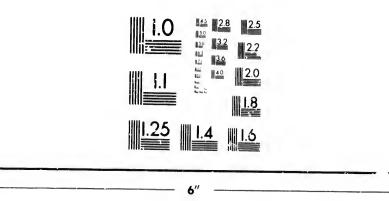


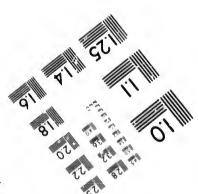
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# Canadian Contractor's Hand-Book,

A COMPENDIUM OF

#### USEFUL INFORMATION

FOR PERSONS ENGAGED ON WORKS OF CONSTRUCTION.

#### SECOND EDITION

PUBLISHED BY

### CHAS. H. MORTIMER,

TORONTO, CANADA.

TORONTO: CANADIAN ARCHITECT AND BUILDER PRESS, 1893.

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#### PREFACE TO SECOND EDITION.

THE first edition of the CANADIAN CONTRACTOR'S HAND-BOOK was issued in 1889. For a time the book was given as a premium to new subscribers to the CANADIAN ARCHITECT AND BUILDER with the object of introducing that journal, which was then in its infancy, as rapidly and widely as possible to Canadian builders.

Since ceasing to use the book for this purpose, there has been such an unexpected demand for it on the part of contractors, architects and others, as to make it appear desirable to publish this second edition.

The contents of the first edition have been thoroughly revised, and any data not applicable to the present time omitted. Upwards of seventy-five pages of new and most valuable information have been added to the present edition.

It is hoped that the Hand-Book in its present form and at the reasonable price at which it is offered, will commend itself to Canadian builders, contractors, architects, etc., for whose information especially it has been compiled.

THE PUBLISHER.

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#### THE MECHANICS' LIEN ACT.

ER Majesty, by and with the advice and consent of the Legislative Assembly of the Province of Ontario, enacts Short Title. as follows :-

I. This Act may be cited as "The Mechanics' Lien Act." R.

S. O. 1877, C. 120, S. I.

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II. Where the following words occur in this Act, or in the Interpretation. schedules thereto, they shall be construed in the manner hereinafter mentioned, unless a contrary intention appears:

I "Contractor" shall mean a person with or employed directly by the owner for the doing of work or placing or furnishing "Contractor." of machinery or materials for any of the purposes mentioned in this Act;

"Sub-contractor" shall mean a person not contracting with "Sub-Contractor." or employed directly by the owner for the purposes aforesaid, but contracting with or employed by the "contractor" or under

him by another "sub-contractor;"

"Owner" shall extend to and include a person having any estate or interest in the lands upon or in respect of which the work is done, or materials or machinery are placed or furnished, at whose request and upon whose credit or upon whose behalf or with whose privity or consent or for whose direct benefit any such work is done, or materials or machinery placed or furnished, and all persons claiming under him, whose rights are acquired after the work in respect of which the lien is claimed is commenced, or the materials or the machinery furnished have been commenced to be furnished. R. S. O. 1877, C. 120, S. 2.

III. No agreement shall be held to deprive anyone otherwise Person not deprived entitled to a lien under this Act, and not a party to the agreement, of lien by agreement, of the benefit of the lien, but the lien shall attach, notwithstand-

ing such agreement. 47 V. C. 18, S. 1, part.

4. Unless he signs an express agreement to the contrary, every mechanic, machinst, builder, miner, laborer, contractor or other person doing work upon, or furnishing materials to be used in, the construction, alteration or repair of any building or erection, or erecting, furnishing or placing machinery of any kind in, upon or in connection with any building, erection or mine, Mechanics and others shall by virtue of being so employed or furnishing, have a lien for to have liens for work the price of the work, machinery or materials, upon the building, done, etc. erection or mine, and the lands occupied thereby or enjoyed therewith, limited in amount to the sum justly due to the person entitled to the lien. R. S. O. 1877, c. 120, s. 3; 47 V. c. 18, s. 1, part.

V. (1) The lien shall attach upon the estate and interest of the owner, as defined by this Act, in the building, erection or Upon what property mine upon or in respect of which the work is done or the materials or machinery placed or furnished, and the land occupied.

thereby or enjoyed therewith.

(2). In cases where the estate or interest charged by the lien is leasehold, the fee simple may also, with the consent of the owner When the estate thereof, be subject to said charge, provided such consent is charged is leasehold, testified by the signature of such owner upon the claim at the The fee may be time of the registering thereof, and duly verified. R. S. O. 1877, cases. c, 120, s. 6, 47 V. c. 18 s. 5.

Mortgaged land.

In case the land upon or in respect of which any work as aforesaid is executed, or labor performed or upon which materials or machinery are placed is incumbered by a prior mortgage or other charge, and the selling value of the land is increased by the construction alteration or repairs of the building, or by the erection or placing of the materials or machinery, the lien under this Act shall be entitled to rank upon the increased value in priority to the mortgage or other charge. R. S. O. 1877, c. 120, s. 7; 45 V. c. 15, s. 13.

