelve is a fair day's work for one coat, and 9 pounds of lead and 1 gallon of oil will paint them. In measuring up inside base, it is customary to reckon 9 inches in width and upwards to 1 foot as 12 inches.

Nails.—One thousand feet of inch stuff will require 10 pounds of 10-penny nails; 1 square of siding or ceiling, 2½ pounds 8-penny, and the same for a square of roof boards or sheathing, and 1,000 shingles will take 6 pounds of shingle nails.

Brick and Stone Work.—A day's work in excavating and filling into cart or wheelbarrow is 11 or 12 cubic yards of common earth, or 7 to 8 yards of clay or coarse gravel, or 14 to 16 cents per yard. In limestone or sandstone a day's work in quarrying will range from one-half to one cord of stone.

# HODGSON 3 ESTIMATOR

**Stone Work.**—A perch  $1 = 16\frac{1}{2}$  feet long,  $1\frac{1}{2}$  feet wide, and 1 foot high, and contains  $24\frac{1}{4}$  cubic feet. In estimates 25 cubic feet is figured as a perch.

A perch in the wall contains about 22 cubic feet of stone and 3 cubic feet of mortar.

The waste ordinarily allowed in laying stone walls from the rock measurement is one-fifth.

A cubic yard of rubble masonry laid in the wall contains 1½ cubic yards of undressed stone and onefourth of a cubic yard of mortar.

Four perches or 100 cubic feet of wall will contain ordinarily 1 cord of stone or 128 cubic feet, 1 barrel of lime, or say  $2\frac{1}{2}$  bushels, and 5 barrels of sand.

A day's work for a mason's helper is moving 4 to 5 perches of stone, and mix and carry to the mason sufficient mortar to lay them.

A man will lay in one day from 4 to 5 perches of rubble masonry in sandstone, or 3 perches in limestone. In many locations sandstone is delivered for \$1.25 per perch, and the labor for laying in ordinary walls, including lime and sand. from 95 cents to \$1.25 per perch.

Stone Ashlars.—These are ordinarily 3 feet to 5 feet long, 1 foot high, and 4 to 6 inches thick.

The price of the rough stone will vary according to locality. The labor on ashlars, including setting, is per square foot as follows:

Fine posts,	hammerwork,	limestone,	<b>3</b> 0	cts.;	sandstone.	25	cts.
Medium	4.4	6.6	28	44	66	22	4.4
Rough	4.6	44	20	4.6	6.6	17	6.6

Freestone ashlars, sawed, are furnished at the mills for 25 to 35 cents per square foot, and caps and sills for ordinary windows and doors from \$1.35 to \$1.70 each.

Brick Work .- The labor and material of brick work

are estimated by the 1,000 brick. In measuring up brick walls it is not customary to deduct for openings. To ascertain the number of bricks in a wall: First obtain the number of superficial feet, and multiply this by 7 for a 4-inch wall, by 14 for a 9-inch wall, 22 for a 14-inch wall, and 29 for an 18-inch wall. If thicker than 18 inches, for each additional 41 inches in thickness add 7 bricks per square foot.

One thousand five hundred brick is an average day's work for outside and inside walls, and we take threequarters of a barrel of lime and 9 bushels of sand to make the mortar. The number of brick a mason will lay in a day on a plain wall depends largely upon its thickness. On 9-inch work, 1,200 to 1,400; on 14-inch work, 1,500 to 2,000, and on 18-inch work, 2,000 to 2,500; veneered work or single-back walls attached to wood work is much slower, from 400 to 600 brick is regarded a day's work; this includes tying the brick with nails to the framework, or sheathing.

The following is given as an illustration of the cost of furnishing and laying 1,500 brick, or one day's work.

1500 brick at \$6 per M	
# barrel of lime at \$1	0
9 bushels of sand at 5 cont	5
1 day's work for many	5
1 day's work for het	)
2 duy s work for helper 2.00	)
Total	-
Or, \$10.14 per M.	)

Chimneys.-Common flues and ordinary chimneys are worth from 40 to 75 cents per running foot, including labor and material. In large chimneys with fireplaces, get the number of brick, charge for lime and sand the same as in brick walls, and estimate the labor at double the price of plain walls of same thickness.

**Plumbing.**—In plumbing for bath-rooms and closets 14-inch pipe is used for water, §-inch for supply, and 4-inch iron pipes for søil-pipe. An average price would be for material and putting in the building: 14-inch pipe, lead, 40 cents per foot; §-inch pipe, lead, 32 cents per foot, and soil-pipe, 35 cents per foot.

Bath-tubs will vary in price from \$15.00 to \$50.00; double bath-cocks, \$12.00 to \$15.00; single, \$1.90 to \$3.00; wash-bowl cocks, from \$2.00 to \$3.00.

A fair price for a corner wash-bowl, marble, with stop-cocks and enclosed with casings, including connections with pipes, will vary from \$12.00 to \$20.00; water-closet basins and connections, \$6.00 to \$8.00.

It must be understood that the foregoing prices are only approximately correct.

### SOME PAINTER'S EXTRAS

In estimating the painter's work, a few facts and data as to the quantity of paint required to cover certain areas of surface are necessary. Thus it is useful to know that 1 pound of mixed white lead paint will cover about 41 superficial yards the first coat, and about 61 yards each additional coat; that 1 pound of mixed red lead paint will cover about 51 vards super. of iron. Some authorities say 45 yards of first coat, including stopping, will require 5 pounds of white lead, 5 pounds of putty and 1 quart of oil; and 45 yards of each succeeding coat will require 5 pounds of white lead and 1 quart of oil. These quantities do not exactly agree, but they are approximately correct, and we may take about  $6\frac{1}{2}$  to 7 yards to be about a fair allowance for 1 pound of paint; if the paint costs, say, 15 cents per pound, the cost would be about 21 cents per yard for material; 1 pound of mixed white lead

paint will cover 1 yard super. on Portland cement first coat); good oil varnish requires 1 pint to 8 or 9 yards superficial, one coat.

In measuring the painting of iron railing, the two sides are measured as flat work, both sides plain, and charged as such, unless gilded; if the railing is delicate and ornamental, the charge is once and a half, or twice is taken for each side.

The rotation in taking the items are generally the windows, base dades, cnimney pieces, doors; but this rule is not strictly observed, and in the abstracting the one-coat work comes before the three, four, or five times in oil; flatting and ornamental work follow the plain painting.

It may be useful to remember that the decimal .27 multiplied by the rate of wages for a painter per hour will give the cost per yard for common work, including stopping, knotting, etc. and the decimal .15 for second and following coats.

Staining, sizing ar.' ing taken at per yard superficial should be a stand is to stain and the number of coats of varnish. We varnished work, state if on natural wood or painted. Graining and vareighing at per yard is similarly measured to plain painting, and should be described as "extra"; state if "combed," "once grained," and varnished, and the wood to be imitated as oak, walnut, etc. if once or twice varnished, and if with spirit or copal, if the wood is to be sized.

### WOOD AND IRON WORK

95 yards 5 feet super. Knotting, stopping, priming, and painting wood work three times in oil and lead color. Taking the decimal .27 and multiplying by

rate of wages per hour would give the cost per yard.

The price-books give 20 to 25 cents per yard for three-coat work.

105 yards super. Dit'o four times on cement work. Add to the above 5 cents per yard, say, 22 cents per yard for a large quantity.

54 yards super. Painting four times balusters of staircase. These are ornamental and close, and the quantity given includes double face. Say, 25 cents per yard.

75 yards 6 feet super. Ditto five times iron railing. About 5 cents per yard more than last.

75-foot run.  $4\frac{1}{2}$  inch reveals in five oils. Worth about 8 to 12 cents per foot.

36-foot run. Painting r. w. pipes in four oils. Put this at 10 cents per foot.

66-foot run. Ditto eaves gutters. Same price.

35-yard run. Painting bars to skylights, four coats in oil. This is worth about 9 cents per yard.

120-foot run. Shelf edge, three coats. 3 cents per foot.

18-foot run. Painting in three oils, cornice 12-inch girth. About 8 cents per foot run.

62-foot run. Painting in four oils, window-sills about 12-inch girth. Price about 8 or 9 cents.

Painting in approved tints wood and stone chimney pieces, four coats. If of ordinary kind, the cost may be put at about 75 cents to \$1.00 each. Ditto ditto, extra coat and flatting. Add, say, 30 cents each.

30 yards super. Painting four times in oil, including knotting and stopping and flatting.

Say	for four-	-coat	work	on	wood	 	 . \$0 25
For	flatting	add.			• • • • • •	 	 

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In some price-books this would be put at 40 cents per yard.

26 yards super. Ditto ditto finished in party colors. Add 5 cents to the above.

5 yards super. Ditto finished in shades of Indian red. This is rather a dear color, and may be priced at 8 to 10 cents in addition.

60-foot run. Paint in three oils, reveals 4½ inches wide. Add about 6 cents per foot.

58-foot run. Ditto three and flatting to skirting not more than 10 inches wide. About 7 cents per foot.

10 yards super. Painting in three oils, enriched cornices and flatting. Price about 75 cents per yard, and add 20 cents per yard for flatting.

No. 12. Sash frames not exceeding 24 feet super., four oils. These may be priced at about 80 to 90 cents each.

No. 4. Ditto large size ditto. Add 25 cents to each.

No. 12. Dozen sash squares, about 2 feet super. each. Worth about 55 cents per dozen.

No. 4. Dozen ditto large. About 80 cents per dozen.

72-foot run. Painting base, four oils. These would be about 7 cents per foot.

72-foot run. Ditto finished in grayish-green. Add 1 cent per foot.

32-foot run. Ditto narrow base, four oils. About 7 cents per foot.

### GRAIN'NG AND VARNISHING

18-foot run. French-polishing handrail. Worth about 20 cents per foot.

50 yards super. Varnishing doors and framing, two coats copal varnish. Price at 20 cents per yard super.

45 yards super. Painting in four oils, doors finished in buff and gray of approved tints.

Price in common colors, four coats, including knot-

62 yards super. Graining extra in oak and twice varnishing. This may be priced at 50 cents per yard for best work, and for twice in copal 30 cents.

105 yards super. Graining wainscot and twice varnishing. Extra over common.

Graining cost per yard.	¢0. 20
Copal varmishing to	
copar varmisning, two coats.	

320 yards super. Varnishing matchboard partitions, etc., in two coats copal varnish, and sizing wood.

Sizing	wood, say																						20	10
Twice	in const and							• •	•	1	•	•	•	•	• •	1	•	•	•	•	•	• •	· 20 ·	10
	m copai, say.	•	• •	٠	• •	•	•	• •		•	•	•	• •									• •		20

32 feet super. Painting carved pediments and trusses four coats in oil, finished in two tints to be approved.

Say cost of four-coat work		81 11
Picking out in two states	1.1.1	07.12
reading out in two tints, per foot	 	

If very elaborate, the cost would be more, according to color selected.

32 yards super. Painting skylights each side four coats. The price would be about 28 cents each side.

12 yards super. Oak combed and shadowed and varnished. This may be for some special doors, and may be priced at 68 cents per yard.

If there are more yards in the work than named in the foregoing, then a reduction of from 3 to 5 per cent may be made. If there is a less number of yards, then an additional price of from 3 to 5 per cent may be added.

### THE PLASTERER AND PAINTER

In estimating for plastering, or for painting also, (1) the description of all materials and work should be kept separate. (2) Plastering on walls to be measured from the floor upwards, or from the point where each description of work commences. (3) Where cornices are lathed on brackets, measure ceiling and walls to the edge of the brackets only. (4) Where cornices are not bracketed, measure the ceiling full size of room, and the walls up to ceiling; all in super. yards. (5) Deduct all openings 100 square feet and over; deduct materials and add labor (hollows) for net sizes of doors, windows, fireplaces, and other openings under -100 feet super. (6) Where ceilings are paneled and coffered, or covered, girth round all portions that are lathed, keeping circular work separate. (7) Ceilings plastered between spars, etc., to be measured across the spars and purlins, and even then kept separate, and described as such. (8) All work run with a mould to be measured lineal on the wall, and the girth given, as cornices, rustics, strings, architraves, soffits, quirks, etc.; count all miters with the girth of mould they belong to; count miters in paneled work. (9) All cornices, etc., lathed on brackets, to be kept separate, and described as such. (10) All cast work to be counted, except running enrichments. (11) Enriched members to be measured lineal, with girth. (12)Modeling of enrichments to be, if special, so stated, and the models to be the property of the designer. (1.) Ceilings or walls covered with panels, formed by small moulds, to be measured super., with illustration or drawing, for "extra price over plain work"; larger paneling or special decorative features to be measured in detail. (14) Angles to pilasters, etc., if specially
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formed, lineal and extra to plastering. (15) Door and window frames, bedding and pointing, to be counted, and state material to be used; also flushing to inside of frames after fixing, or behind casings, window backs, or other work to be given. (16) Making goods generally, and after plumber, gas-fitter, bell-hanger, etc., and chimney pieces, as in item, stating numbers. (17) Coloring and white-washing walls, etc., to be in super. yards, measuring over all openings under 100 super. feet; if the work has to be pointed by the plasterer, state so. (18) Painting to include stopping and knotting, and to be given in square yards. Priming to be separate, if on work painted before being fixed. Painting to be girthed round all exposed surfaces, except as below. (19) Balusters, if ordinary square, and girds, gates, and other metal work painted on both sides, with bards about 5 to 6 inches apart, to be measured one surface only; if closer or slightly ornamental, 11 surfaces, and for very close or very ornamental work, 2 to 21 surfaces. (20) Windows to be measured each surface over full size of opening for painting frame and sheets, or else the frames counted, and the sheets, if large squares, counted: but if in small squares (as old-fashioned crown glazing), then count the squares instead of the sheet. (21) Fancy or ornamental painting to be measured in detail, with lengths of mouldings picked out, gilt, etc. All work in parti-colors to be kept separate from plain work.

The cost of internal plastering largely depends on the number of coats; the second or floating coat involves four processes: running the screeds, fillingin, scouring with a hand-float, and "keying" the surface for the finishing coat. This coat costs about a lc. more than the two coats and set. The third or finishing

coat also entails extra care and trouble. It involves laying, scouring, troweling, etc., and it requires "fine stuff," consisting of pure lime, slaked, saturated till semi-fluid sand. If "gauged" with plaster of Paris in the proportion of three or four to one, the work dries This is also used for cornices and enrichauicker. ments. Gauging with plaster costs about 8 cents each coat per yard extra, and therefore adds materially to the cost. The cements known as Keene's and Parian have quick-setting properties, and give a hard, nonporous surface; they are laid in two coacs, the first of cement and sand about 3 inch thick, and the finishing coat of neat cement. This kind of cement finish is used for angles and arrises, often ca Portland cement grounds, also for mouldings, girder-casings, soffits, skirtings, and other decorative features. Compared with ordinary three-coat work, it costs about one and a half times as much. Some authorities give 70 cents per yard on brick, and others 20 on lath, including profits, and on Portland cersent grounds.

There are several patent fibrous plasters used on canvas, wood, and etal for ceilings and decorations, that are advertised. These vary in price about 28 to 40 cents per yard. The estimator can obtain prices for any selected ceiling, wall filling, or decoration.

In estimating items of plasterer's work, care is necessary in ascertaining the quantities, and whether for "narrow widths," or for circular work. If for narrow widths, an extra price is necessary, being for labor, which would come to about 6 cents per foot super. more, or 1 cents if in plaster of Paris The quantity should also determine the price; for large quantities the labor might be priced at 1 cent less. Keene's fine quality cement takes a fine potish, and is used for

internal decorations, panels, columns; on brick walls it should be applied on a rendering coat of Portland cement. Parian cement is used as a stucco, and is valuable on new-built walls, as it can be papered or painted very soon afterwards; 4 bushels of Parian to 4 of clean washed sharp sand will cover 10 super. yards 1 inch thick. The price is about the same as Keene's cement.

Rake out joints of old brick work to form "key" for plaster.

This may be done in brick work for 3 to 5 cents per foot super., say 32 cents per yard, and the price depends much on the hardness of the mortar to be raked out. Raking out cement joint would be about 6 cents per foot.

Dubbing out 1 inch thick in tiles and cement to fill hollow in wall. This may be taken at from 9 to 13 cents per foot super., according to the kind of wall, and whether a scaffold is necessary.

Render, float, and finish in troweled stucco for paint. May be put down at 35 cents per yard on brick. Add for last coat finished troweled stucco for paint 13 cents per yard. Troweled stucco on lath would cost about 9 cents per yard more.

Lath, plaster, and set, finished troweled stucco in narrow widths. This would come to about 9 cents per foot super.

Ditto sloping ceiling in panels between ribs.