VI. (1). Without prejudice to any lien which he may have under the preceding sections, every mechanic, laborer or or other person who performs labor for wages upon the construction, alteration or repairs of any building or erection, or in erecting Lien for 30 days' or placing machinery of any kind in, upon, or in connection with, any building, erection, or mine, shall to the extent of the interest of the owner have, upon the building, erection, or mine, and the land occupied thereby and enjoyed therewith, a lien for such wages, not exceeding the wages of thirty days, or a balance equal to his wages for thirty days.

> (2). The lien for wages given by this section shall attach when the labor is in respect of a building, erection or mine belonging to the wife of the person at whose instance the work is done upon the estate or interest of the wife in such property, as well as upon

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that of her husband. 45 Vict. c. 15, ss. 2, 3.

In all cases, the owner shall, in the absence of a stipu-Owner may retain to lation to the contrary, be entitled to retain for a period of thirty per cent. of contract days after the completion of the contract, ten per centum of the price to be paid to the contractor. 45 V. c. 15, s. 5.

VIII. In case the lien is claimed by a sub-contractor, the amount which may be claimed in respect thereof shall be limited Claim by sub-con to the amount payable to the contractor or sub-contractor (as the case may be) for whom the work has been done, or materials or machinery have been furnished or placed. R. S. O. 1877, c.

120, s. 6.

IX.—(1). All payments, up to ninety per centum of the price to be paid for the work, machinery or materials, as defined by section 4 of this Act, made in good faith by the owner to the contractor, or by the contractor to the sub-contractor, or by one sub-contractor to another sub-contractor, before notice in writing, by the person claiming the lien has been given to such owner, contractor or sub-contractor (as the case may be), of the claim of such person, shall operate as a discharge pro tanto of the lien created by this Act, but this section shall not apply to any payment made for the purpose of defeating or impairing a claim to a lien existing or arising under this Act. 41 Vic. c. 17, s. 1.

A lien shall, in addition to all other rights or remedies given by this Act, also operate as a charge to the extent of ten per centum of the price to be paid by the owner for the work, machinery or materials as defined by section 4 of this Act up to ten days after the completion of the work or of the delivery of the materials, in respect of which such lien exists, and no longer, unless notice in writing be given as herein provided. 41 V. c.

17, S. 2. (3). A lien for wages for thirty days, or for a balance equal to

wages.

Property affected.

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the wages for thirty days, shall, to the extent of the said ten per cent. of the price to be paid to the contractor, have priority over Priority of lien for 30 all other liens under this Act, and over any claim by the owner days' wages. against the contractor for, or in consequence of the failure of the latter to complete his contract. 45 V. c. 15, s. 4.

X. Save as herein provided, the lien shall not attach so as Extent of owners' lito make the owner liable to a greater sum than the sum payable ability. by the owner to the contractor. R. S. O. 1877, c. 120, S. 6, part;

45. V. C. 15, S. 4.

XI. All persons furnishing material or doing labor for the person having a lien under this Act, in respect of the subject of such lien, who notify the owner of the premises sought to be affected thereby, within thirty days after such material is furn-Notice to owner of ished, or labor performed, of an unpaid account or demand claims against lienagainst such lien-holder, for such material or labor, shall be en-holders. titled, subject to the provisions of sections 6 and 9, payable by such owner, under said lien; and if the owner thereupon pays the amount of such charge to the person furnishing material, and doing labor as aforesaid, such payment shall be deemed a satisfaction *pro tanto* of such lien. R. S. O. 1877, C. 120, S. 8.

XII. In case of a dispute as to the validity or amount of an unpaid account or demand, of which notice is given to the owner under the preceding section, the same shall be first determined by action in the proper court in that behalf, or by arbitration, in Disputes as to claims manner mentioned in section 14 at the option of the person having the unpaid account or demand against the lien holder; and pending the proceedings to determine the dispute, so much of the amount of the lien as is in question therein may be withheld from the person claiming the lien. R. S. O. 1877, C. 120, S. 9.

XIII. In case the person primarily liable to the person giving such notice as mentioned in section 11 fails to pay the amount awarded within ten days after the award is made, the owner, contractor or sub-contractor may pay the same out of any Failure to pay amount moneys due by him to the person primarily liable as aforesaid, awarded. on account of the work done or materials or machinery furnished or placed in respect of which the debt arose; and such payment if made after an award (or if made without any arbitration havbeen previously had or dispute existing, then, if the debt in fact existed and to the extent thereof,) shall operate as a discharge pro tanto of the moneys so due as aforesaid to the person primarily liable. R. S. O. 1877, C. 120, S. 10.

XIV. (1.) In case a claim is made by a sub-contractor in Disputed claims of respect of a lien to which he is entitled, and a dispute arises as sub-contractors to be to the amount due or payable in respect thereof, the same shall referred to arbitration be settled by arbitration.