Say ordinary work. Extra for lathing	50
Add for setting coat between ribs.	18
D	00

Ter yard	 \$0.72

Moulded cornice, 15-inch girth. Price this as before, say, 28 cents per foot super.

Cornice, 5-inch girth. Worth about 14 cents per foot run.

Miters to ditto. Each, say, 14 cents.

Ditto 9-inch girth. Worth about 30 cents.

Miters to ditto. 28 cents each.

Enrichments 7-inch girth to detail; at 5 cents for each inch girth per foot, would come to 30 cents per foot.

Render, float, and set walls, gauged with equal quantities of lime and cement. Add 13 cents per yard to former price, say, 34 cents.

Ditto in narrow widths. Price at 50 cents.

If circular. About 50 per cent more than the straight.

Hacking face of old walls to form key for plaster. This is labor only, and may be put down at 5 cents per yard.

Ditto and raking out mortar joints. Add another 5 cents per yard.

Rendering chimney backs. Worth about 25 cents each.

Plaster plain face on brick in narrow width. If this is for lime and hair finished with setting stuff, it may be priced at 50 per cent more than for ordinary plastering; the difference is entirely for labor.

Plain face in Portland cement for skirting 10 inches high with sunk bead on top. Worth for plain face about 5 cents per foot.

Worth for plain Bead, per foot	face about 6c	per foot	\$0.06
in first struct	* * * * * * * * * *	*****	
			\$0.16

## PLASTER CORNICES AND ENRICHMENTS

Moulded cornice, as per detail, on lath. This item may be priced the same as previous item, adding lathing, say, 4 cents.

Papier-maché center flowers to drawing-room and dining-room, about 3-foot diameter, according to design. It is not easy to price this item without seeing the design, as they vary according to the degree of enrichment. For plain designs we may price them at 20 cents per inch diameter. For elaborate designs, 30 to 60 cents per inch would not be too much. Get list of prices.

Plaster center flowers, 18-inch diameter. These are worth about \$3.50 each

Ceiling decorations, as per design. No special decoration is described; if plain, the cost would be about 10 and 14 cents per foot, and fixing, say, another 12 cents.

Cornices to ditto to design. Price from 20 cents, for fixing add 20 cents per foot.

Frieze. About 30 cents per foot, including fixing.

#### KEENE'S CEMENT

Keene's cement, coarse quality, on brick walls, on rendering of Portland cement. Troweled on brick, at 70 cents per vard. This includes profit.

Ditto on single-lath partitions. Price at 78 cents per vard.

Ditto circular ditto. Add 14 cents per yard.

Pilasters and architraves ditto. This item depends on detail; 14 cents per foot for plain work would do

Skirting 9 inches high and moulding 3-inch girth. About 20 cents per foot.

Miters to ditto. About the same price each.

Enrichment, 12-inch girth. About 12 cents for every inch girth per foot run.

Moulded cornice, 15-inch girth. Price at 50 cents.

Angle 6-inch girth, and arris in Keene's cement. Worth about 10 cents.

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Staff bead 2-inch girth and quirks. About 14 cents per foot.

Moulding on ditto 4-inch girth. 16 cents per foot run.

Keene's fine quality cement, on Portland coment grounds, polished face, in narrow widths. This is priced at 75 cents, including profit, per foot.

Ditto polished, plain face, on lath partition. This may be put at about the same.

Ditto to pilasters on brick. More labor is necessary in troweling and floating the surface of diminished pilasters, and the cost would be about 75 cents per foot.

Ditto to columns. Add 14 cents to last.

Ditto in No. 2 spherical heads of alcoves 6 feet wide each. The price for these would be about the same per foot super. There would be about 28 square feet in each head.

18 feet super. Moulding to ditto polished. The price for these is about 75 cents to \$1.10 per foot; for circular work, another 20 cents may be added.

Arrises. Put at 6 cents per foot.

Moulded cornice round saloon bracketed with two enrichments, per detail. (See Fig. 16.) This cornice is run on lath, bracketed out, and the items may be put down thus:

4-meh pine brackets and Moulding per fort	plugging,	per	foo	t			.\$0,14
Two enrichments	say	• • •	•••	• •		• •	30
Add for lathing.	· · · · · · · · ·	•••	•••	••	•	•••	35
Per foot super			•••	••	•	•••	¢() (0)

Miters to ditto.

As these entail extra labor, they may be put down equal to 1-foot run of cornice, which is equal to nearly 2 feet super., say, 80 cents each.

I show several examples in decorative plastering in Figs. 11, 12, 13, 14, 15, and 16, which will give some idea of the character of work estimated on in the foregoing analysis, and aid the estimator in working out his figures.





#### MICROCOPY RESOLUTION TEST CHART

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Fig. 15.



Fig. 16.

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## EXTERNAL PLASTERING, PORTLAND CEMENT

Work in Portland cement is costly, as both the material and labor are heavy. Portland cement is mainly used in external work. Plastering will take 3 of sand to 1 of cement, except in reservoir or hydraulic work, when it is used nearly neat. It is generally sold by the bag of 100 pounds in place of the bushel, and its price is variable according to the quality and locality. Price lists should always be kept on hand. A gritted face is better for Portland cement finish than troweled work, as the latter is apt to blister. One bushel of cement and two of sand will cover 41 yards 2 inch thick; one of cement to three of sand will cover about  $6\frac{1}{2}$  yards of that thickness. So that practically we may say that one-sixth of a bushel of cement will cover one vard at the above thickness, worth about 12 cents; and three bushels of sand will cost 60 cents, one-sixth of which will be also about 10 cents; add to which labor and profit, about 37 cents, making about 60 cents per yard. The items should clearly state whether the cement is "rendered" or "plain face," or "render and float," or "troweled," if on brick or lath. Mouldings, grooves, arrises are taken per foot run if not exceeding 12-inch girth; if above this, by foot super.; and all narrow widths, like panels, rustics, etc., should be stated, as the labor is greater.

#### OUTSIDE PLASTERING

Troweled rough stucco, with blue lias lime and good sand, and jointed. This is worth about 50 to 55 cents per yard.

Moulded cornice ditto, 2-foot 6-inch girth. May be priced at 28 cents per foot super.

Portland cement, weathering, dubbing, etc. The labor would be about 7 cents, the material about the same, say, 14 cents per foot.

78 yards super. Portland cement, plain face. If 1 of cement to 3 of sand, the cost would be about 55 cents.

Ditto in narrow widths. About 10 cents per foot super.

Ditto 14-inch thick rustics in narrow widths. This may be put at the same, as these rustics are in narrow widths between grooves. This would be for the plain face; vermiculated work costs more.

Rough-cast made with clean washed sand and shingle and good lime in proper proportions.

Say, materials Labor	per	yard	super	•••	•••	· · .	• • •	• • • • •	0.08
	••••	••••	••••	•••	•••	•••	• • •	• • • • • •	.30
									\$0.38

Rough-cast on brick, with washed sand and shingle and Portland cement. Add 14 cents to former item, for a yard of cement  $\frac{3}{4}$  inch thick will take about one-sixth of a bushel, say, 12 cents; add for extra labor 3 cents.

#### PORTLAND CEMENT WORK

460 yards super. Portland cement, plain face on brick, floated; 1 of cement to 3 of sand.

Say, cost of rendering (cement and say Labor to ditto	nd)\$0.25
Floating, add 14 cents	·····
Profit, add	\$0.53
	<b>\$0.56</b>

The price-books put down for this work 65 cents per yard, which is high for a large quantity.

Ditto in narrow widths. Price at about 10 cents per foot super., or about 90 cents per yard super.

Ditto plain face on brick-jointed. This may be put down at above price, or

Plain face	
find for jointing	
	\$0.58

and 80 cents for circular.

Plain face ditto as plinth. This may be put down at 8 cents per yard more than last, or, say, 90 cents.

Moulding, etc., ditto, 9-inch girth. Price at 35 cents per foot.

Plain cement face to pilasters, etc. Worth about 10 cents per foot.

4½-inch reveals to windows and arris. May be priced at 8 or 10 cents per foot.

Vermiculated work according to sample for quoins. Worth about 55 to 70 cents per foot. This will be extra on the plain face before taken.

Quoins 18 inches long, 12-inch return, 12 inches in height, including dubbing out and projecting 1 inch from face. These are plain, and would cost about 15 cents each.

Returned and mitered ends to moulding. The cost of these may be put at the price of 1-foot run of moulding; a sketch should be given. Put at about the same.

Portland cement plain face. This is worth about 55 cents per yard (see previous items).

14 inch thick in rustics. This is chiefly for labor, and may be worth from 55 to 85 cents or more per foot, but the price depends on the class of work.

Moulded grooves to rustics, as per detail. This will be worth from 8 to 14 cents per foot, according to girth.

Miters to ditto. Worth about 10 cents each.

Rustic grooves, V-shaped. These are simpler than moulded, and the labor is less, say, 5 to 8 cents per foot run.

Miters to ditto. Put these at 3 cents per inch girth.

Portland cement cornice, per detail. If the moulding is plain, the work may be done for 38 cents per foot.

Ditto in short lengths. Add 10 cents per foot. Miters. Say, 42 cents each.

Floating beds on concrete for tile paving.

Say, cost of cement Labor, say	and	sand,	ete.,	• • •	•••	 \$0.20
	• • • • •	•••••	• • • • •	•••	• • •	 25
						\$0.45

Floating beds on concrete for wood-block paving. This may be priced the same.

Portlan cement laid as paving, 2 inches thick. The cost would be about 5 cents for materials, 8 cents for labor or 13 cents.

Selenitic cement grounds for Keene's cement. This cement forms a good ground, and can be worked to a smooth face. Obtain price and instructions from the manufacturers or dealers.

#### FORTLAND CEMENT

Angle 8-inch girth, and arris. If circular add 50 per cent to previous prices.

Splayed angle, 8-inch girth, and artis. This may be priced at the same as the other angle.

6 inches by  $\frac{1}{2}$  inch square skirting and dubbing out. 6 inches of plain face in narrow widths 50 cents; add 3 cents for arris and narrow retu 4nd add 3 cents for dubbing; 3 $\frac{1}{4}$  inches in all.

Miters. Price these at 1-foot run of the skirting.

Stops. Price these at one-half the last item.

Moulded skirting 9 inches high and 1 inch projection, and dubbing out. Say, 4 cents for plain face, 4 cents dubbing and 5 cents for small moulding.

Ditto, ditto, raking, and ramped over steps and risers. Price this at 50 per cent on last price.

Both these prices are rather high.

Internal miters. These are worth 1-foot run of the straight skirting; but the above prices are sufficient to cover the cost of miters and other extra labors.

External miters. These are of the same value. Stops. These are worth half a miter.

#### TILING

The cost of tiles and tiling can only be accurately ascertained by first getting price of tiles and cost of labor in laying them. These are laid in different ways; a door boarding, on cement, or on laths or battens. The latter method is that generally employed. Tiles, in shape, are of two main classes; those which, like pantiles, interlock, and those which, like common plain tiles, are nearly flat, and are laid on the same principle as slates. In the former class innumerable

forms have been patented, but few of them get into general use, chiefly owing to difficulties of replacing when broken, and the trouble of fitting them to irregularly-shaped roofs. Plain or crown tiles are such as have a rectangular form and plane surface. A custom is supposed to regulate size, but they are generally 102 inches long, 61 inches broad, and 1 inch thick, with two holes in them, through which oak pins are inserted to hang upon the laths. Sometimes cast-iron pegs are used instead, or frequently extra large flatheaded wrought nails, made of pure zinc or zinc and copper, which have the advantage of allowing a tile to be replaced from the inside of the roof by lifting up the others to place in the tile and drop in the nails in a few seconds. Sometimes, also, tiles have projecting nibs cast on in lieu of pegs, or they may be both holed and nibbed, so that if the nib is broken off, the tile may be nailed. In use, one tile laps over another, and that part which then appears uncovered is called the gauge of the tiling, likewise known as the fade or Many tilers have a practice, when plain tiles a mortar, not to peg more than one hole in .... . sometimes only every third or tenth course is te. This is bad, as with the decay of the mortar, nailed. the tile will slip down. For walls, battens, nailed or plugged to walls, are the best mode of fixing for vertical tile-hanging, the top of each tile being bedded in cement mortar, and the bottom double course bedded and pointed in cement on a tilting fillet.

In dealing with tiles as a roof covering, the first thing to be sure of is that the tile selected is capable of excluding all damp, and will withstand the disintegrating influence of the weather.

Pantiles are the commonest class of tiles, and are

very cheap. They hold moisture a long time, and require extra strong roof timbers. They are best laid to a slope of about 24 degrees, and are mostly used for covering sheds, barns, and buildings which do not require a plastered ceiling.

Plain tiles are smaller than pantiles, but being laid with more lap, are heavier per square. They can be laid to any slope from 25 to 60 degrees.

Fancy roofing tiles are similar in many respects to plain tiles and are much used for external walls of half-timbered houses in some countries.

Roofing tiles are subject to the same defects as terracotta, viz., if they are burnt thoroughly many of them twist and warp and will be found to be untrue, and if they are not burnt very hard they are liable to decay.

A good tile should be well tempered, of good color, free from stones, carefully trimmed, should give forth a clear ringing sound, and take its weathering quickly.

The characteristics of a good roofing tile are density, toughness, and incipient vitrification, the last named quality producing, to some extent, that pleasing tint familiarly known as "bloom," one of the peculiarities of some makes of tiles.

Among the best are Jersey tiles, the color of which varies from pale strawberry red to dark brindle (a deep reddish brown), or even to blue, through an almost infinite gradation of color, so that almost any color can be obtained. They get their weathering quickly, and are not porous.

Sizes of tiles. Plain tiles,  $10\frac{1}{2}$  inches by  $6\frac{1}{2}$  inches by  $\frac{1}{2}$  inch, and weigh about  $2\frac{1}{2}$  pounds each, and 11 inches by 7 inches by  $\frac{4}{5}$  inch, and weigh about 3 pounds each. Pantiles are  $13\frac{1}{2}$  inches by  $9\frac{1}{2}$  inches by  $\frac{1}{2}$  inch, and weigh about  $5\frac{1}{4}$  pounds each.

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A square of roofing requires 800 plain tiles laid to a 3-inch gauge, 700 tiles laid to a 3}-inch gauge, and 600 tiles laid to a 4-inch gauge; and 180 pantiles laid to 10-inch gauge, 164 pantiles laid to 11-inch gauge, and 150 pantiles laid to 12-inch gauge.

One square of plain tiles weighs about 15 cwt., an.l one square of pantiles about 8 cwt.

Spruce tiling laths or battens are  $1\frac{1}{2}$  inches by  $\frac{3}{2}$  inch to 2 inches by 1 inch, and oak tiling laths  $1\frac{1}{2}$  by  $\frac{1}{2}$  inch.

100 plain tile laths 5 ft. long.....1 bundle

12 pantile laths, 10 ft. long.....1 bundle

One square of tiling requires 1 bundle of laths, 12 hundred of nails, 1 peck of tile pins, and three hods of mortar.

One square pantiling requires 1 bundle of laths and 14 hundred of nails.

To ascertain the comparative merits of tiles, as to their weathering properties, there is no better test than the amount of water they will absorb.

Most roofing tiles are slightly absorbent, except in the case of highly-fired brindled and blue tiles, and for this reason old tiles have, in a few isolated instances, attained to a higher market value than new tiles, as by age and atmospheric deposit they have acquired an artificial surface coating and lost the property of absorption, at least on their outer exposed surface.

Tiles of a bright red, or an earthy red, color should be viewed with suspicion and avoided. They are invariably absorbent, and will not weather well. Tiles may be obtained of almost any color.

Well-formed roofing tiles are straight in their width and hollow in their length, that the tails of each course may lie close and tight on the backs of the under course.

Straight tiles will not clear themselves one over the other, and should therefore be rejected. Where pointing is necessary, it is customary in good work to grind down some of the broken tiles, to mix with the Portland cement as a substitute for sand, that the finished pointing may approximate in color to the general tone of the roof covering.

The gauge of tiling is the distance from head to tail minus the lap divided by 2; thus a 10<sup>3</sup>-inch tile laid to a 2<sup>1</sup>/<sub>2</sub>-inch lap will only expose 4 inches of its length to the view when the work is completed.

#### FIXING

Old-fashioned tiles have no nibs or stubs for hanging, and must therefore be kept *in situ* by means of two wooden pins or nails.

When tiles are bedded in lime and hair mortar the tops only should be bedded, the mortar extending, say, not more than 3 inches below the head of the tile.

When a roof is close-boarded (and sometimes felted) there is no need for bedding, though of course a covering of bedded tiles ', less liable to breakage when a man is climbing about a roof than would be one of unbedded tiles.