(2). One arbitrator shall be appointed by the person making the claim, one by the person by whom he was employed, and the Appointment of arbitrators. third arbitrator shall be appointed by the two so chosen.

(3). The decision of the arbitrators or a majority of them Decision to be final. shall be final and conclusive. R. S. O. 1877, c. 120, s. 18.

(4). In case either of the parties interested in any such dispute, refuses or neglects within three days after notice in writing Refusal to appoint arrequiring him to do so to appoint an arbitrator, or if the arbitrators. requiring him to do so, to appoint an arbitrator, or if the arbitra-

tors appointed fail to agree upon a third, the appointment may be made by a County Judge of the county in which the lands in respect of which the lien is claimed are situate. R. S. O. 1877,

C. 120. S. 19.

moved.

During the continuance of a lien, no portion of the pro-Property affected by perty or machinery affected thereby, shall be removed to the the lien not to be re- prejudice of the lien; and any attempt at such removal may be restrained by application to the County Court or the Judge thereof, or the High Court respectively, according as the claim is under or over the sum of \$200. R. S. O. 1877, c. 120, s. 22.

Claim may be registered.

XVI. (1). A claim of lien applicable to the case, may be registered in the registry division in which the land is situate, and shall state:

(a). The name and residence of the claimant and of the owner of the property to be charged, and of the person for whom and upon whose credit the work is done or materials or machinery furnished, and the time or period within which the same was, or was to be, done or furnished;

The work done or materials or machinery furnished;

The sum claimed as due, or to become due; The description of the land to be charged;

The date of expiry of the period of credit agreed to by the lien-holder for payment for his work, materials or machinery, where credit has been given.

The claim may be one of the forms given in the schedule in this Act, and shall be verified by the affidavit of the claimant, Affidavit of verifica or of his agent or assignee having full knowledge of the matters tion may be made by required to be verified, and the affidavit of an agent or assignee shall state he has such knowledge. R. S. O. 1877, c. 120, s. 4 (1, 2); 47 V. c. 18, ss. 2, 3.

be combined.

Fee.

XVII. A claim for wages may include the claims of any number of mechanics, laborers, or other persons aforesaid, who may Claims for wages may choose to unite therein. In such case each claimant shall verify his claim by his affidavit, but need not repeat the facts set out in the claim; and an affidavit substantially in accordance with form 4 in the schedule to this Act, shall be sufficient: 45 V. c. 15, ss. 8, 10.

XVIII. (1). The registrar, upon payment of his fee, shall register the claim, so that the same may appear as an incum-Registration of claims brance against the land therein described. R. S. O. 1877, c. 120, s. 5; 47 V. c. 18, s. 4, puri.

(2). The fee for registration shall be twenty-five cents; if several persons join in one claim, the registrar shall have a further fee of ten cents for every person after the first. 45 V. c. 15, S. I ..

(3). The registrar shall not be bound to copy in any registry Mode of registration, book any claim or affidavit, but he shall number each claim, and shall insert in the alphabetic and abstract indexes, the like particulars as in other cases; he may describe the nature of the instrument as "Mechanics' Lien." 45 V. c. 15, s. 11.

XIX. Where a claim is so registered, the person entitled to Registry Act to ap-the lien shall be deemed a purchaser pro tanto, and within the ply. Rev. Stat., c. provisions of The Registry Act, but except as herein otherwise

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d to the wise provided, The Registry Act shall not apply to any lien arising under this Act. R.S. O. 1877, c. 120, ss. 4 (3), 26.

XX. (1). Where the lien is for wages under sections 6 or 9, the claim may be registered,

(a) At any time within thirty days after the last day's labor

for which the wages are payable, or

(b) At any time within thirty days after the completion of Time for registration of of claim for wages. the construction, alteration or repair of the building or erection, or after the erecting or placing of the machinery, or in towards which, respectively, the labor was performed and the wages earned, but so that the whole period shall not exceed sixty days from the last day's labor aforesaid.

(2). Such lien shall not be entitled to the benefit of the provisions of sections 6 : d 9 after the said respective periods, unless the same is duly registered before the expiration of the said

periods so limited. 45 V. c. 75, s. 6.

(3). Such lien shall have the same priority for all purposes

after as before registration.

XXI. In other cases the claim may be registered before Time for registering or during the progress of the work, or within thirty days from claim not arising unthe completion thereof, or from the supplying or placing the der s. 5.

machinery. 45 V. c. 15, s. 7.

XXII. Every lien which has not been duly registered under the provisions of this Act shall absolutely cease to exist on the expiration of the time herembefore limited for the registration thereof, unless in the meantime proceedings are instituted to realize the claim under the provisions of this Act, and a certifi-When unregistered cate thereof (which may be granted by the Court or a Judge 1.2- lien shall cease. fore whom or in which the proceedings are instituted), is duly registered in the registry office of the registry division wherein the lands in respect of which the liep is claimed are situate. R. \$. O. 1877, c. 120, s. 20.

XXIII. Every lien which has been duly registered under the provision of this Act shall absolutely cease to exist after the expiration of ninety days after the work has been completed, or materials or machinery furnished, or wages earned, or the expiry of the period of credit, where such period is mentioned in the When registered lien claim of lien filed, unless in the meantime proceedings are insti-shall cease. tuted to realize the claim under the provisions of this Act, and a certificate thereof (which may be granted by the Court or Judge before whom or in which the proceedings are instituted), is duly registered in the registry office of the registry division wherein the lands in respect of which the lien is claimed are Bituace. R. S. O. 1877, c. 120, s. 21.

XXIV. If there is no period of credit, or if the date of expiry of the period of credit is not stated in the claim so filed, the lien shall cease to exist upon the expiration of ninety days after the When lien to cease. work has been completed or materials or machinery furnished, unless in the meantime proceedings shall have been instituted

pursuant to section 23 of this Act. 47 V. c. 18, s. 2.

XXV. In the event of the death of a lien-holder, his right of lien shall pass to his personal representatives; and the right of Death of lien-holder. a lien-holder may be assigned by any instrument in writing. R. S. O. 1877, c. 120, s. 16.

Discharge of liens.

discharge.

XXVI. A lien may be discharged by a receipt signed by the claimant, or his agent, duly authorized in writing, acknowledging payment, and verified by affidavit and filed; such receipt shall be numbered and entered by the registrar like other instruments, but need not be copied in any book; the fees shall be the same as for registering a claim or lien. 45 V. c. 15, s. 15; 47 V. c. 18, s. 4.

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XXVII. Where there is a contract for the execution of the work, as hereinbefore mentioned, the registration of all dis-Cost of registering charges of liens shall be at the cost of the contractor, unless a court or judge otherwise orders. 45 V. c. 15, s. 16; 47 V. c. 18,

XXVIII. (1) Where the amount of the claims in respect of any lien is within the jurisdiction of the County or Division Courts respectively, proceedings to recover the same, according to the usual procedure of the said court by judgment and execution, may be taken in the proper Division Court or in the County Court of the County in which the land charged is situate; or proceedings may be taken before the judge of the said Enforcement of lien courts, who may proceed in a summary manner by summons and order, and may take accounts and make requisite enquiries, and in default of payment may direct the sale of the estate and interest charged, and such further proceedings may be taken as the judge directs.

(2). Any conveyance under the seal of the County Court Judge shall be effectual to pass the estate or interest sold.

The fees and costs in all proceedings taken under this section shall be such as are payable in respect of the like or similar matters according to the ordinary procedure of the said courts respectively. R. S. O. 1877, c. 120, s. 12.

XXIX. In cases other than those specified in the preceding section the lien may be realized in the High Court, according to the ordinary procedure of that court. R. S. O. 1877, c. 120, s. 13. XXX. (1) Any number of lien-holders may join in one

action, and any action brought by a lien-holder shall be taken to be brought on behalf of all the lien-holders of the same class Action by lien-holder who shall have registered their liens before or within 30 days to be for joint benefit, after the commencement of the action, or who shall within the said 30 days file in the proper office of the court from which the writ issued a statement entitled in or referring to the said action, of their respective claims.

(2). In the event of the death of the plaintiff, or his refusal or neglect to proceed, any other lien-holder of the same class who Prosecution of claim has registered his lien or filed his claim in the manner and withwhen plaintiff dies, in the time above limited for that purpose, may be allowed to prosecute the action on such terms as may be deemed just and reasonable. 47 V. c. 18, s. 6.

(3). In case of a sale of the estate and interest charged with Time when sale may the lien, the court or judge may direct the sale to take place at any time after one month from the recovery of judgment, and it shall not be necessary to delay the sale for a longer period than is requisite to give a reasonable notice thereof.

(4). The said court or judge may also direct the sale of any machinery and authorize its removal. R. S. O. 1877, c. 120, s. 14.

in a Division Court.

Enforcing lien in High Court.

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Where judgment is given in favor of a lien, the court or judge may add to the judgment the costs of and incidental to Costs, registering the lien as well as the costs of the action. 45 V. c.

Where there are several liens under this Act against the same property, each class of the lien-holders shall, subject to the provisions of sections 5, 9 and 11, rank pari passu for their sev-Several liens. eral amounts, and the proceeds at any sale shall, subject as aforesaid, be distributed amongst them *pro rata*, according to their several classes and rights, and they shall respectively be entitled to execution for any balance due to them respectively after said distribution. R. S. O. 1877, c. 120, s. 17.

(7). Upon application to the County Court, in claims under \$200, and to the High Court in other cases, the court or judge in lieu of lien. may receive security or payment into court in lieu of the amount of the claim, and may thereupon vacate the registry of the lien.

(8). The court or judge may annul the said registry upon any Registry may be an-

other ground. R. S. O. 1877, c. 120, s. 23.