In tiling roofs it is well to cover them with roof boarding and felting before laying the laths and tiles. This should always be done in good work. Occasionally architects are compelled, owing to the cost of work, to eliminate the felting, or covering with 2-ply paper, and lay the tiles on the boarding; but this practice is unwise, as experience shows, and the boards alone do not stop draughts.

Secret gutters should only be used in positions

where they will always clear themselves, especially if the dwelling be surrounded by trees.

Tiles, and tile-and-a-half, should be worked against all secret and other gutters, where practicable, alternately on each course.

Tiles overhanging secret gutters should not be bedded on the leal nor should their edges be pointed, otherwise rain may be drawn into the roof.

The lead welt should stand its own thickness above the backs of the battens, forming a tilt for the tiles, so as to throw the water away from their edge on to the main body of the roof.

Ridge tiles should be of such a section as to ad..it of being pressed or made in one piece. Where an ornamental cresting is required, it should be made as a separate piece entirely detached from the ridge tile proper, the latter being made with a groove to receive the cresting.

The cresting should not be stuck on the ridge tiles by means of semi-liquid clay while they are in the c state. Such work is more or less defective and unsat ... factory in the end.

Pantiles should be laid on laths and a good bed of hair plaster, in order to secure them to the root.

Tiles hung against vertical walls are treated precisely as are those on roofs.

## HIPS, VALLEYS, AND RIDGES

In a tiled roof valley and hip tiles should be used in preference to lead gutters, secret or otherwise, bedding the valley tiles at their heads to keep them *in situ* and steady while laying the plain tiles.

Hip and valley tiles should be purpose made, with proper regard to their enclosed angle or pitch.

Where a minor roof runs in at right angles to a greater or main roof, intersecting it at a point below the main ridge, it is desirable to use a piece of 4-pound lead dressed to the shape of the minor ridge and the slope of the main roof, and called a saddle-piece. This prevents the possibility of rain getting in at the junction of the roofs.

The simplest form of ridge tile is that consisting of the two wings terminating in a roll at their angle of intersection.

Another good form of ridge tile is that of a plain vertical blade rising from the angle of intersection of the wings, and with the square angle at each end of the blade cut off at an angle of 45 degrees, and which car be pressed in one piece by a simple operation.

Ridge tiles should be well soaked before use, bedded in gauged lime-and-hair, and their vertical joints drawn up solid with cement, not simply pointed after they are fixed.

When the roof is enclosed on the under side, it is customary to bed in lime-and-hair the eaves courses only, for the sake of steadiness in the fixing.

As before stated, the cost of a tile roof will vary much according to locality and quality of materials used. The average cost per square, however, will be about \$16.00 for the best tiling and about \$12.00 for the more common kinds. While these figures are not correct, they may act as a sort of guide to the estimator when figuring on tile roofs. In all cases, however, wherever possible, I advise that the local prices be obtained and that at least 10 per cent be added to these prices, unless the work is executed in a large city where prices are more constant than in country places; then only the usual percentage of profit be added.

So little tiling is done in this country (more the pity) that expert tilers are scarce and wages high and varied, so that nothing can be given definitely regarding the cost of this work.

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In measuring for tiling, take the whole superficial area, and allow extra for eaves next parapets, 4 inches; dripping eaves, 6 inches; all hips and cuttings, 3 inches, and for valleys, 12 inches.

For pantiling, also take the whole superficial area, and at hips, take the length of the hip-rafter by 12 inches for cutting and waste, to be added to the superficial area; take the run of hip and ridges, and of mortar or cement filleting, and the plain tile heading.

Take in all cases the number of hip hooks and T nails to be painted in oil.

Secure gauge of the tile, the quantity and description of the laths and nails used; also if laid dry or pointed outside or inside with mortar or cement, and charge up accordingly; get exact cost of one square according to data given in the foregoing, and then find number of squares to be tiled, and multiply the number of squares by the cost of one square estimated upon.

#### THE SLATER

The great similarity which runs through the specifications for slaters' work, no matter by whom drawn, or for what class of work it is intended to apply them, is a mistake, as it often leads to bad results. The most suitable slate for the particular work in hand should be carefully selected.

The architect should consider the pitch it is intended to give the roof, the length of span, and also whether

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mitered or close-cut hips are to form any portion of the roof.

If the hips are to be mitered, the angle should not be less than 45 degrees, otherwise very large slates must be used at the hip, which looks unsightly, and on no account should small pieces be allowed.

Soakers should invariably be used where soft slates are laid, as flushing or bad work of any sort stains the slates and produces a bad effect.

In exposed situations, where snow may be driven over the lap, it is better that the roof should be boarded and felted. If battens are used instead, vertical ones are less liable to cause a collections of snow at certain points, and apparent leakage when that occurs.

When snow may slide off main roof on to any glass below, wire guards should be fixed along the eaves to check it. Open batten show-guttering should be provided to all V and parapet gutters to allow snow-water to get away.

Mitered hips and valleys with 4-pound lead soakers under slates make the neatest finish to slated roofs, and, if properly secured, the most satisfactory. In order to make a neat finish the roofs should be 45 degrees pitch and the slates used in such cases should be small, say  $16 \times 8$ ; the slater has then the choice of such sizes as  $16 \times 9$ , 10, and 12 to work up the hip with. It is impossible to obtain wider slates, and this often induces the slater to lay the slate lengthways to save the introduction of small pieces; the sides of the roofs forming the miter should be of the same pitch.

If additional precaution is deemed necessary, small rolls may be screwed down to the hip rafter, over the mitering; this is rather unsightly and not recommended if soakers are used.

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Slates should be nailed with copper nails, which are practically imperishable. The life of a zinc nail rarely exceeds twenty years, and iron still less.

In soft and rag slating the nails should be very stout, and the length 2 inches, 1<sup>4</sup> inches, and 1<sup>1</sup> inches; few of the latter, if any, should be used, say on the last three or four courses only; the strength should be 90, 110, and 130 to the pound respectively.

In regular-sized work such stout nails are inadvisable, as the heads are large and will not recess as readily into the slate, and the top of the head must be flush with the surface of the slate, or anything pressing on that particular part will damage the slate above; 14-inch and 14 inch nails are recommended, 180 and 250 to the pound respectively.

Gauge.-The gauge of slating is the part left exposed, viz., deduct the lap from the total length of slate and half the remainder, thus,

## $20 - 3 = 17 = 8\frac{1}{2}$

Lap for soft and rag slating should never be less than 3 inches. For regular-sized slating, 4 inches to 21 inches, according to pitch of roof.

Repairs to roofs should be done by an experienced slater and straps prohibited; the lead or zinc strap is a ready way, but raises the tail of the slate up, and is turned back by snow slipping down and slate slips

Slating on unplaned boards is preferable to that on battens, because it is more waterproof and prevents the ingress of driving snow. The cost of good quality 3-inch rough board is about \$2.00 per square as compared with 50 cents for  $2 \times 4$  inch slate battens, and the labor of laying and quantity of nails equal in each case.

In superior work heavy felt (inodorous or otherwise)

is inserted between the covering boards, and the slates or battens may be added above the felt to render the building more proof against sun heat.

Bedding and pointing on under side is not recommended unless the roofs are well ventilated; the heat of the house will condense on the under side of slate and quickly rot the wood work, and, in course of time, the slate also. Experience shows that a rough slate will keep out driving rain better than a smooth one, if well laid; the reason for this is that there is a considerable quantity of air between the surface of rough slate and practically no suction; also the thick edge of the slate breaks up the force of the wind on the surface of the slates.

In church roofs, where the pitch is very sharp, small slates are recommended, from  $14 \times 8$  to  $18 \times 9$ , according to pitch; as the pitch decreases the slate should be wider.

For roofs of warehouses, where much depends on the work being perfectly water-tight, "tin" slates are recommended; they are about  $\frac{3}{8}$  inch thick, and are large and laid in diminishing courses, the gauge being about 15 inches at the eaves and 10 inches at the ridge.

These slates are scarce, and architects should insist on the order being placed when the contract is signed, to ensure delivery in time.

If it is thought advisable to use the above-mentioned slates, sizes such as  $16 \times 12$ ,  $18 \times 12$ ,  $20 \times 22$ ,  $22 \times 12$ , and  $24 \times 12$  are suggested, the size varying according to pitch.

For curb and mansard roofs, slates larger than  $16 \times 9$ should never be used, the whole weight being thrown on the nail in such cases. The appearance of small slates is also far better on such roofs.

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With span of 25 feet to 40 feet, which entail a deep rafter and flat pitch, it is a wise precaution to very the lap, giving extra at eaves and for a third of the way up the slope; in such roofs the slates should not be less than 11 inches wide, the extra width being a safeguard against side leakage through the nail holes

Merchants are only able to obtain a proportion of sizes yielded by the rock, consequently it is sometimes impossible to fulfill the general specification of  $20 \times 10$  Countess slate, as the quarries will not sacrifice the rock to make the full demand of  $20 \times 10$  if the block will make  $20 \times 12$ ; if  $20 \times 10$  were insisted on, in that case it would entail an extra cost of about \$3.50 per square over other sizes.

Actual size has little to do with the quality of the work, the lap is the principal factor, and the result in  $16 \times 10$  or  $20 \times 10$  is exactly the same. The best allround size is probably  $16 \times 10$ .

Single samples are very unsatisfactory means of judging of the quality of the bulk; at least six should be demanded, showing medium and thinnest. Where possible, the inspection of the bulk should be made.

A good slate is hard and tough, will give a harp metallic ring when struck with the knuckles, does not split under the slater's ax, is easily holed without fracture, not tender or friable at the edges, and should contain no white iron pyrites (marcasite).

A bad slate feels smooth and greasy to the touch, absorbs moisture if stood in water, splits while being holed or trimmed at the bead, breaks when pressed upon, emits a clayey odor when breathed upon, and is liable to premature decay.

Slate ridge rolls and wings should be fastened with

brass or copper screws, and bedded and pointed in lead cement, one-third lead and two-thirds best oil putty. Iron screws should not be permitted, they oxidize and burst the rolls. If wings more than  $\frac{1}{2}$  inch thick are used the upper edges must be beveled.

Half or checker slating is sometimes employed for farm buildings or where special ventilation or cheap covering is required. The saving by this method is in the quantity of slates and nails used; the battens or boards remaining the same. In place of the slates being butted close to one another, they are spaced laterally in such a manner as to just cover the joint between the slates in the course below. This slating, known also as open slating, is well adapted for use in farm buildings, covered yards, etc., as by its construction it affords a certain amount of ventilation.

In laying slate there is always an element of risk of breakage that must be accounted for, and, as all roofs must be left in good order and perfectly water-tight, an allowance of about 25 cents per square must be made above all other provisions. It is very necessary to go carefully over the slating and see that the slater who does the work makes good any deficient or broken slates before he leaves it; and beyond that there is the risk of breakages from other wo .men, for some men must go on the roof after, although as much as possible this should be avoided.

Cutting round small ventilators, V-shaped on plain, and 12 inches by 12 inches.—If the ventilator itself measures 12 by 12, the flashing round it will, of course, exceed the dimension and the slate will not run close up; giving another foot run of cutting, the slate would have to be *tilted* against the ventilator to throw the water off, or a secret gutter formed. The eaves cutting price at 10 inches of the slating, the plain cutting at 6 inches of it.

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Cutting round 3-inch lees pipe (ventilators from soilpipes) and making good.—These are at \$1.00 each, including profit.

The following prices are given herewith as being approximately correct, being taken from the price list of the Slatington Slate Co., Slatington, Pa., but I would advise estimators to get other price lists, down to the latest date, as the prices are continually changing.

Sizes	No. 1 Price per Sqr. F. O. B. Quarries	No. 1 Ribbon Price per Sqr. F. O. B. Quarries	Sizes	No. 1 Price per 3qr. F. O. B. Quarries	No. 1 Ribbon Price per Sqr.
24 x 14 24 x 19	\$3.50	\$3.10	16 x 12	2\$3.85	
22 x 12	3.85	3.10 3.25	16 x 1(	1 4.25	3.50
22 x 11. 20 x 12	3.85	3.25	16 x 8	$3 \dots 4.50$	3.50
20 x 12. 20 x 11.	$\cdots$ 3.85 $\cdots$ 4.25	3.25	14 x 10	3.85	3.50
$\frac{20 \times 10}{10}$ .	4.25	3.35	14 x 8 14 x 7	$\dots 4.25$	3.50
18 x 12. 18 x 10.	3.85		12 x 8	3.75	••••
18 x 9.	4.40	3.50	$12 \times 7$ $12 \times 6$	···· 3.50	• • • •

BANGOR NO. 1	l E	LACK	ROOFING	ST	ATE
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Add 15 per cent to above prices.

## BROWNVILLE MAINE SLATE

No more beautiful slate is quarried in the world than the Brownville. It is very uniform in thickness and of smooth surface, and when laid on the roof presents a surface equal to polished steel. For costly private residences, churches, and public edifices, it has no superior.

Sizes		Price per Square F. O. B. Quarries		
24 x 14		No. 1	No. 2	
$24 \times 14$ ,	**************	.\$6.20	None.	
$23 \times 12$	• • • • • • • • • • • • • • • • • •	6.20	\$4.95	
24 X 14		6.10	None	
$\frac{22 \times 12}{12}$ , $22 \times 11$		6.30	4 05	
20 x 14.		6.50	None	
20 x 12, 20 x 11, 20	x 10	6 70	Aone.	
18 x 14		6 45	4.95	
18 x 12, 18 x 11		0.40	None.	
18 x 10, 18 x 9		0.40	4.75	
16 x 12, 16 x 11	•••••••••••••••	7.20	5.20	
16 x 10, 16 x 9, 16 x	· · · · · · · · · · · · · · · · · · ·	7.00	5.00	
$14 \times 12$ $14 \times 10$ 14		7.20	5.20	
$14 \times 7$	x 9, 14 x 8	$6 \ 45$	4.60	
$12 \times 10$ 12 0 10	• • • • • • • • • • • • • • • • • • • •	6.25	4.50	
$12 \times 10, 12 \times 9, 12 \times 10, 10 \times 10, 10$	$8, 12 \times 7$	6.00	4.10	
$12 \times 0, 11 \times 8, 11 \times 7$	7	5.20	3 15	
10 x 8		5.00	3.15	
9 x 7		4 00	None	
4114		1.00	Mone.	

Add 15 per cent to above prices.

## GREEN, PURPLE AND RED ROOFING SLATE

For ornamental roofs these colors are in steady demand. They are also used for entire roofs in many instances.

	Unfading Green. Price tur	Purple.	Red.
Sizes	Square F. O. B.	Square F. O. B.	Square F. O. B.
24 x 14 24 x 19	Quarries	Quarries	Quarries
$99 \times 14$ 99 10	•••••\$3.50	\$4.00	
$20 \times 14, 22 \times 12$	3.50	4.00	
$20 \times 14$ , $10 \times 12$ , $16 \times 12$	$2 \dots 3.50$	4.00	
$18 \times 11$ , 20 x 12, 20 x 1]	3.75	4.25	
14 - 10 - 14 - 0	3.75	4.25	•••••
$14 \times 10, 14 \times 9$	3.75	4.25	\$11 00
$16 - 10$ , $18 \times 10$	· · · · · · 4.00	4.50	11 00
10 X 10.	4.00	4.50	11.00
$14 \times 8, 14 \times 7$	4.00	4 50	11.00
$12 \times 10$	3.25	3 50	11.00
12 x 8.	3.25	3 50	0.95
$12 \times 7, 12 \times 6$	3.25	3 95	9.20
$18 \times 9$ , $16 \times 9$	4 00	4 50	9.25
16 x 8.	1 00	4.50	11.00
	4.00	4.00	11.50

To these prices add 20 per cent.

### PEACH BOTTOM SLATE

Sizes	Price per Square
$20 \ge 10, 18 \ge 10, 18 \ge 0$	r. O. B. Quarries
$16 \ge 9, 16 \ge 8$	•••••\$5.60
16 x 10 10 11 10	
$10 \times 10, 10 \times 11, 18 \times 11$	
$18 \ge 12, \ 20 \ge 11, \ 20 \ge 12$	0.00
20 x 13, 22 x 11, 22 x 12	5.50
22 x 13, 22 x 14, 24 x 12	
24 x 13, 24 x 14	$\cdots \cdots 5.50$
$24 \times 15$ $24 \times 16$	5.35
11 - 7 $14 - 9$ $-1$	
$14 \times 7$ , $14 \times 8$ , $14 \times 9$ , $14 \times 10$	F 0F
$12 \times 6$ , $12 \times 7$ , $12 \times 8$ , $12 \times 0$ , $10 \times 10^{-10}$	10
11 x 5, 2-inch lap.	10 4.75
$11 \times 6$ $11 \times 7$ $11 \times 6$ $0 \times 1$	$\cdots \cdots \cdots \cdots \cdots \cdots \cdots 3.50$
10, 11, 11, 11 x 8, 2-inch lap.	3.75
$10 \ge 5$ , 2-inch lap	0.10
10 x 6, 10 x 7, 10 x 8, 2-inch lan	3.25
Strictly 3-16 juch in thistory	••••••••••••••• 3.50
Four to the to 1 to the Concess	
Tom to the mch m rick.	7 50
Strictly 1 inch in thickness	1.00
Drilling and counterright -	9.00
s und countersinking, 50 cts.	Der square extra

#### NO. 2

All sizes above	16 inch
	16 inch
	14 inch. 3.40
	12 irah
	9 75

The peach bottom slate is one of the best in the country; it is almost everlasting, never loses its color and is non-absorbent.

Add from 10 to 12 per cent to above prices.

### SEA GREEN ROOFING SLATE

This is extensively used in many of the Western States. The color is not permanent, but it is strong and durable. For low-cost buildings it is a favorite in many localities and while the color changes, the dura-

bility of the material does not seem to suffer. It makes a good all-round slate roof.

Sizes	Price per Square F. O. B. Quarries	Sizes	Price per Square
24 x 14		10 - 10	rio, p. Quarries
24 x 19	0.10	10 X 12	\$3.00
00.1.	····· 3.10	16 x 10	
22 x 14	· · · · · · · · · 3.00	16 x 0	0.00
22 x 12.	3 10	10	···· 3.00
22 × 11	0.00	10 X 8	2.90
20 A 11	$\cdots \cdots 3.20$	14 x 10	. 9.00
20 x 12	· · · · · · · · 3.10	1.1 v 0	
20 x 10	3 20	14 0	$\cdots 2.90$
10 - 10		14 X 8	
10 X 12	3.10	14 x 7	9.70
18 x 10		19	2.70
18 x 9	3.10	12 X 8	$\cdots 2.70$

Add from 5 to 10 per cent to these prices.

To obtain the correct measurement of a surface of a slate when laid, and the number of squares on any particular surface, we simply subtract the lap from the length of the slate and half of the remainder will give the length of the surface exposed, which, when multiplied by the width of slate, gives = surface sought; so that to obtain the exact number of slates of any description required to cover any given surface is quite a simple matter. Further on I will give a rule for finding the number of slates required for covering any given area.

The following table gives the weight of slates of different thicknesses per square foot super.

Slate  $\frac{1}{16}$  of an inch thick, 2.71 pounds per square foot. Slate  $\frac{1}{4}$  of an inch thick, 3.62 pounds per square foot. Slate  $\frac{3}{8}$  of an inch thick, 5.43 pounds per square foot. Slate  $\frac{1}{2}$  of an inch thick, 7.25 pounds per square foot. Slate  $\frac{3}{8}$  of an inch thick, 9.06 pounds per square foot.

Slate 4 of an meh thick, 10.87 pounds per square foot. Slate inch thick, 14.5 pounds per square foot. Slate 14 inches thick, 18.64 pounds per square foot. Slate 14 inches thick, 22.48 pounds per square foot. Slate 2 inches thick, 30.00 pounds per square foot.

There are certain rules that are generally recognized by estimators and builders for the measurement of roofs, whether of slate, shingles or other materials, and may be given as follows:

For plain roofs, measure the length of the roof and multiply by the length of the rafter.

For roofs with hips, valleys, gables, dormers, etc., measure each section through the center and multiply by length of rafter, and in addition to the actual sur-

face of the roof, measure the length of all hips and valleys, by one foot wide. No deduction is made for dormer windows, skylights, chimneys, etc., unless they measure more than 4 feet square. If more than 4 feet square, and less than 8 feet square, deduct one-half; if more than eight feet square, deduct the whole. If hips are mitered, charge extra. Ridge rolls, flashings, valleys, etc., are charged extra.

The names given to ornamental slates and shingles are



known by the trade and workmen as given and shown in Fig. 17; the examples are among those most used.

The expert slater, at the present writing, receives

\$4.50 per day of nine hours, and he is supposed to lay about 14 squares, providing everything is handy for him. This wage, however, may be, and is, only given in a few localities. In some towns it is less, and in some cities it is more; so that the estimator should, whenever possible, obtain local prices both for labor and material; then he cannot well make many errors in his estimate if he is at all careful.

The following hints regarding preparing estimates may not be out of place, and I give them even if I risk being accused of repetition, as I have given nearly the same advice in previous pages; but it cannot be given too often to the young and inexperienced estimator, for the omission of a single item may result in grievous loss.

The first thing to do, before commencing to make an estimate of the cost of a job of work, is to see to it that the drawings and specifications be carefully studied and remarks made for future guidance or reterence.

**Excavator and Bricklayer.**—Take the dimensions for the excavations wholly from plans and sections, then refer to the specification and add whatever does not appear on the drawings.

Take brick work as above directed.

The chimney bars may be taken with the dimensions of the breasts; refer to specifications for description.

The centering and spring pieces may be taken with arches.

The wood bricks and springing pieces may be taken with brick arches.

Take dimensions by the foot run of the making good and restorings of all stone sills, after mason, etc.

**Slater.**—Take from the plan of roof and section, then refer to specification and clear all off.

In the bill provide for leaving all slating perfect at completion of the work.

**Carpenter.**—Take from plans and sections, referring to specification. Take all iron attached to timbers. Find the quantity of boarding, or battening, to roof, with the slating, deducting eaves, cuttings, etc., from the latter; if much discrepancy, there must be an error.

**Plumber.**—Take everything from specification, referring to drawings only for lengths. Be careful in allowing all turnings up under slating and against walls, round rolls, flashing, etc. (refer to the quantity of boarding to flats, etc., as a check.)

Mason.—Take from specification, referring to the drawings only for dimensions. Attend to the cube quantities, scantling lengths, etc., also to the method pointed out for taking labor.

Joiner and Hardware.—Peruse specification, referring to drawings only for dimensions. Take hardware from floor to floor. Provide for casing stairs, and covering handraii to prevent any injury during the progress of the work, and for projecting masonry in like manner.

Provide for attending plumbers to sinks, cisterns, W. C., etc., stating how many of each.

**Plasterer, Internally.**—Look carefully to specification, particularly as to enrichments, referring to drawing for dimensions. Whitening and coloring is taken from plastering, but appears separately.
Provide for making good round mantels after mason.

**Glazier**.—Can find all in specification, referring to drawings for any size. Check quantity of glass by the sashes, allowing for wood. Provide for leaving same clean and sound.

Painter .- All taken from specification.

All wood work painted may be collected from the joiner; one-seventh for edges; when both sides are painted, double dimension. Painting for plastered walls, from plasterer.

Paper-Hanger.-May be taken from plasterer.

Summary of trades in order. Conditions of contract to be taken from specification, and furnished in the memorandum sheet.

Fees. —Government, municipal, sewer, and architect's fees to be attached at end.

At the head of each trade give fair description from specification of quality of materials, etc.

**Lastly**.—Generally review the whole of the drawings and specifications, that nothing may be omitted or misrepresented.

### RULES, TABLES, NOTES, DATA, AND POINTERS USEFUL TO THE ESTIMATOR

The following tables, data, etc., have been specially selected for the use of the estimator, and will be found useful for reference and for making hurried approximate estimates of work in detail or in bulk. The items are carefully indexed, so that any particular one

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of them may be found without much loss of time; a matter of considerable importance to the busy man. It would extend the limits of this book too far beyond the size intended to insert tables of scantling measurement, wages, extended tables of diameters, circumferences and areas of circles and similar matter, which after all are not of much actual service to the estimator, but which are usually published in works of this kind.

The average weight of medium and heavy cast-iron drain pipes are given in the following tables, viz.:-

Dlame- ter of Pipe	Length exclu- sive of socket	Thick- ness of Metal	Depth of Socket	Ave	rage w per plj	elght pe	App	roxim ight p	ate er
$ \begin{array}{c} \text{in.} \\ 4 \\ 4\frac{1}{2} \\ 5 \\ 6 \\ 7 \\ 8 \\ 9 \\ 10 \\ 2 \\                                      $	ft. 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	11 12 35 38 38 39 32 4 12 7 6 12 12 12 12 12 13 12 36 7 3 4 15 8 17 18 7 1 17 6 12 7 6 12 7 2 12 7 2 12 7 3 7 2 5 5 8 7 3 4 15 8	$\begin{array}{c} \text{in.} \\ 4 \\ 4 \\ 4 \\ 4 \\ 4 \\ 4 \\ 4 \\ 4 \\ 4 \\ $	ewt. 1 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2	$\begin{array}{c} qr. \\ 1 \\ 1 \\ 2 \\ 3 \\ 0 \\ 1 \\ 2 \\ 3 \\ 1 \\ 1 \\ 2 \\ 0 \\ 2 \\ 2 \\ 0 \\ 2 \\ 0 \\ 2 \\ 0 \\ 2 \\ 3 \\ 2 \\ \end{array}$	$\begin{matrix} lb.\\ 12\\ 20\\ 14\\ 0\\ 0\\ 0\\ 7\\ 14\\ 0\\ 0\\ 24\\ 7\\ 0\\ 0\\ 24\\ 7\\ 0\\ 0\\ 24\\ 0\\ 14\\ 7\\ 14\\ 14\\ 14\end{matrix}$	$ \begin{array}{c}     for \\       cwt. \\       0 \\      $	$\begin{array}{c} \text{ot run} \\ \hline qr. \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1$	$\begin{matrix} \text{lb.} & 17 \\ 18 \\ 20 \\ 22 \\ 25 \\ 0 \\ 3 \\ 7 \\ 8 \\ 12 \\ 18 \\ 23 \\ 0 \\ 6 \\ 15 \\ 19 \\ 26 \\ 14 \\ 18 \\ 25 \end{matrix}$

## WEIGHT AND THICKNESS OF CAST-IRON DRAIN-PIPES

n				17.9.9	TATTAL TAL					
Per sq.				1 H	ICKNES	S OF W	ALLS			
100t	4 <u>4</u> m	10.1		118	1	1.00.1				
of wall [	or <del>1</del>	9 in. or	13 in. or	1 10 10	22 in or	20 m,	20 :	35 in.		111 inch
1	brick	1 brick	11 brick	012	94 brief	or 3	ou in, of	or 4	[39 in, or	TT Inch
It in				DFICK	a 2 DITCK	brick	32 brick	brick	41 brick	oro
0.4		-		1						DEICK
0.0	31	7	104	14	171	0.1		1		
1.0	7	14	21	28	252	41	241	28	314	35
1.6f	$10\frac{1}{2}$	21	314	49	501	42	49	56	63	70
2.0[]	14	28	19	20	021	62	734	84	9.11	105
2.6	174	35	591	30	70	84	i 98"	112	196	100
3.0	- 21 1	49	041	10	$-87\frac{1}{2}$	105	1224	140	1571	140
3.6	2.11	42	0.3	84	105	126	147	169	10/2	175
4 0	002	40	132	98	1224	147	1711	100	189	210
1 6		50	84	112	140	168	1002	190	2201	245
5.0	012	03	941	126	1571	180	- 190	224	252	280
0.0	30	70]	105	140	175	210	2201	252	-2831	315
0.0	384	77	1154	154	1021	0.01	245	280	315	350
0.0	42	84	126	168	210	2.51	2691	308	3461	385
0.6	451	91	1364	182	- 210	252	294	336	378	420
7 0	49	98	147	100	2212	273	318]	364	4001	155
7.6	524	105	1551	910	245	294	343	392	4.11	100
8.01	56	112	165	210	2021	315	3674	420	4791	490
8.61	591	110	1751	224	280	336	392	418	501	025
9.0	63	196	1602	238	$297\frac{1}{2}$	357	-1161	176	5951	560
9.6	Gal	120	189	252	315	378	441	50.0	0001	595
10.01	707	100	1993	266	3321	300	4651	520	507	630
15 0	100	140	210	280	350	420	100	032	5981	665
20.01	105	210	315	-120	525	620	700	360	630	700
20.0	140	280	420	360	700	810	(33)	840	945	1050
30.0	210 +	420	630	810	1050	1960	980	1120	1260	1400
40.0	280	560	840	1120	1 100	1200	1470	1680	1890	2100
50.0	350	700 1	050	1.100	1750	1080	1960 [	2240.1	2520	2800
60.0	120	840 1	260	1600	1700	2100	2450	2800	3150	2500
70.0 4	90	980 i	170	1000	100	2520	2940	3360	1780	1900
80.0 5	60 1	120 1	200 1	1900	2100	2940	3430 🗍	3920	1110	1000
90.0 0	30 1	260 1	800 1	2240 2	2809	3360; ;;	3920	4480		1900
100.0 7	00 1	100 9	100 2	2520 3	3156 + ;	3780 4	4410	50.10	650	0000
200 0 114	00 3	400 2	100   2	800 ;	3500   .	1200 4	1900	5600	010	300
300 0 21		300 4	200 5	5600  7	'000 E s	8400 d	1800 11		300 7	000
400 0 20		200 6	300   8	3400 10	500 119	600 14		200 12	600 14	000
500 0 20		000 8	400  11	20014	000 11	800 10		800 18	900 21	000
600.0 35	00 7	000 10	500 14	000 17	500 121	000 19	500 22	400 25	200 28	000
200.0 42	00 8	400 12	600 lin	800 21	000 15	200 24	000 28	000 31	500 35	000
100.0 49	00 9	800 14	700 119	600 5.	300 20	-00 29	400 33	600.37	800 42	000
800.0 56	11 00	200 168	800 22	100 50	000 29	-00.34	300 39	200 44	100 49	000
900.0 630	00 12	600 189	00 55	200 20	500 33	000.39	200   44	800'50	400 56	000
000.0 700	0 14	000 216	100 50	000.31	300 37	800.44	100  50	400 56	700 63	000
			28	000.35	000 [42	0001-400	000 56	000'63	000 70	000

#### TABLE SHOWING NUMBER OF BRICKS IN WALLS OF VARIOUS THICKNESSES

Brick work is generally measured by 1,000 bricks laid in the wall. In consequence of variations in size of bricks, no rule for volume of laid brick can be The following scale is, however, a fair exact. average:

7 compressed bricks to a super. foot 4-in. wall.

14 compressed bricks to a super, foot 9-in, wall. 21 compressed bricks to a super, foot 13-in, wall. 28 compressed bricks to a super, foot 13-in, wall.

35 compressed bricks to a super. foot 22-in, wall.

Corners are not measured twice, as in stone work. Openings over 2 feet square are deducted. are counted from the spring. Fancy work counted  $1\frac{1}{2}$ bricks for 1. Pillars are measured on their face only.

A cubic yard of mortar requires 1 cubic yard of sand and 9 bushels of lime, and will fill 30 hods.

nch 5 ck

3705050505050505050505050505050

One thousand bricks closely stacked occupy about 56 cubic feet.

One thousand old bricks, cleaned and loosely stacked, occupy about 72 cubic feet.

One superficial foot of gauged arches requires 10 bricks.

Pavements, according to size of bricks, take 38 brick on flat and 60 brick on edge per square vard. on an

Five courses of brick will lay 1 foot in height on a chimney; 6 bricks in a course will make a flue 4 inches wide and 12 inches long, and 8 bricks in a course will make a flue 8 inches wide and 16 inches long.

### SAFE BEARING LOADS

### BRICK AND STONE MASONRY

Brickwork

Bricks, hard, laid in time me Lbs, per so it	
Hard, laid in Portland mortar	
Hard, laid in Rosendale ement mortar	
Masonry 150	
Granite, caristono	
Squared stonework 700	
Sandstone, canstone 350	
Squared stonework 350	•
Rubble stonework last in 175	
Rubble stonework loid in time mortar	
Linestone, capstone, raid in cement mortar 150	
Squared stonework 500	
Rubble, laid in line worter	
Rubble, laid in cement morter 80	
Concrete, 1 Portland, 2 smd 5 has	
oundation Soils Dand, 5 broken stone 150	
Rock, hardest in native hed Tons per sq. foot.	
Equal to best Ashlar masoury 100-	
25-40	

Foundation Soils		
Faul to hast h 1	Tons	per sq. foot.
riqual to ber brick.		15-90
Ulay, dry, in thick beds.		1 0
Moderately dry, in thick hode	• • • • •	. 4-0
Soft		. 2-4
Crossel as 1		. 1-2
Sand compares sand, well cemented		. 8-10
Clean dry	• • • •	. 4-6
Oujekeend ollustist with	• • • • •	. 2-4
Quicksand, and via sons, etc		5-1

#### EXCAVATIONS

Excavations are measured by the yard (?? cubic feet), and irregular depths or surfaces are generally averaged in practice.