(9). In any of the said cases mentioned in sub-sections 7 and 8, the court or judge may proceed to hear and determine the matter of the said lien, and make such order as seems just, and Wrongful claim or in case the person claiming to be entitled to such lien has refusal to discharge wrongfully refused to sign a discharge thereof, or without just costs. cause claims a larger sum than is found by such court or judge to be due, the court or judge may order and adjudge him to pay costs to the other party. R. S. O. 1877, c. 120, s. 24; 47 V. c. 18, s. 7.

XXXI. Where any mechanic, artisan, machinist, builder, uniner, contractor or other person, has furnished or procured materials for use in the construction, alteration or repair of any building, erection or mine, at the request of and for some other When the materials person, such materials shall not be subject to execution or other tion of buildings are process, to enforce any debt (other than the purchase thereof) not to be subject to due by the person furnishing or procuring such materials, and executions. whether the same have or have not been in whole or in part worked into or made part of such building or erection. R. S. O.

1877, C. 120, S. 25.

XXXII. (1) Every mechanic or other person who has bestowed money or skill and materials upon any chattel or thing in the alteration and improvement in its properties or for the purpose of imparting an additional value to it so as thereby to be entitled to a lien upon such chattel or thing for the amount or value of the money or skill and materials bestowed, shall, while such lien exists but not afterwards, in case the amount to which he is entitled remains unpaid for three months after the same ought to have been paid, have the right in addition to Mechanics entitled to all other remedies provided by law to sell the chattel or thing in sell the chattleif (after respect of which the lien exists, on giving one week's notice by three months) payadvertisement in a newspaper published in the municipality in ment is not made. which the work was done, or in case there is no newspaper published in such municipality, then in a newspaper published nearest thereto, stating the name of the person indebted, the amount of the debt, a description of the chattel or thing to be sold, the time and place of sale, and the name of the auctioneer,

and leaving a like notice in writing at the last or known place of residence (if any) of the owner, if he be a resident of such municipality.

Application of ceeds of sale.

(2) Such mechanic or other person shall apply the proceeds and sale, and shall upon application pay over any surplus to the person entitled thereto.

(2) Such mechanic or other person shall apply the proceeds for advertising and sale, and shall upon application pay over any surplus to the person entitled thereto.

#### ERRATA.

Sec. XI, page 11, line 7, after the words "sections 3 and 9," insert the words "to a charge therefor pro rata upon any amount."

Sec. XVI (1), page 12, line 2, after words "in the registry" insert the words "office of the registry," etc.

Sec. XIX, last line, page 13, add "47 V, c. 18, s. s. 2-3."

NOTE.—Subsection 1 of section 30 of this act is amended; see section 39 of chapter 37 following.



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#### AN ACT TO SIMPLIFY THE PROCEDURE FOR ENFORCING MECHANICS' LIENS.

(Assented to 7th April, 1890.)

ER Majesty, by and with the advice and consent of the Legislative Assembly of the Province of Ontario, enacts as follows :---

I. Any person claiming a mechanics' lien may enforce the Procedure for ensame by means of the proceedings hereinafter set forth.

2. Without issuing a writ of summons or taking any other preliminary proceeding, the plaintiff may file a statement of Statement of claim claim in the office of a master or official referee having jurisdic- to be filed. tion in the county wherein the lands in question are situate.

3. Such statement of claim shall be verified by affidavit. Upon the filing of such statement of claim and affidavit, the master or claim and certificate referee shall issue a certificate in duplicate of the filing of the of filing.

4. Upon the registration of such certificate in the proper registry office, or lands titles office, the action shall be deemed Registration of certo have been commenced as against the owner and all other tificate, necessary parties to the action.

5. The master or referee shall also, in and by such certificate, appoint a time and place at which he will inquire into the claim Certificate to name of the plaintiff and take all necessary accounts; such certificate time and place for and appointment shall be issued in duplicate and may be in the taking accounts. form set forth in the schedule hereto.

6. A copy of such certificate and appointment shall be served Service of copy of on the owner and all other proper parties at least ten days before certificate and apthe day therein named for taking the first proceeding thereunder. pointment.

7. Within ten days after the service of such certificate and Notice disputing appointment, any person served therewith may file a notice dis-claim. puting the plaintiff's right to a lien.

8. In case a notice disputing the plaintiff's lien is filed, the master or referee shall, before taking any further proceeding, Determination of the determine the question raised by the notice, or may adjourn the question raised by question before a judge in chambers, and if so required by any parties may thereupon issue a certificate of his finding.

9. But if not required to issue such last named certificate, it shall suffice for the master or referee to enter in his book a note master. Entry of finding of of his finding.