#### MASONRY

Stone masonry is measured by two systems, Quarryman's and Mason's Measurements.

By the Quarryman's Measurements the actual contents are measured; that is, all openings are taken out and all corners are measured single.

By Mason's Measurements, corners and piers are doubled, and no allowance made for openings less than  $3 \times 5$  feet and only half the amount of openings larger than  $3 \times 5$  feet.

Range work and cut work is measured superficially and in addition to wall measurement.

An average of six bushels of sand and cement per perch of Rubble Masonry.

Stone walls are measured by the perch  $(24\frac{3}{4}$  cubic feet). Openings less than 3 feet wide are counted solid; over 3 feet deducted, but 18 inches are added to the running measure for each jamb built.

Arches are counted solid from their spring. Corners of buildings are measured twice. Pillars less than 3 feet are counted on 3 sides as lineal, multiplied by fourth side and depth.

It is customary to measure all foundation and dimension stone by the cubic foot. Water tables and base courses by lineal feet. All sills and lintels or ashlar by superficial feet, and no wall less than 18 inches thick.

The height of brick or stone piers should not exceed 12 times their thickness at the base.

Masonry is usually measured by the perch (containing 24.75 cubic feet), but in practice 25 cubic feet are considered a perch of masonry.

Concreting is usually measured by the cubic yard (27 cubic feet).

A cord of stone, 3 bushers of lime, and a cubic yard of sand, will lay 100 cubic feet of wall.

Cement, 1 bushel, and sand, 2 bushels, will cover  $3\frac{1}{2}$ square yards 1 inch thick;  $4\frac{1}{2}$  square yards  $\frac{3}{4}$  inch thick, and  $6\frac{3}{4}$  square yards  $\frac{1}{2}$  inch thick; 1 bushel of cement and 1 of sand will cover  $2\frac{1}{4}$  square yards 1 inch thick, 3 square yards  $\frac{3}{4}$  inch thick, and  $4\frac{1}{2}$  square yards 2 inch thick.

THE PROPORTION OF STOCK BRICKS AND MORTAR TO A ROD OF BRICKWORK

$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	Inickness of Mortar Joints	Gauge or Height of 4 Courses	Cubic Feet of Bricks	Cubic Feet of Mortar	Number of Bricks
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		$ \begin{array}{c} 121\\ 12\\ 111\\ 121\\ 121\\ 121\\ 12\\ 111\\ 12\\ 111\\ 1 \end{array} $	258 257 256 237 236 234	58 59 60 79 80 82	4180 4350 4540 4010 4176 4176

Bricks absorb about 15 of their weight of water.

A bricklayer's hod measures 16 in. x 9 in. x 9 in. = 1296 cubic inches.

Ditto will hold 20 bricks.

Ditto, ditto 2 ' abic foot of mortar.

Ditto, ditto ½ bushel nearly.

The proportions of lime, sand, or eement required for a rod of briekwork are:

Of white stone lime	26)
Sand	78 Cubie Feet.
Gray lime	36)
Sand	72 Cubie Feet.
Blue lime	381
Sand	77 <sup>f</sup> Cubie Feet.
Roman or Portland eement	45)
Sand	45 ∫ Cubie Feet.

One rod of brickwork requires 126 gallons of water to slake the lime and mix the mortar.

A load of Mortar = 1 eubic yard, and will fill 30 hods.

Mortar produced

1 imperial bushel of blue lime, unslaked, weigh-
ing 70 lbs)
2 imperial bushels of sand, weighing 103 lbs 2.75
6½ gallons of water)
1 imperial bushel of blue lime, unslaked
3 imperial bushels of sand
7 <sup>1</sup> / <sub>2</sub> gallons of water)
1 imperial bushel of Portland eement, weighing
99 lbs
1 imperial bushel of sand, weighing 103 lbs $1.75$
3 <sup>3</sup> gallons of water
1 imperial bushel of Portland eement
2 imperial bushels of sand
51 gallons of water
1 imperial bushel of Portland eement
3 imperial bushels of sand
67 gallons of water
1 imperial bushel of Roman eement, weighing 72)
lbs
9½ gallons of water)
Note:-The mortar produced weighed 106 lbs.
1 imperial bushel of Roman cement
1 imperial bushel of sand (103 lbs)
9½ gallons of water
Note:-The mortar weighed 196 lbs.
2

Concrete produced

1 imperial bushel of Portland cement..... In cubic feet

imperial bushel of sand.....

41 gallons of water.....

Lime and sand, and cement and sand lose about one-third of their bulk when made into mortar.

Lime, or Portland cement, and sand require to mix into mortar about one-third of their bulk of water.

Brick nogging requires-

Per yard superficial, 45 stock bricks laid flat.

Per yard superficial, 30 stock bricks on edge.

Per yard superficial, <sup>3</sup> cubic foot mortar when flat.

Per yard superficial,  $\frac{1}{2}$  euble foot mortar on edge.

THE NUMBER OF BRICKS AND QUANTITY OF BRICKWORK IN WELLS AND CYLINDRICAL SEWERS FOR EACH FOOT IN DEPTH OR LENGTH

	HALF BRICK THICK		11	ONE DURIN T			
	Number	of Bricks	1		ONE BRICK THICK		
	Tabl	I T I I I	Cubic Feet	Numbe	er of Brick	S Cubic D	
	Dry	Laid in Mortar	of Brick- work	Laid Dry	Laid in Mortar	of Brick- work	
$ \begin{array}{c} 1.0\\ 1.3\\ 1.6\\ 1.9\\ 2.0\\ 2.3\\ 2.6\\ 3.0\\ 3.6\\ 4.0\\ 4.6\\ \end{array} $	28 33 38 43 48 53 58 68 79 89 100	$\begin{array}{c} 23 \\ 27 \\ 31 \\ 35 \\ 41 \\ 44 \\ 48 \\ 57 \\ 65 \\ 73 \\ 82 \end{array}$	$\begin{array}{c} 1.6198\\ 1.8145\\ 2.2089\\ 2.5035\\ 2.7979\\ 3.0926\\ 3.3870\\ 3.9760\\ 4.5651\\ 5.1541\\ 5.7432\end{array}$	$\begin{array}{c c} 70\\ 80\\ 90\\ 102\\ 112\\ 122\\ 132\\ 154\\ 174\\ \cdot 194\\ \cdot 194\\ \end{array}$	$\begin{array}{c} 58\\ 66\\ 74\\ 82\\ 92\\ 100\\ 108\\ 126\\ 142\\ 159\\ \end{array}$	$\begin{array}{r} 4.1233\\ 4.7124\\ 5.3015\\ 5.8905\\ 6.4795\\ 7.0686\\ 7.6577\\ 8:8357\\ 10.0139\\ 11.1919\end{array}$	
$\begin{array}{c} 4.6\\ 5.0\\ 5.6\\ 6.0\\ 6.6\\ 7.0\\ 7.6\\ 8.0\\ 8.6\\ 9.0\\ 0.0\\ \end{array}$	100       110       120       130       140       150       160       170       180       191       212	82 90 98 107 115 123 131 140 148 156 174	$\begin{array}{c} 5.7432\\ 6.3322\\ 6.9213\\ 7.5103\\ 8.0994\\ 8.6884\\ 9.2775\\ 9.8665\\ 10.4556\\ 11.0446\\ 12.2227\\ \end{array}$	$\begin{array}{c} 214\\ 234\\ 254\\ 276\\ 296\\ 316\\ 336\\ 358\\ 378\\ 398\\ 438\\ \end{array}$	$\begin{array}{c} 139\\ 176\\ 192\\ 209\\ 226\\ 242\\ 260\\ 276\\ 292\\ 308\\ 326\\ 360\\ \end{array}$	$\begin{array}{c} 11.1919\\ 12.3701\\ 13.5481\\ 14.7263\\ 15.9043\\ 17.0825\\ 18.2605\\ 19.4387\\ 20.6167\\ 21.7949\\ 22.9729\\ 25.3291\\ \end{array}$	

#### THE THICKNESS OF WALLS FOR DWELLING HOUSES-BRICK

#### Maximum Height = 100 feet. Maximum Length.

45 feet.	80 feet.	Unlimited.
Two stories of $21\frac{1}{2}$ Three stories of $17\frac{1}{2}$ Remainder13	Two stories of 26 Two stories of 21 $\frac{1}{2}$ Two stories of 17 $\frac{1}{2}$ Remainder13	Inches One story of 30 Two stories of 26 Two stories of 211 Two stories of 171 Remainder13

#### Maximum Height = 90 feet. Maximum Length.

45 feet.	70 feet.	Unlimited.
Inches Two stories of $21\frac{1}{2}$ Two stories of $17\frac{1}{2}$ Remainder13	$\begin{array}{c} \text{Inches}\\ \text{Oue story of 26}\\ \text{Two stories of 21} \frac{1}{2}\\ \text{Two stories of 17} \frac{1}{2}\\ \text{Remainder}, \ldots, 13 \end{array}$	Inches One story of 30 Two stories of 26 One story of 211 Two stories of 171 Remainder13

#### Maximum Height == 8' feet. Maximum Length.

40 feet.	60 feet.	Unlimited.
One story of $21\frac{1}{2}$ Two stories of $17\frac{1}{2}$ Remainder13	Two stories of $21\frac{1}{2}$ Two stories of $17\frac{1}{2}$ Remainder13	Inches One story of 26 Two stories of 211 Two stories of $17\frac{1}{2}$ Remainder13

#### Maximum Height = 70 feet. Maximum Length.

40 feet.	55 feet	Unlimited.		
Two stories of 17½ Remainder13	One story of $21\frac{1}{2}$ Two stories of $17\frac{1}{2}$ Remainder13	Inches One story of 26 Two stories of 211 One story of 171 Remainder13		

#### Maximum Height = 60 feet. Maximum Length.

30 feet.		50 feet.	Unlimited
One story of 17½ Remainder13	Т	Inches tories of 17½ nder13	Inches One story of 211 Two stories of 171 Remainder13

#### Maximum Height = 50 feet. Maximum Length

	incligen.	
30_feet	45 feet.	Unlimited
Wall below the topmost story 13 Topmost story 81 Remainder 82	Inches One story of 17½ Rest of wall below topmost story 13 Topmost story 8½ Remainder84	Inches One story of 214 One story of 175 Remainder13

Maximum Height = 40 feet.

Maximum -Length.

35 feet.	Unlimited.
Wall below two topmost stories       Inches topmost         Two topmost stories of       13         Remainder       8½	Inches       One story of     17½       Rest of wall below topmost story     13       Topmost story     8½       Remainder     8½

#### Maximum Height = 30 feet. Maximum Length.

35 feet.	Unlimited
Inches         ries	Wall below topmost story

 $\begin{array}{ll} {\rm Maximum} \ {\rm Height} = 25 \ {\rm feet}. \\ {\rm Maximum} \ {\rm Length}. \end{array}$ 

30 feet.	Unlimited
From base to top of wall $8\frac{1}{2}$	Wall below topmost storm 12
	Topmost story

Maximum Height in feet	Maximum Length in feet	Thickness at Base in inches	Maximum Length in feet	Thickness at Base in inches	Maximum Length in feet	Thickness at Base in inches
90 80 70 60 50	$     \begin{array}{r}       55 \\       60 \\       45 \\       30 \\       35 \\       40 \\       20 \\     \end{array} $	$\begin{array}{c} 26 \\ 26 \\ 21\frac{1}{2} \\ 17\frac{1}{2} \\ 17\frac{1}{2} \\ 17\frac{1}{2} \\ 17\frac{1}{2} \\ 17\frac{1}{2} \\ \end{array}$	70 70 60 45 50 70	$\begin{array}{c} 30 \\ 30 \\ 26 \\ 21\frac{1}{2} \\ 21\frac{1}{2} \\ 21\frac{1}{2} \\ 21\frac{1}{2} \\ \end{array}$	.ength ilimited	$     \begin{array}{r}       34 \\       34 \\       30 \\       26 \\        26 \\        26 \\       26$
30 25	45	$\begin{array}{c c} 13 \\ 13 \\ - \end{array}$			L 19	$     \begin{array}{r}       20 \\       21 \\       17 \\       12 \\       13     \end{array}   $

## THE THICKNESS OF WALLS FOR WAREHOUSES-BRICK

The thickness of the walls at the top for warehouses, and for 16 feet below the top, shall = 13 inches; and the intermediate parts of the wall, between the base and such 16 feet below the top, to be solid throughout the space between straight lines drawn on each side of the wall from the base to the part 16 feet below the top, as above determined; but in walls not exceeding 30 feet in height, those of the topmost story may be 8½ inches thick.

The thickness to be increased to one-sixteenth part of the height of the story for dwelling houses, and to one-fourteenth part for warehouses, in case the thickness determined by the foregoing tables be less than that proportion.

The width of the footings at the base to be *double* the thickness of the wall, to diminish in regular offsets, and to be equal in height to one-half of the width at base.

### ROOFS GENERALLY

#### SHINGLING

To find the number of shingles required to cover 100 square feet deduct 3 inches from the length, divide the remainder by 3, the result will be the exposed length of a shingle; multiplying this by the average width of a shingle, the product will be the exposed area. Dividing 14,400, the number of square inches in a square, by the exposed area of a shingle will give the number required to cover 100 square feet of roof.

In estimating the number of shingles required, an allowance should always be made for waste.

Estimates on cost of shir ; le roofs are usually given per 1,000 shingles.

Length of Stungles	Exposure to Weather Juches	No. of sq. ft. ered by 100	of Roof Cov- 90 Shingles.	No. of Shingles Required for 100 sq. feet of Roof.		
15 in. 18 21 24	4 5 6 7	4 in. Wide 111 139 167 194	6 in. Wide 167 208 250 201	4 in. Wide 900 720 600	6 in. Wide 600 480 400	
27	8	222	333	$\frac{514}{450}$	343 300	

ABLE FOR	ESTIMATING	SHINCLES
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#### SLATING

A square of slate or slating is 100 superficial feet.

In measuring, the width of eaves is allowed at the widest part. Hips, valleys and cuttings are to be measured lineal, and 6 inches extra is allowed.

The thickness of slates required is from 3-16 to 5-16 of  $z^{-1}$  inch, and their weight varies when lapped from 4.5 to  $6\frac{3}{4}$  pounds per square foot.

The "laps" of slates vary from 2 to 1 inches, the standard assumed to be 3 inches.

### TO COMPUTE THE NUMBER OF SLATES OF A GIVEN SIZE REQUIRED PER SQUARE

Subtract 3 inches from the length of the slate, multiply the remainder by the width and divide by 2. Divide 14,400 by the number so found and the result will be the number of slates required.

### TABLE SHOWING NUMBER OF SLATES AND FOUNDS OF NAILS REQUIRED TO COVER 100 SQUARE FEET OF ROOF

Size of Slate	Length of Exposure	No. Required	Anite D
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c} 121 \text{ in.} \\ 105 \\ 95 \\ 85 \\ 75 \\ 65 \\ 55 \\ 45 \\ 45 \\ 45 \\ 45 \\ 45 \\ 45 \\ 4$	83 114 138 165 214 277 377 533	Nails Required .6 Hs. .833 1. 1.33 1.5 2. 2.66 3.8

## APPROXIMATE WEIGHT OF MATERIALS FOR ROOFS

Material	Averag	e weight lb.
Copper, 16 oz, standing seam	 T	21
Felt and asphalt, without sheathing.	•••••	11
Hemlock sheathing 1 in the	•••••	17
Lead, about i inch thick.	• • • • •	2
Mackite, 1 inch thiak middle and the second second	••••	6 to 8
Neponset roofing felt, 2 layers	• • •	10
Spruce sheathing, 1 inch thick.	••••	21
Slate, # inch thick, 3-inch double lap	••••	$6\frac{2}{4}$
Shingles, 6"×18", 1 to weather	••••	$\frac{41}{2}$
Slag roof, 4-nly.	••••	4 to 10
· · · · · · · · · · · · · · · · · · ·		4

Terne plate, IC, without sheathing	Average per	weight I
Frine Plate, iX, without sheathing Files (plain), $10\frac{1}{2}'' \times 6\frac{1}{4}'' \times \frac{1}{2}'' - 5\frac{1}{4}''$ to weather. Files (Spanish), $14\frac{1}{4}'' \times 10\frac{1}{4}'' - 7\frac{1}{4}''$ to weather. White-pine sheathing, 1 inch thick. Fellow-pine sheathing, 1 inch thick.	• • • • • •	18 18 85 25

#### SNOW AND WIND LOADS

Data in regard to snow and wind loads are necessary in connection with the design of roof trusses.

**Snow Load.**—When the slope of a roof is over 12 inches rise per foot of horizontal run, a snow and accidental load of 8 pounds per square foot is ample. When the slope is under 12 inches rise per foot of run, a snow and accidental load of 12 pounds per square foot should be used. The snow load acts vertically, and therefore should be added to the dead load in designing roof trusses. The snow load may be neglected when a high wind pressure has been considered, as a great wind storm would very likely remove all the snow from the roof.

Wind Load.—The wind is considered is blowing in a horizontal direction, but the resulting pressure upon the roof is always taken *normal* (at right angles) to the slope. The wind pressure against a vertical plane depends on the velocity of the wind, and, as ascertained by the United States Signal Service at Mount Washington, N. H., is as follows:

Velocity Mi. per Hr V	Pressure	
our ber mit.)	(LD. per Sq. Ft.)	
$\frac{10}{20}$	0.4	Fresh breeze.
20	· · • • • • • • • • I.6	Stiff hangen
30		of the preeze.
40	0.0	····. Strong wind.
=0	····· 0.4	High wind
		Danne Willia.
60	14 4	
80	*********************	· · · · · · · · · · Violent storm
00		Humison
100	40.0	in in in the second sec
		····· Violent hurricane.

The wind pressure upon a cylindrical surface is onehalf that upon a flat surface of the same height and width.

Since the wind is considered as traveling in a horizontal direction, it is evident that the more nearly vertical the slope of the roof, the greater will be the pressure, and the more nearly horizontal the slope, the less will be the pressure. The following table gives the pressure exerted upon roofs of different slopes, by a wind pressure of 40 pounds per square foot on a vertical plane, which is equivalent in intensity to a violent hurricane.

### WIND PRESSURES ON ROOFS

Rise						
In. per Foot of Run	Angle with Horizontal	Pitch Proportion of Rise to Suan	Wind Pressure Normal to Slope			
4	18 957	The to span				
6	26 33 /	Ê,	16.8			
8	33 41'		2: .7			
12	45'' - 0'	1	29.1			
10	53 7'	2	38.7			
24	00 20' 62 07/	3	39.3			
	03 27	1	40.0			

#### (Pounds per Square Foot)

In addition to wind and snow loads upon roofs, the weight of the principals or roof trusses, including the other features of the construction, should be figured in the estimate. For light roofs having a span of not over 50 feet, and not required to support any ceiling, the weight of the steel construction may be taken at 5 pounds per square foot; for greater spans, 1 pound per square foot should be added for each 10 feet increase in the span.

### COMPARATIV: COST OF ROOFS

It often happens that an estimater is asked as to the difference in the cost of roofs, and on his answer the construction of the stock may depend; therefore it is necessary that he should be able to give his answer with some degree of intelligence and exactness; and the following, to some extent, will enable him to do this.

For instance, take a "span roof," by which we mean one having two sides inclining to a ridge, and let the length of the rafter be 16 feet, and that of the roof from edge to edge be 14 feet.

Then it contains on each side a trifle over 7 squares of 100 superficial feet each.

If the roof is to be slated or tinned it will require the sheathing to be laid close, and with what is called "match mill-planed timber," which is provided with tongue and groove, and need not, as the name implies, be mill-planed, although it usually is.

We next come to consider the cost of sheathing, nails, and labor required in putting it on, which, approximately, is as follows:

### PREPARING FOR SLATE OR TIN ROOF

squares of roofing require 700 feet of sheathing
Labor required in putting same on, at 50 cents
Nails for fastening sheathing boards 3.50
Total cost

#### SLATE ROOF

We find the cost of the slate roof to wit:
For preparing for roof\$15.00 For 7 squares of slating, including labor, material.
etc., ar \$12 per square 84.00
Total eost

22I

Thus it will be seen that the total cost of 7 squares of slating aggregates a cost of \$99.00, or \$14.15 per square.

#### TIN ROOF

Since the work of preparing for the tin roof is the same as for slate, we add to it the cost for tin and painting as follows:

For preparing for roof\$15.00 For 7 squares of tin work at 75 cents per square, including material and labor	
per yard 11.70	
Total cost	
t those C	

At these figures we find that 7 squares of tin roofing will cost \$79.20, or a trifle over \$11.31 per square.

#### SHINGLE ROOF

In estimating the amount of sheathing required for a shingle roof, we bear in mind the fact that it will not be necessary to lay boards close together; but strips 3 inches wide can be used, and if so, it will require about one-half of the amount it does when laid close, as for the slate or tin roof. Hence the following is the approximate estimate of cost.

300 feet of sheathing at \$12.50 per M\$	3.75
Nails for sheathing ste	. 05
7,000 shingles, nails and labor at \$7 per square 49	.50 1.00
Total cost	20

Thus the cost of 7 squares of shingling will aggregate \$54.30, or a trifle over \$7.75 per square.

#### COMPOSITION ROOF

Now suppose that the slope of the roof permitted the surface to be covered with gravel or composition roofing, then the sheathing need not be laid as carefully as for tin or slate, and an inferior quality of lumber can be used; the only requirements being that the surface must be level and smooth.

In such a case the estimate of cost would be as follows:

700 feet of sheathing at \$12.50 per M.8.75Putting on same at 35 cents per square.2.45Nails for sheathing, etc.1.007 squares roofing material, etc., \$4 per square.28.00
Total eost

Making the cost of 7 squares amount to \$40.20, or a trifle over \$5.74 per square.

Slate on iron as 1:
Motal tile til
Metal the, the $\frac{1}{10}$ Metal the \frac{1}{10} metal the $\frac{1}{10}$ metal the \frac{1}{10} met
metal the, steel, lead-coated
Rubber roofing
Felt and gravel
Ornamental tile
Tile shingles. 40.00 to 60.00 per M.
Charcoal tin plates I.C. 14, 200 to 35.00 per M.
Charcoal tin plates, I.C., $14 \times 20$ ins. 6.00 to 6.50 per box of 112.
Charcoal time tasks, 1.0., $20 \times 28$ ins. 12.00 to 13.00 per box of 119
Chance 1 tim plates, 1. $\Lambda_{.,14} \times 20$ ins. 7.50 to 8.50 per box of 112.
Cohered tim plates, I.X., $20 \times 28$ ins15.00 to 17.00 per box of 112.
Coke plates, tin, I.C., $14 \times 20$ ins 5.50 per hox of 112.
Coke plates, tin, I.C., $20 \times 28$ ins 11 50 to 12 00 per box of 112.
Coke plates, tin, I.X., $14 \times 20$ ins 7 50
Chareoal plate, terne, LC 14×20 ing 5 50 per box of 112.
Charcoal plate, terme I.C. 20 y 20 in 5.50 per box of 112.
"harcoal plate (erne, I.V., 20 × 28 lns 10.75 to 11.00 per box of 112.
$\frac{112}{112}$ barcoal plate torus <b>I</b> $\frac{112}{112}$ barcoal plate torus <b>I</b> $\frac{112}{112}$
place, terme, 1.A., $20 \times 28$ ins. 12.80 per box of 112.

### FLAT SEAM TIN ROOFING

Table showing quantity of  $14'' \ge 20''$  tin required to cover a given number of square feet with flat seam tin roofing. A sheet of  $14'' \ge 20''$  with  $\frac{1}{2}''$  edges measures, when edged or folded,  $13'' \ge 19''$  or 247 square inches. In the following, all fractional parts of a sheet are counted a full sheet.

		1	1			•				
No. of sq. feet	Sheets required	No. of sq. feet	Sheets required	No. of sq. feet	Sheets required	No. of sq. feet	Sheets equired	No. of q. feet	sheets quired	-
$\begin{array}{c} 100\\ 110\\ 120\\ 130\\ 140\\ 150\\ 160\\ 170\\ 180\\ 190\\ 200\\ 210\\ 220\\ 220\\ 230\\ 240\\ \end{array}$	$\begin{array}{r} 1\\ 59\\ 65\\ 70\\ 76\\ 82\\ 88\\ 94\\ 100\\ 105\\ 111\\ 117\\ 123\\ 129\\ 135\\ 140\\ \end{array}$	$\begin{array}{r} 3 \\ \hline 280 \\ 290 \\ 300 \\ 310 \\ 320 \\ 330 \\ 330 \\ 340 \\ 350 \\ 360 \\ 350 \\ 350 \\ 350 \\ 350 \\ 350 \\ 360 \\ 410 \\ 410 \\ 420 \\ \end{array}$	$\begin{array}{c} 122\\ 164\\ 170\\ 175\\ 181\\ 187\\ 193\\ 199\\ 205\\ 210\\ 216\\ 222\\ 228\\ 234\\ 234\\ 246\\ 245\\ 205\\ 205\\ 205\\ 216\\ 205\\ 205\\ 205\\ 205\\ 205\\ 205\\ 205\\ 205$	$\begin{array}{c} 460\\ 470\\ 480\\ 490\\ 500\\ 510\\ 520\\ 530\\ 540\\ 550\\ 550\\ 560\\ 570\\ 580\\ 590\\ \end{array}$	$\begin{array}{c} 2 & \frac{3}{2} \\ 269 \\ 275 \\ 280 \\ 292 \\ 298 \\ 304 \\ 309 \\ 315 \\ 321 \\ 327 \\ 333 \\ 339 \\ 344 \\ \end{array}$	$\begin{array}{c} 2 \ 9 \\ \hline 640 \\ 650 \\ 660 \\ 670 \\ 680 \\ 690 \\ 700 \\ 710 \\ 720 \\ 730 \\ 740 \\ 750 \\ 760 \\ 770 \\ \end{array}$	$\overline{x} \underline{\xi}$ 374 379 385 391 403 409 414 420 426 432 438 444 449	R 5 820 830 840 850 850 870 880 890 900 910 920 930 940 950	$\begin{array}{c} \overline{z} \ \overline{by} \\ 479 \\ 484 \\ 490 \\ 502 \\ 508 \\ 514 \\ 519 \\ 525 \\ 531 \\ 537 \\ 543 \\ 543 \\ 554 \end{array}$	
250 260 270	$\frac{146}{152} \\ 158$	430 440 450	245 251 257 263	600 610 620 630	$350 \\ 356 \\ 362 \\ 368$	780 790 800 810	455 461 467 473	960 970 980 990	560 566 572 578	

1000 square feet 583 sheets.

A box of 112 sheets  $14'' \times 20''$  will cover approximately 192 square feet.

### STANDING SEAM TIN ROOFING

Table showing quantity of  $20'' \times 28''$  tin required to cover a given number of square feet with standing seam roofing. The standing seams and the locks on a steep roof require  $2\frac{3}{4}''$  off the width and  $\frac{3}{4}''$  off the length of the sheet; fractional parts are counted as a full sheet. A sheet will cover 475 square inches.

				1		1	-		
60	Les	60	ts.	57	E	22	T x	1	
· • •	E	1 a 🐃	E G	1.24	8.1	1 . T	1.5	ిక	Let
ZZ	S D	ž z	175	Žż	Ë F	19-	lu	21	30
				a.	1 2 3	1 2 2	Les 1	2.5	SS
100	31	280	85	460	1.10	610	101	000	
110	34	290	88	170	119	040	194	820	249
120	27	200	01	100	143	050	197	830	252
120	10	910	91	480	147	660	200	840	255
130	40	310	94	490	149	670	203	850	959
140	43	320	97	500	152	680	206	03.9	961
150	46	330	100	510	158	600	200	000	201
160	49	340	103	520	161	700	209	010	264
170	52	350	106	530	161	710	412	880	267
180	55	360	109	510	167	790	210	890	270
190	58	370	112	550	170	720	218	900	273
200	61	380	115	500	170	130	221	910	276
210	6.1	200	110	500	173	740	224	920	279
- 220'	67	100	118	5/0	176	750	228	930	282
220	201	400	122	580	182 [	760	231	940	285
230	- 70	410	125	590	184	770	234	950	288
240	- 73	420	128	600	185	780	937	060	
-250	76	430	131	610	185	700	210	070	491
260	79	440	134	620	199	000	240	970	294
270	82	450	137	620	101	800	243	980	297
			1.01	000	191	810	246 1	990	300

1000 square feet 303 sheets.

A full box 112 sheets  $20'' \times 28''$  will cover approximately 370 square feet.

It must be understood that the figures given in the foregoing are not considered as being correct or suited to all localities; they may be taken as approximately exact, but in all cases the percentage of difference in cost may be taken as fairly correct, and it is this result for which the tables were prepared.

# SPECIFIC GRAVITY AND WEIGHTS

## BUILDING MATERIALS

Name of Material	Weight per	Specific
Brick, pressed	Cu. ft. fb.	Gravity
Brick, common	150	2.40
Cement, Portland	125	2.00
Cement, Rosedale	80 to 100	1.44
Common brickwork	56	.89
Common briek work the	r 130	2.10
Concrete cement	120	1.90
Earth, dry, shakon	140	2.25
Earth, rammed	82 to 92	1.36
Glass, window	90 to 100	1.52
Granite.	157	2.52
Granite or limestone - 111	170	2.72
Granite or linestone, rubble work.	138	2.21
Limestones and markle	165	2.65
Lime, Quiek.	168	2.70
Mortar, hardened	53	.85
Plaster of paris	103	1.65
Pressed brickwork	141.6	2.27
Sand.	140	2.25
Sandstone 9	0 to 106	2.65
Shales.	151	2.41
Slate	162	2.60
Frap Rock	175	2.80
	187	3.00
		~

## WOODS (DRY)

Name of Material	Weight	Weight non	a
Ash.	Per ft. Bm.	Cu. ft. 10.	Specific
Ash, American white	3.9	47	.752
Boxwood	3.2	38	.610
Cherry.	5.	60	.960
Chestnut	3.5	42	.672
Cork	3.4	41	.660
	1.3	15	250

Elm.	2.9	35	560
Howlesh	6.3	76.1	1.220
Hickory	2.1	25	.400
Lignum Vita	4.4	53	.850
Mahogany, Spanish	6.9	83	1.330
Mahogany, Honduras	4.4	53	.850
Maple	2.9 4 1	35	.560
Oak, live	4.9	49	. 790
Oak, white.	4.0	48	. 950
Oak, red.	3.2	40	- 7 70
Pine, white	2.1	25	400
Pine southers	2.8	34.3	.550
Sycamore	3.7	45	.720
Spruce	3.1	37	.590
Walnut.	2.1	25	.400
	3.2	- 38	.610

The estimated weight of logs is one-half more than the estimated weight of the green lumber of the same kind of wood.

#### THE METRIC SYSTEM

The metric system is based on the meter, which, according to the United States Coast and Geodetic Survey Report of 1884, is equal to 39.370432 inches. The value commonly used is 39.37 inches, and is authorized by the United States government. The meter is defined as one ten-million the distance from the pole to the equator, measured on a meridian passing near Paris.

There are three principal units: the meter, the liter (pronounced lee-ter), and the gram, the units of length, capacity and weight, respectively. Multiples of these units are obtained by prefixing to the names of the principal units the Greek words Deca (10), hecto (100), and kilo (1,000); the submultiples, or divisions, are

## HCDGSON'S ESTIMATOR .

obtained by prefixing the Latin words Deci (1/10), centi (1/100), and milli (1/1000). These prefixes form the key to the entire system. In the following tables the abbreviations of the principal units of these submultiples begin with a small letter, while those of the multiples begin with a capital letter; they should always be written as here printed.

### MEASURES OF LENGTH

Millimeter (mm.) Centimeter (cm.) Decimeter (dm.) Meter (m.) Decameter (Dm.)		Meters .001 .010 .100 1.000 10.000	1 1 1 1	U. S. In. .039370 .393704 3.937043 39.370432	1 1 1 1	Feet .003281 .032809 .328087 3.380869
Hectometer (Hm.) Kilometer (Km.) Myriameter (Mm.)	1 11 11	$100.000 \\ 1,000.000 \\ 10,000.000$	1	.621 mi. 6.214 mi.		$\begin{array}{r} 32.808690\\ 328.086900\\ 3,280.869000\\ 32,808.690000\end{array}$

The centimeter, meter and kilometer are the units in practical use, and may be said to occupy the same position in the metric system as do inches, yards and miles in the United States and English system of measurement.