10. Where no notice disputing the plaintiff's lien is filed as atoresaid, and the proceedings are instituted by a sub-contractor, the owner is to file in the office of the master or referee a state- Where claim not disment of account showing what, if anything, he admits to be due, puted, owner to file for the satisfaction of the plaintiff's lien and all other liens of the statement of amount, if any, admitted to same class as the plaintiff's. Such statement is to be filed at be due. least four days before the day named in the certificate mentioned in section 5 for taking accounts, and in case the owner shall not file such statement, or shall file an untrue statement, he may be

forcing liens.

ordered by the master or referee to pay all costs incurred in establishing the true amount due and owing from him.

11. All lienholders of the same class served with the appointment or who may claim to be entitled to the benefit of the action, shall also within four days named in the appointment for taking the accounts, or within such further time as the master or referee may allow, file in the office of the master or referee a Other lien holders to statement of account showing the just and true sum due to them respectively, after giving credit for all sums in cash, merchandise, or otherwise, to which the debtor is entitled to credit on account of their respective claims, which accounts shall be verified by affidavit, and such accounts and affidavit may be in the form mentioned in the schedule hereto.

file accounts.

12. A lien-holder who has not filed his claim within the time limited by the next preceding section may apply to the master or referee to be let in to prove his claim at any time before the his claim may apply amount realized by the proceedings for the satisfaction of liens to be let in. has been distributed, and such application may be granted or refused, and upon such terms as to costs and otherwise as may appear just.

13. Upon the return of the appointment to take accounts, the master or referee shall proceed to take an account of what is due from the owner, and also that is due to the respective lienholders and incumbrancers who have filed their claims, and shall also tax to them respectively such costs as he may find them entitled to, and shall settle their priorities and shall make all other inquiries and take all other necessary accounts for the adjustment of the rights of the various parties, including therein counts, etc., and rewhere there is a prior mortgage or charge, and the holder port. thereof is a party to the proceedings, the amount by which it shall appear to the master or referee that the selling value of land has been increased by reason of the work or materials for which a lien is claimed on the land, and shall thereupon make a report of the result of such inquiries and accounts, and shall direct that the money found due by the owner shall be paid into court, to the credit of the action at the expiration of one month from the date of the report.

14. In case any dispute arises as to the amount due from the owner for the satisfaction of the mechanics liens, or as to the amount claimed to be due, to any lien-holder or incumbrancer, cretion of master. the costs occasioned by the dispute shall be in the discretion of the master or referce and shall be borne and paid as he directs.

15. It nothing is found due by the owner, the master or referee may make an order, staying all further proceedings, and make such order as to costs as shall be just, and at the expiration of Procedure where fourteen days thereafter may grant a certificate, vacating the lien nothing found to be ot the plaintiff and all other liens of the same class as the plain- due from owner. tiff's, unless the issue of the certificate shall in the meantime be stayed, and if such stay is granted, the certificate may issue forthwith after the removal of the stay, or so soon thereafter as the fourteen days shall expire.

16. When anything is found due by the owner, he may on or

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17. The master or referee may make such order as to the Owner's costs where owner's costs of obtaining and registering any certificate vacat- lien vacated. ing a lien as may be just.

18. Upon the registration in the proper registry office or land Registration of certitles office of a certificate vacating any lien or liens the same tificate vacating lien. shall be thereupon vacated and discharged.

19. Upon payment into court of the amount which may be found due by owner the same shall (subject to the payment of amount paid in by any costs thereout as may be ordered) be paid out to the parties found entitled by the report of the master or referee.

owner.

20. In default of payment by owner within the time directed by the report, the plaintiff may apply ex parte, to the said master or referee who, upon due proof of the default may issue a judgment for the sale of the land in question for the satisfaction

Judgment for sale of land on default of

of the lien of the plaintiff and other liens of the same class. 21. The judgment for sale may be in the form set forth in the Form of judgment.

schedule. 22. Such judgment for sale shall be entered as other judgments are required to be entered in the proper office for entering judgments in the county in which the judgment is made, and shall have the same force and effect as a judgment made at a trial of an action between the same parties.

Entry of judgment.

23. The sale under said judgment shall be conducted in the manner prescribed by the Consolidated Rules, respecting sales Conduct of sale. had under the order of the court.

24. After the sale the master or referee shall make his report upon the sale, and shall tax the costs of the sale to the party entitled thereto, and shall in the same report apportion the money Master to make rerealized among the parties entitled thereto, and upon the confircosts. mation of the report, the moneys realized may be paid out of court to the parties found entitled thereto by the report, without further order.

25. For the purpose of the proceedings to obtain an order for sale and for carrying out the sale, and the apportionment of the Plaintiff to represent moneys realized thereunder the plaintiff shall be deemed suffi- lien holders in prociently to represent all other lien-holders entitled to the benefit of ceedings for sale. the action unless the court or master or referee otherwise orders.

26. Any lien-holder entitled to the benefit of the action may apply for the carriage of the proceedings, and the master or referee may thereupon make such order, as to costs and other- Carriage of prowise as may be just, and any lien-holder who obtains the car-ceedings. riage of the proceedings shall in respect of all proceedings taken by him be deemed to be the plaintiff in the action.