#### MEASURES OF AREA

Name Sq. millimeter (mm. <sup>2</sup> ) Sq. centimeter (cm. <sup>2</sup> ) Sq. decimeter (dm. <sup>2</sup> ) Sq. meter or centare	11 11 11	Sq. Met. .0000010= .0001000= .0100000=	Sq. In. .00153 .15500 15.5003	50 == )3 == =	Sq. Ft .0010764 .1076410	11 10	Ac <b>r</b> es
(m.² or ca.) Sq. decameter or are	===	1.0000000 ==	1,550.03	=	10.7641000	0=	.000247
(Dm. <sup>2</sup> or A.) Hectare Sq. kilometer Sq. myriameter	== == =	$\begin{array}{l} 100.0000000 == 15\\ 0.000.0000000 ==\\ .3561099 \ \mathrm{sq. \ r}\\ 38.6109000 \ \mathrm{sq. \ r} \end{array}$	5,003 mi. mi.	= = =10, =	1,076,4101 107,641.01 764,101	= = = 24	.024710 2.47110 247.110 4.711.0
Name Cu. centimeter (cm Cu. decimeter (dm. Centistere Decistere	, <sup>3</sup> )	MEASURES = Cu. Met. = .000001 = .091000 = 010009	OF VC = .0 = .0 = .0 = .0	LUN 1n. 6102 254	ME 5 Cu, F	't.	Cu. Yd.

ere re =cu. m. (m. <sup>3</sup> )]	I I II II	0.091000 = 0.1000 = 0.1000 = 0.1000 = 0.1000 = 0.00000 = 0.0000000000	61.0254 6.0.2540	11 11	.35316 3.53156		
16	=	10.5		4	$35.3156 \\ 353.156$	11 11	1.308

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Stere [ Decaste

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## CURRENT MEASURES

#### LINEAL MEASURE

12	inches (in.).	= 1 foot	
3	feet	= 1 vard	••••••ft.
-5.5	yards	= 1  rod	•••••yd,
40	rods	= 1 inclose	•••••••••••••.rd.
8	furlongs	= 1 mile	·····fur.
	$\begin{array}{ccc} \text{In.} & \text{Ft.} \\ 36 = & 3 \end{array}$	Yd.	Rd. Fur. Mi.
	198 = 16.5	5 = 5.5 =	1
	7,920 = 660	= 220 =	40 = 1
	63,300 = 5,280	=1,760 =	320 = 8 = 1
ther	unite of		

## Other units of measure are:

. . .

5	feet	equal 1 pace.

- 21 feet equal 1 military pace.
- 6 feet equal 1 fathom.
- 9 inches equal 1 span.
- 18 inches equal 1 cubit.
- 4 inches equal 1 hand (to measure horses).
- 21.8 inches equal 1 Bible cubit.

### SURVEYOR'S MEASURE

· · · ·	.92 inches = 1 link
25	linksl.
4	rods:) - 1 rod
100	links = 1 chain ch
66	feet: )
80	chains
	1  min = 00  J
	$1 \text{ mi.} = 30 \text{ ch.} = 320 \text{ rd.} = 8,000 \text{ li}_{\cdot} = 63,360 \text{ in}_{\cdot}$

#### SQUARE MEASURE

144	square inche	s(sq, in) =	1 course fact	
9	square feet	· · · · · · · · ·	1 square 100t	• •sq. it.
301	Pomono mont	••••••	1 square yard.	sq. vd.
100	square yards	$\cdots \cdots \cdots \cdots =$	1 square rod.	
100	square rods.	==	1 acre	A A
640	acres.		1	· . A.
Sami A		••••••	square mile.	sq. mi.
$1 - c_4$	Sq. rd.	Sq. yd.	Sq. ft.	Sa in
1 = 040	v = 102,400	= 3.097.600 =	= 27 378 100 -	1.011.100.000
				±,014,489,600

0), rm es Ьof ld

2

1

## SURVEYOR'S SQUARE MEASURE

$\begin{array}{c} 625 \text{ square links (sq. li.).} \\ 16 \text{ square rods} \\ 10 \text{ square chains} \\ 640 \text{ acres} \\ 36 \text{ sq miles (6 mi. square)} \\ 1 \text{ sq. mi.} = 640 \text{ A.} = 6,400 \text{ sq. ch} \\ \text{sq. li} \end{array}$	= 1 square rod sq. rd. = 1 square chain sq. ch. = 1 acre
sq. li.	= 102,400 sq. rd. $= 64,000,000$

The acre contains 4,840 square yards, or 43,560 square feet, and in form of a square is 208.71 feet on a side.

THE WEAR AND TEAR OF BUILDING MATERIALS

		elling	Brick (shin)	dwelling de roof)	Fran	le store	Brie. (shing	k store de roof)
Material in Building	Aver gelife	Per of depreciation per annum	Average life Years	Per cent of depreciation Per annum	Average life Years	Per cent of epreciation ber annum	verage life Years	er rent of Preciation er annum
Brick. Plastering, outside Painting, inside Sningles Comice Weather bo'ding Sheathing. Flooring Doors, complete Vindows, comp. Stairs and newel Base Inside blinds Building h'dware Piazzas & porches burside blinds Stills and first- floor joints	$   \begin{array}{r}     20 \\     5 \\     7 \\     16 \\     40 \\     30 \\     20 \\     30 \\     30 \\     40 \\     30 \\     40 \\     30 \\     20 \\     20 \\     16 \\     25 \\   \end{array} $	$   \begin{array}{r}             5 \\             20 \\             14 \\             6 \\             21 \\             33 \\             2 \\           $	$\begin{array}{c} 75\\ 30\\ 7\\ 7\\ 16\\ 40\\ -\\ 50\\ 20\\ 30\\ 30\\ 30\\ 30\\ 20\\ 20\\ 16\\ 16\\ 16\\ 16\\ 16\\ 16\\ 16\\ 16\\ 16\\ 16$	$ \begin{array}{c} 1 \\ 1 \\ 3 \\ 3 \\ 1 \\ 4 \\ 1 \\ 4 \\ 2 \\ 2 \\ 5 \\ 3 \\ 3 \\ 3 \\ 3 \\ 3 \\ 5 \\ 5 \\ 6 \\ \end{array} $	$\begin{array}{c} - \\ \hline 16 \\ 5 \\ 5 \\ 16 \\ 30 \\ 40 \\ 13 \\ 25 \\ 20 \\ 30 \\ 13 \\ 20 \\ 16 \\ \end{array}$	$\begin{array}{c} - & - & - & - & - & - & - & - & - & - $	$\begin{array}{c} - \\ \hline & \\ 66 \\ 30 \\ 6 \\ 6 \\ 16 \\ 40 \\ \hline \\ 50 \\ 13 \\ 30 \\ 30 \\ 30 \\ 30 \\ 30 \\ 30 \\ 3$	$\frac{d}{d} = \frac{1}{1} \frac{1}{3} \frac{1}{4} \frac{1}{2} \frac{1}{4} \frac{1}{2} \frac{1}{2} \frac{1}{8} \frac{1}{3} \frac{1}{3} \frac{1}{5} \frac{1}{3} \frac{1}{3} \frac{1}{3} \frac{1}{8} \frac{1}{5} \frac{1}{5} \frac{1}{3} \frac{1}{3} \frac{1}{8} \frac{1}{5} \frac{1}{5} \frac{1}{5} \frac{1}{3} \frac{1}{3} \frac{1}{8} \frac{1}{5} $
and asion fumbr	50	2	75	11	25 40	$\frac{4}{2\frac{1}{3}}$	30 66	31

These figures represent the averages deduced from the replies made by eighty-three competent builders unconnected with fire-insurance companies, in twentyseven cities and towns of eleven Western States.

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0.0.0

### HOW TO FIGUE PLASTERING

Multiply the distance around the four sides of the room in feet by the height of the room in feet. Multiply the product by the price per square yard and divide this product by 9, because there are 9 square feet in a square yard. For the ceiling, multiply the length of the room by the width of the room in feet and then by the price per square yard, and divide by 9 as before. Add these two results and you have the entire cest of plastering the room.

To every barret of lime estimate about § of a cubic yard of good sand for plastering.

One-third of a barrel of stucco will hard finish 100 square yards of plastering.

Six bushels of lime, 40 cubic feet of sand and 1½ bushels of hair will plaster 100 square yards with two coats of mortar.

In plastering, no deductions are made for openings, because it is considered that the extra work in finishing around them balances the material saved.

## WEIGHTS OF PACIFIC COAST LUMBER

Oregon Fir 1 inch rough	Lbs, p	er M.
Washington Rod Calar 1	2	,200
Washington Red Cedar, 1 meh, rough	2	,300
Colifornia a Dia Dia Dia Dia Dia Dia Dia Dia Dia	2	,000,
California Sugar Pine, 1 inch, rough	2	200
California Redwood, 1 to 2 inch, rough.	. 2	500
California Redwood, 1 to 2 inch, S1S		200
California Redwood, 1 to 2 inch S2S	· · · · <u>-</u> , c	-00
Cedar Shingles, * A *	•••• 4,	000
		200 -

## STANDARD WEIGHTS OF CYPRESS LUMBER

Lumber rough 2 inches and 1	per M.
Lumber, rough, 2 menes and under.	3,000
Linch Florida 1 G w	3,500
g-men rooring and Ceiling	2,300

)00 60 a

2-inch Ceiling		Lb	s. per M
A-jach Coiling	• • • • • • •		1,600
J-inch Coiling	• • • • • • •		1,300
Lingh David P P			1,000
Shingles all 1			1,000
Jinal Distant States			300
g-men Plaster Lath			500
g-men rence Lath			900
1 x 1 x 4 D, & II. Pickets			1 600
& x 21 x 4 D. & H. Pickets		•••	1 800
2-meh O. G. Battens		•••	500
21-ineh O. G. Battens	•••••••	•••	(00)
B-inch O. G. Battens	• • • • • • •	• •	000
			1111

### ESTIMATED WEIGHTS OF WHITE PINE

Timbers rough		Green	M. Feet Dry
Lumber rough	••	3,250	2,500
Lumber dressed	•••	3,000	2,400
Lumber D & M	•••	2,500	2,000
Battens O G	•••	.2,400	1,800
Siding and & Colling	•••	1,900	1,500
Shingles.	••	1,250	800
Lath	•	450	250
		950	500

### ESTIMATED WEIGHTS OF NORWAY PINE

Timbers rough	Lbs. per M Feet Green Dry
Lumbor must	. 3,500 2,750
Lumber, rough	3,250 2,650
Lumber, dressed	2,900 2,300
Lumber, D. & M $\ldots$	. 2,600 2,000

These weights are taken from reports issued by the Argicultural Department of the United States.

## ESTIMATING FRAME OR BALLOON BUILDINGS

In estimating the cost  $\infty$  labor necessary to convert rough lumber into available building material, the estimator should divide the labor as follows:

First, ascertain the cost of framing sills, joist, studs, rafters, and like dimension stuff on the ground ready to go into the building.

Second, estimate the cost of placing it on the building, or into the work. Siding, roof boards, sheathing, furring and flooring requires no primary labor to prepare it for the building; and, therefore, this class of material calls for the price of labor only to put it on the building.

The simplest method to estimate the labor of framing dimension or piece stuff, as scantling of all kinds, is by the thousand thet. A general rule adopted by us after a long experience and considerable and tigation, is to add the entire bill of dimension stuff cogether, and price it for medium work at \$1.00 per thousand for the labor of framing on the ground, and \$5.00 per thousand for labor of working it into the building. We base our rule on the following demonstrations:

Two good carpenters will lay out and frame 50 pieces of  $2 \times 10$  joist, 16 feet long, in a day of 9 hours, or about 1,350 feet; or they will frame 100 pieces of  $2 \times 6$ studding, 12 feet long, in a day, or 1,200 feet; or they will frame 70 pieces of  $2 \times 6$ , 16 feet long, for rafters, in a day, or 1,120 feet; or they will frame 14 pieces of  $8 \times 8$  sills, 16 feet long, or 1,190 feet. Calling carpenters' wages at \$2.00 per day, we find that the framing of

Joist, 1,350 feet, cost.	00
Studding, 1.200 feet cost	.00
Rafters 1 120 feet west	.00
Sille 1 100 c	.00
5ms, 1,190 feet, cost	.00

Averaging the above, we find the price to be about \$1.00 per 1,000 feet.

For siding, roof boards, sheathing and flooring, the price may be fixed as on the following basis:

Two good carpenters will put on 800 feet of lap siding in a day, or 1,600 feet of roof boards per day; staging not included. Calling wages at \$3.00 per day, we find that to put on

Siding, costs \$7.20 per 1,000 feet. Roof boards, cost \$3.80 per 1,000 feet. Sheathing, costs \$2.70 per 1,000 feet.

One good man will lay 900 feet of  $1 \times 6$  matched flooring in a day, or 700 feet of 1 × 1 matched flooring in a day. At the same rate of wages the  $1 \times 6$  floor will cost \$3.25 per 1,000 feet to lay, and the  $1 \times 4$  floor will cost \$3.86 per 1,000 feet to lay.

A good man will carry up and lay on a roof from 1,600 to 2,400 shingles per day, which estimated at the

same rate of wages and averaged, is \$1.50 per 1,000. Two men will put on 2,000 feet of felt paper per day, which being reduced from the same rate of wages, makes it cost 30 cents per square of 100 feet.

Two men will lay 500 to 600 feet of outside beaded

ceiling work per day, or say \$11.25 per 1,000 feet. A man will put down 200 feet of plain base per day,

or 100 feet of moulded base.

A man will fit and nail 400 pieces of bridging per day, or 3 cent each.

Returning again to dimension stuff, as joist, studs, rafters, sills, etc., we find that two good men will place 50 pieces of  $2 \times 10$  joist, 16 feet long, in a day, or 150 pieces of  $2 \times 6$  studs, 12 feet long, in a day, or 100 pieces  $2 \times 6$  rafters, 16 feet long, in a day, or z) pieces of  $8 \times 8$  sills in a day.

For the labor necessary to place material on a

building, some builders estimate labor by the square, as follows: Wages \$3.00 per day.

Drop siding, 60 cents a square. Lap siding, 72 cents a square. Sheathing, 25 cents a square. Surface boards, 30 cents a square. Róof boards, plain, 30 cents a square. Hip roofs, 60 cents a square. Steep roofs, 65 cents a square. Steep roofs, 65 cents a square. Shingles, \$1.10 to \$1.25 a square. Floor pine, 1 x 6, 35 to 60 cents a square. Floor pine, 1 x 4, 35 to 60 cents a square. Floor pine, 1 x 3, 75 cents to \$1.25 a square. Outside wall ceiling, \$1.00 a square. Soffit ceiling, \$1.00 a square. Wainscoting, from \$2.00 to \$3.00 a square. Cleaning off pine floor, from 75 to 95 cents a square.

Tin work, valleys 14 inches wide, a man will lay from 1 to  $1\frac{3}{4}$  square feet of valleys per day.

In closing this series of tables upon one of the most vital subjects connected with the building profession, I desire to call attention to the fact that the manner of taking out quantities in the United States is somewhat different from that of Europe, and especially that of England, where the rules and methods connected with this particular branch of building are settled and well defined. In the embryonic state of our building practice, we have no universal or general methods of drawing off quantities, excepting what has come out of necessty.

The time will doubtless come when we shall have a universal method that shall not only be thoroughly established by practice, but indorsed by the various building trades and architectural associations throughout the entire country, so that a mechanic, having

become conversant with the rules and methods of New York, will not be called upon to study and make himself familiar with the rules and methods practiced in St. Louis or Chicago.

Large cities, by virtue of the facility for organization in the several branches of the building trades, are enabled to establish rules of measurement that govern their individual membership, but cannot control the conduct of other trades; hence, upon examination, it will be found that the rules of measurement for masonry in New York City vary from the rules in use in Cincinnati, Chicago and other large cities.

I am aware that the primary rules of mensuration, that is, the method of measuring any given surface or body, is governed by certain algebraic and mathematical calculations, which may be used by any one when he has mastered the proper method of procedure, and it is to illustrate and make plain this method that this book is written not only from a practical standpoint but from an American builder's view of the methods best adapted to the business interests of the builder.

Another somewhat different method than the foregoing is given herewith; it is taken from a trade journal of reliability, and possesses considerable merit.<sup>\*</sup> The system is all right, but the prices given are not to be followed, as they are much too low, not being within 25 to 35 per cent as high as current prices in the larger cities. This is especially arranged for balloon frame.

The first is an analysis of cost of four squares outside walls. For convenience, suppose a space  $20 \times 20$  feet as a basis, resulting in 400 square feet, or 4 squares. The studding employed is  $2 \times 4$  inch, sized on one

side and one edge. The studding is placed 16 inches from centers and covered with dressed and matched stuff. Building paper is next laid on, and then first or second clear siding is used. Plates are included in the cost and are put on double thickness.