27. Any person affected by the proceedings may apply to the master or referee to dismiss the same for want of due prosecu-tion and the master or referee may make such order upon the secution. application as to costs or otherwise as may be just.

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28. Where any infants are named for defendants the appointment referred to in section 5 may be served upon the official guardian ad litem for such infants, who shall thereupon become and be the guardian ad litem for such infant in the proceedings; Official guardian to and it shall not be necessary to serve any such infant defendant with any further or other proceedings and such infant shall be bound thereby.

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29. Where the taxed cost of proceedings to enforce any mechanic's lien which are payable out of the amount realized by such proceedings for the satisfaction of the lien shall exceed 25 per cent. of the amount realized thereby for the satisfaction of costs not to exceed 25 per cent. of the the lien such costs shall be reduced proportionately by the mas- amount realized. ter or referee so as the same shall not in the aggregate exceed the same 25 per cent. and no more costs than such reduced amount shall be recoverable between the party and party, or solicitor and client.

30. After the amount of lien shall be realized any lien-holder who has proved a claim may apply to the said master or referee upon notice to his primary debtor for judgment for the payment of any balance which may remain due after deducting the amount Judgment for balreceived or payable in respect of the lien, and thereupon the ance of the realizing amount of lien. master or referee may refuse the application upon such terms as to costs or otherwise as may be just or in case he sees fit to grant the application he shall grant a certificate of the amount, for which he finds the applicant is entitled to judgment for debt and costs.

31. Such certificate may be filed in the proper office of the High Court for the entry of judgments if the amount awarded exceeds the sum of \$400 and the same shall thereupon be entered When judgment to in the judgment book, and shall thereupon become a judgment of the High Court and may be enforced in like manner as any other judgment for the payment of money is enforced in the High Court.

be entered in High

32. Where the amount awarded by the certificate is less than \$400 but exceeds \$100 such certificate may in like manner be And when in County entered in the County Court of the County in which the said Court. proceedings are carried on and may be enforced in like manner as a judgment of such court.

33. Where the amount awarded does not exceed \$100 the certificate may be entered with the clerk of the Division Court of the division in which the debtor resides in like manner, as a Court, judgment of such court is entered and thereupon the same shall become and be, a judgment of such court and may be enforced in like manner as any other judgment of such Division Court.

And when in Division

34. The fees payable for entering such certificate as a judgment shall be as follows:

Fees for entering certificates

In the High Court, \$1.60 In the County Court .80 In the Division Court .50.

35. Orders and certificates made by a referee or master under the Act shall be appealable in like manner as orders made in Appeals. Chambers by a local judge.

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36. This Act shall not in any way affect, alter, or diminish the jurisdiction or proceedure of the County Courts or Division Act not to affect Courts for enforcing mechanics' liens in a summary manner, as summary enforce-provided by the 28th section of the Mechanics' Lien Act save in ment of mechanics' liens. so far as sub-section 1 of section 30 of the said Act is hereby amended.

37. Where an action to enforce a mechanic's lien is brought and prosecuted in the High Court of Justice, otherwise than in and prosecuted in the High Court of Justice, otherwise than in Costs where action the manner prescribed by this Act, no more costs shall be taxed improperly brought or allowed to the plaintiff than would be incurred by proceeding in High Court. in the manner prescribed by this Act unless the court or judge otherwise orders.

38. The proceedings under this Act shall be dee...ed to be an Act to be deemed an "action."

Proceeding under action.

39. Sub-section I of section 30 of the Mechanics' Lien Act is Rev. Stat. c. 129, s. amended so as to read as follows:

30 (1) amended.

-30.—(1). Any number of lien-holders may join in one action or summary proceeding, and any action or summary proceeding brought by a lien-holder shall be taken to be brought on behalf of all the lienholders of the same class who shall have registered their liens before or within fourteen days after the commencement of the action, or who shall within the said fourteen days or Action by one of within such further time as may be allowed for that purpose file several lien holders to be for joint benefit. in the proper office of the court where the pleadings are required to be filed (where the action has been commenced by writ) or in the office where the proceedings are being carried on (where they are being prosecuted in a summary manner) a statement entitled in, or referring to the said action of their respective claims.

to be for joint benefit.

40. This Act shall be read as part of the Mechanics' Lien Act incorporated with Rev. Stat. c. Act subject to the provisions of this Act.

NOTE.—The various forms of affidavits and notices, etc., are to be found in Chapter 37, p. 73, Statutes of Ontario, 53rd Victoria, 1890.

#### An Act to Consolidate the Acts Respecting Compensation to Workmen in Certain Cases.

[55 V., c. 30.]

[Assented to 14th April, 1892.]