#### ANALYSIS OF OUTSIDE WALLS

19 pieces, $2 \ge 4$ inch, 20 feet long = 247 foot	+
\$14.50 per M	\$2 50
466 feet dressed and matched stuff, at \$17.50	8 16
475 feet siding, at \$21	9.97
11 pounds nails.	. 40
Framing and aper, at 22 cents per pound.	.75
at \$8 per M	,
Laving 4 south row of flooring at 50	1.98
Laving 4 sources of siding at \$1 101	-2.00
Laying 4 squares at 121 cents per square	4.50
v is a quare	.50
Total.	291 04

Dividing this sum by 4 gives the price of a single square, \$7.96.

The analysis of cost of 4 squares of roofing, the rafters being 2 = 4 inch scantling, set 2 feet between centers, covered with dressed and matched stuff, and the best quality of cedar shingles, laid  $4\frac{1}{2}$  inches to the weather, is as follows:

#### ANALYSIS OF ROOF WORK

<ul> <li>12 scantlings, 2 x 4 inch, 20 feet long = 156 feet, at \$14.50 per M.</li> <li>466 feet matched stuff, at \$17.50 per M.</li> <li>3½ M shingles, at \$2.75 per M.</li> <li>14 pounds 3d. nails.</li> <li>10 pounds 8d. and 10d. nails.</li> <li>Framing and putting in place 156 feet 2 x 4 scantling, at \$8 per M.</li> <li>4 squares of roof boarding, at 50 cents per square.</li> <li>4 squares of shingling, at \$1.25 per square.</li> </ul>	\$2.26 8.16 9.17 .63 .30 1.25 2.00 5.60
Total	. 63

This sum, in turn, divided by 4 gives as the cost of a single square, \$7.35.

The following is an analysis of cost of 4 squares of flooring, laid on joists  $2 \times 8$  inches, the flooring being selected from No. 1 boarding, and the joists being placed 16 inches between centers. Allowance is made for doubling where necessary.

## ANALYSIS OF FLOORING

17 joists,  $2 \ge 8$  inch, 20 feet long = 459 feet, at 

 17 joists, 2 x 8 men, 20 reet long = 405 reet, at

 \$14.50 per M.

 466 feet of flooring, at \$17.50 per M.

 815 feet of 1 x 2 inch bridging, at 2 cents.

 30

 10 pounds of 8d. common nails..... 3 pounds of spikes.... Laying 4 squares of flooring, at 50 eents per square 2.00 Framing 459 feet of joists, at \$5 per square..... 2.30 Bridging. .50 Total. ......\$20.28

Dividing this amount by 4, as in the previous cases, gives \$5.07 as the cost of 1 square of flooring.

The following is an analysis of the cost of an inside door, 2 feet 8 inches by 6 feet 10 inches, 13 inches thick, cased and finished complete except the one item of painting:

## ANALYSIS OF COST OF DOOR

Frame, 2 set casings - 1 DOOR	
18 feet of moultings and stops.	
1 thousand at 21 inches	\$2.00
I unreshold, hardwood	98
I first quality door	1.5
31-inch marting door, size as given above	.15
of men mortleed loek, bronzo fues half	1.95
ing plate	
Porcelain knobe what is	00
1 pair of 21	- 03
Setting of 32-meh japan butts and	.40
Setting frame.	95
Casing up 2 sides	
Putting down at a sides	. 25
Maul 1 Marshold	.40
moulding, I side	15
"itting, hanging and a	. 10
er mignig and trimining door.	.20
Tetal	. 75
Total.	
	17 10
	1.42

The following is an analysis of cost of a 4-light window, with sash  $14 \times 30$  inches,  $1\frac{3}{8}$  inches thick, checkrail, the window set, cased and finished complete.

### ANALYSIS OF COST OF WINDOW

3371 1

window frame prepared for weights	
Sash glazed	\$2.15
20 feet 21-inch moulding	2.10
25 feet inside case and mind	. 30
28 pounds of sash weight	.75
Sash cord	. 56
Grounds for plastering and	.18
Setting frame	.30
Casing up	. 25
Fitting sash	. 55
Nails.	.15
Sash lock	.10
Putting on such look	. 25
on sash lock	. 10
Total -	
	7 64

Add to the foregoing not less than 30 per cent, but it is better in all cases that local prices of material and labor be embodied in the analysis.

## ESTIMATING FOR OUTSIDE DOOR AND WINDOW FRAMES

For ordinary buildings, either wood or brick, the following prices, which are for labor only, will be found to be as nearly correct as possible where local conditions are unknown. For simply making the frames, setting same, hanging sashes, doors, blinds, etc., the number that can be made, hung. or set in a day of nine hours, is given, as well as the price which will enable the estimator to tell approximately the cost of any number of frames either in place or out.

3.0	No. of Pieces in	Price for
Making plane frames for moint t	Day's Work	Each
Setting frames in wall	3	\$1.00
Hanging ontside blind	14	
Hanging inside hit 1	10	. 44
Fitting and Dinds, 50c. to \$1.0	0 5	.00
Handling sash per window.	10	.00
Casting sash, trimming, locks and l	lifto 14	.18
Casing.	14	. 23
Putting on stops	•••• 10	. 30
Band moulding	· · · · 35	.09
Fitting stool	$\cdots 25$	.12
Fitting apron	· · · · 13	21
	25	19
Total	-0	.12
Local,		00 40

Fitting and hanging doors on outside frames, trimming with 4-inch loose pine, joint hinges, mortise lock, bronze or plated rose, hardwood knob, night latch, and all complete, three hinges to the door, door 1<sup>3</sup>/<sub>4</sub>-inch thick, pine, to complete \$1.95. If two hinges, and 1<sup>3</sup>/<sub>8</sub>-inch door, \$1.50. If hardwood, add 15 per cent.

If frames are bought at the factory all ready made, no blinds to hang, no band mouldings to plant, then the cost for setting, hauging, casing complete on one side, doors or windows, will be \$1.25.

The average quantity of material required to make frames for common houses, running measure, allowing for waste and joints on the basis of a 2-light window, with glass  $24 \times 36$  inches, and a door measuring 2 feet 8 inches by 6 feet 8 inches, is given in the following table, which covers all the items required to complete common frames:

Window jambs and heads with dat	Feet
Door jambs and heads	18
Outside casing, window	18
Outside easing, door	18
Inside casing, window with another	19
Inside casing, door, each side	20
of the stude	18

About the same number of feet in length will be required for mouldings and stops.

### TABLE FOR ESTIMATING NAILS

1000 shingles require 3½ pounds 4d. nails.
1000 lath require 6½ pounds 3d. nails.
1000 feet of beveled siding require 18 pounds 6d. nails.
1000 feet of sheeting requires 20 pounds 8d. nails.
1000 feet of sheeting requires 25 pounds 10d. nails.
1000 feet of flooring requires 30 pounds 8d. nails.
1000 feet of flooring requires 35 pounds 10d. nails.
1000 feet of studding requires 14 pounds 10d. nails.
1000 feet of studding requires 10 pounds 20d. nails.
1000 feet of furring, 1 x 2, requires 10 pounds 10d. nails.
1000 feet of <sup>7</sup>/<sub>8</sub> finish requires 30 pounds of 8d. nails.

The following table shows the name, length and number of nails to the pound of the different sizes:

ranne	Length	
3d fine.	1 mab	No. to a pound
3d common	11: 1	· · · · · · 1150
4d common	••••••••••••••••••••••••••••••••••••••	· · · · · · 720
5d common	····· Ig inch	432
6d Cut 1	$\ldots$ 1 $\frac{1}{2}$ to 1 $\frac{3}{2}$ inch $\ldots$	352
ou nnish		
od common	2 inch	
7d common		100
8d finish		100
8d common		190
9d common	24 inch	132
0d finish	2 in al	$\dots 110$
Od common	·····	137
2d common		87
Od common		66
od common		
$o_1$ common $\dots$	•	
va common		
Ud common		15

2345

### NUMBER OF NAILS TO THE POUND
# HODGSON'S ESTIMATOR

# PAINTERS' MEASUREMENTS

In England the custom is to employ a clerk quick at figures, whose duty it is to take off, from the plans and specifications, an accurate list of all the materials and labor required in the performance of the work, setting down in each case the number of yards or feet, as the case may be, of each item. In the case of painting, the figures obtained for the carpenter and joiner prove of service also for the work to be done by the painter. The following is a table that is intended to indicate the method of measurement of painters' work, and also the order in which the various items may be taken. A similar table added to, or changed, as might be necessary to suit American methods of construction, would be very useful to have on hand when getting out estimates, as it would insure nothing The table which follows accurately indicates the English practice.

Lead, in oi	lon v	white	work,	at	Der	vard a		
Ornamonto	с 1	ement	"	4.6		yang si a	uper.	
Skylighte	railin	ngs, et	te.,	"	66	"	-11	
Skirtinge 1	o :			44	~ ~	"		
Strings, 1	2-m.	girth	and	unde	r, at-	Der	foot	
Chair rails	"			46	4.6		1001	run.
Hand "				"	66	"	"	"
Balusters	"			"	44	**	"	
Newels.	"	 			**	"	"	"
Rain pipes.	"				"	~~	"	"
Ornamental	honda				"	66	"	"
Ears,	ncaus				66		num	han
Shoes,					**		**	oer.
Eaves, gutter					<b>«</b> «		**	
topped ends.					"	f	oot r	Un
Jutlets,	,				"		numh	er.
wan necks,					"		44	
					"		"	

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# AND CONTRACTOR'S GUIDE

Cement reveals (inmba)		
Cornices under - girth	at	foot run.
Window sills "		66
Coping edge "		66
Stone strings "		66
Stone plinths "		"
Iron castings "		**
Grate bars "		"
Sash squares		66
Sash frames		dozen.
Small "		number.
Two-light casement from a		66
Four " " "		"
Sash squares	••	"
Brackets		dozen.
Finials	**	number.
Step ladders	"	"
Dressers	"	"
Chimney pieces	**	"
Four oils and ortro fairly 1		• 6
grav	rnish,	
Grainer: extra grain for	"	Yard super.
twice ynmich	tand	
Grainer: extra ancia - 1		66 66
brackets t inil	t for	
Stainer: stain to	**	foot run.
and twice normal in the	tint	
consil versich	best	
French polishers Erect 1 11 1	~ ~	foot super.
French polishing to be designed	5, "	66 66
Gilder: gilding on Art.	66	foot run.
Gilder op genued me but surface,	**	foot super.
and description	ight	
Moulded work that	"	foot run.
Boardy etc. "	"	6+ 66
Carried cons	"	66 66
Brassos and dimenta to		66 66
nature	ilar	
nature.	66	

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## HODGSON'S ESTIMATOR

## TO FIND THE NUMBER OF SINGLE ROLLS OF PAPER NEEDED FOR ANY GIVEN ROOM.

To find the number of single rolls required for a wall, multiply the distance around the room by the height, taking out 20 square feet for each opening, and divide by 30. To find the number of rolls for the ceiling, multiply the length by the width and divide by 30. The number of yards of border required can easily be measured.

For example, room  $12 \times 14$ , 10 feet high, two doors and three windows:

Length, two walls, 14 feet coal		
Width, two walls, 12 feet each	28	feet
, a rect caen	24	4.4
Multiply by height	52	6.6
	10	4.6
Less five openings, allowing 20	520	44
Divided by number of so ft :	00	**
$1$ a roll $\dots$ $30)4$	20(1	4 rolls
•	re	quired
	30	•
12	20	
10	0	

To find the quantity of border required, divide length around the room, 52 feet, by 3, equal to about 18 yards.

The price of border is for a single strip, the width of the border and one yard long.

The price of the paper is for a single roll, one-half yard wide and eight yards long. waste, this will cover 30 square feet. Allowing for all The following table will be useful to the estimator:

a he g, ne le n S

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	35	52	to	12	30	-	3	5		1	1	1		81 4	-	ē	3		-611
	Tr.	52	3	1.	-	-	16	1 0	1	-	113		116	1-	n	8			J. G.
00	33	19	10	5	37	*	-	1			12 -	1	15	<u>t-</u>	÷1	- 28			hq.
3	-	10	9	-	1					11	2	13	15	164	- 51	E.			ted
31		24	2	19		-	77		2	2	13	13	=	10	10	50		100	righ
9	2	5	10	10	148		*			2	=	13	11	154	02			er i	oby
00		2-80 1-91	27		-	=		-		<b>1</b> 10		51	13	15	8	13		5. ?	is e
X	- -		-	*	27		-41	-100	0) 194	0	10	12	13	14	193	-		TI AL	ble
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20	18		10 1		10 H					- 1	419 1	- UC	30	5	-	<u> </u>	ate \$32	t pe	10
19		10	2							) G		0		2 .	2	16	lth.	cos	ad.
18		32	23	-	-	N 140	40 • • •					0 0	0	2	2	9	it th	4.)	e or
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	-	140	~		12	170 170	<del>م</del>	32 3	32 3	32 4	-	15	2	1.		-1	10-10-10-10-10-10-10-10-10-10-10-10-10-1	12	owe
6	-	12	16	61	24	ter.	23	5	00	3	33	+++	44 5	8	X	-1	D Hi	so h	pd le
×	11	÷ - +	-	140	12	-		-	21 21	~	-	32				-		un si o	rat
9	-	1+1	+		11	11	0) (14)	54	5	2	3	34	31	**	8	-	s of	ot r th 3	ghe
10	1-100	-	_	+	141	-	1	-8.1	-	02	-	40.4	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		10	-	ure.	w In	r hi
*	-		-		-	=	-	+		141	41	2 11 2	63	-			alur	doe	S, 0
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I.

## HODGSON'S ESTIMATOR

## CONCLUSION

In conclusion I would suggest the following simple method of keeping i record of cubic contents and cost, and would say the the information an architect has of this kind from bis own buildings is the best for him, as it is probable that no other architect is quite similar in his style  $\epsilon$  cork and finish.

A book or a number of the two means ruled in suitable width for the following columns: 1st, date (year); 2d, name of building (for owner); 3d, where erected; 4th, short descriptio : 5th, cubical contents in feet; 6th, cost of building; 5th, cost per cubic foot; 8th, remarks. The 1 ads of buildings should be classified so that prices one class may be seen and compared at a glance in one column. An example is here shown.

Date	Maria						
· wre	TASTIN.	W liere Resstr	Description	Cubie		Cts	
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In computing the cubical content the rule most commonly used is to measure the building as a whole or in parts from the bottom of the footings to a point halfway up the slope of roof, this being done is parts where there are different heights of roofs, towe s, etc. In measuring brick or stone buildings, light soden porticos or verandas are usually omitted. There should be a uniform system of omitting or in lading such items as heating, mantels, grates and tills, electric wiring, or of noting two rates, one omiting at the other including these.

## AND CONTRACTOR'S GUIDE

In eed, an exact r dof the cost of all buildings the outract a new a should be kept, and any thing peculi r or a ominon or unisual should be noted, that is the full re the knowle ge obtained in this manier r, where r is odd purpose.

this work ve be ral schemes for e imatic and we com ws as to their resp. t. mer. . . . . . . . e examined into the quest of est at lam confirmed in the visco spresse in rest es e the work, numery, the "rolex i tii ng can be , iven " n. hat t. best. relia way is to es n' tail. of methods h e certain ge prod out, ru they lack reliability, a q ty t! in itractor does not want to be up ag t: It or he should follow the safer, if more l'briot w v or figuring on every item goit to though is about to tender for. A ce rate artist or explained that his success as nt. rose i s following the rule, "First know writ you do ind then do it." So here, before anyt in be lone, it is necessary for careful plans to have  $\rightarrow$  show what is wanted, and these and he date fully studied and every item nown them ir described in the specifications should 26.

rus g my efforts will prove useful to the young n rogr sive workman who has a desire to become atrana and that they will aid and assist him in ttering t and with this hope in view I close this volume

ple nd for ite be t, d, al r s e n



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÷ t SECOND FLOOR PLAN -£



Floor Flans of "The Rae"



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Floor Plans of "The Watkins"

÷



Width, 26 feet Length, 56 feet

SIZE

Blue prints consist of floor plans; roof plan; front and side elevations. Complete typewritten specifications with each set of plans.





Floor Plan of "The Gastonia"

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SIZE Width, 30 feet Length, 42 feet

> Blue prints consist of floor plan; roof plan; front and side elevations. Complete typewritten specifications with each set of plans.





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SIZE

Width, 34 feet Length, 48 feet Blue prints consist of cellar and foundation plan; floor plan; roof plan; front and side elevations. Complete typewritten specifications with each set of plans.



Floor Plans of "The Orchard Crest"



Blue prints consist of cellar and foundation plan; roof plan; floor plans; front and slde elevations. Complete typewritten specifications with each set of plans.







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Floor Plans of "The West Baden"





Width, 44 feet

Width, 44 feet Length, 28 feet Blue prints consist of cellar and foundation plan; floor plans; roof plan; front and side elevations. Complete typewritten specifications with each set of plans.