ER Majesty, by and with the advice and consent of the Legislative Assembly of the Province of Ontario, enacts as follows:-

1. This Act may be known and cited as "The Workman's Short title. Compensation for Injuries Act, 1892."

2. Where the following words occur in this Act they shall be Interpretation. construed in the manner hereinafter mentioned, unless a con-

trary intention appears:

(a) "Superintendence" shall be construed as meaning such "Superintendence," general superintendence over workmen as is exercised by a meaning of. foreman, or person in a like position to a foreman, whether the person exercising the superintendence is or is not ordinarily engaged in manual labour. 52 V., c. 23, s. 2 (3).

(b) "Employer" shall include a body of persons corporate or "Employer." unincorporate, also the legal personal representatives of a deceased employer and the person liable to pay compensation under section 4 of this Act. R. S. O. 1887, c. 141, s. 2 (2), 52 V., c. 23,

s. 2 (4).

(c) "Workman" does not include a domestic or menial ser- "Workman." vant, but, save as aforesaid, means any railway servant and any person who being a laborer, scrvant in husbandry, journeyman, artificer, handicraftsman, miner, or otherwise engaged in manual labour, whether under the age of twenty-one years, or above that age, has entered into or works under a contract, with an employer, whether the contract be made before or after the passing of this Act, be express or implied, oral or in writing and be a contract of service or a contract personally to execute any work or labour.

(d) "Packing" shall mean a packing of wood or metal or some equally substantial or solid material, of not less than two inches in thickness, and which, when filled in, shall extend to within one and a half inches of the crown of the rails in use on any railway, shall be neatly fitted so as to come against the web of such rails and shall be well and solidly fastened to the ties on which such rails are laid. R. S. O. 1887, c. 41, s. 2 (3-4).

(e) "Railway servant" shall mean and include a railway ser- "Railway Servant." vant, tramway servant and street railway servant. R. S. O. 1887,

c. 141, s. 2, ; 52 V. c. 23, s. 2.

3. Where personal injury is caused to a workman:

(a) By reason of any defect in the condition or arrangement employer. of the ways, works, machinery, plant, buildings or premises, connected with, intended for, or used in the business of the employer: or

(b) By reason of the negligence of any person in the service of the employer, who has any superintendence entrusted to him

whilst in the exercise of such superintendence: or

(c) By reason of the negligence of any person, in the service of the employer, to whose orders or directions the workman at

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Where workman to have claim against the time of the injury was bound to conform, and did conform, where such injury resulted from his having so conformed: or

(d) By reason of the act or omission of any person in the service of the employer, done or made in obedience to the by-laws of the employer or in obedience to particular instructions given by the employer, or by any person delegated with the authority of the employer in that behalf: or

(e) By reason of the negligence of any person in the service of the employer who has the charge or control of any points, signal, locomotive, engine, machine, or train upon a railway,

tramway, or street railway.

The workman, or in case the injury results in death, the legal personal representatives of the workman and any persons entitled in case of death shall have the same right to compensation and remedies against the employer as if the workman had not been a workman of nor in the service of the employer, nor engaged in his work. R. S. O. 1887, c. 141, s. 3; 52 V. c. 23, ss. 3, 4, 5.

4. (1) Where the execution of any work is being carried into Employer, who to be effect under any contract, and

deemed.

(a) The person for whom the work or any part thereof, is done, owns, or supplies any ways, works, machinery, plant, buildings, or premises used for the purpose of executing the work,

(b) By reason of any defect in the condition or arrangements of such ways, works, machinery, plant, buildings or premises, personal injury is caused to any workman, employed by the contractor or by any sub-contractor: or

(c) The defect, or failure to discover the defect or remedy the defect arose from the negligence of the person for whom the work or any part thereof is being done, or of some person being in his service and entrusted by him with the duty, of seeing, that

such condition or management is proper:

The person for whom the work or that part of the work is done shall be liable to pay compensation for the injury as if the workman had been employed by him, and for that purpose shall be deemed to be the employer of the workman, within the meaning Provided, always, that any such contractor or subcontractor shall be liable to pay compensation for the injury as if this section had not been enacted, so however that double compensation shall not be recoverable for the same injury.

(2) Nothing in this section contained shall affect any rights or liabilities of the person for whom the work is done and the contractor and sub-contractor (if any) as between themselves. 52 V. c. 23, s. 6.

5. Where within this province personal injury is caused to a

workman employed on or about any railway:

(a) By reason of the lower beams or members of the superstructure of any highway or other overhead bridge, or any other erection or structure over said railway not being of sufficient height from the surface of the rails to admit of an open and clear headway of at least seven feet between the top of the

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orkman to n against highest freight cars then running on such railway and the bottom of such lower beams or members: or

(b) By reason of the space between the rails in any railway frog, extending from the point of such frog backward to where the heads of such rails are not less than five inches apart, not

being filled in with packing: or

(c) By reason of the space between any wing rail and any railway frog and between any guard rail and any other rail fixed and used alongside thereof as aforesaid and between all wing rails where no other rail intervenes (save only when the space between the heads of any such wing rail and railway frog as aforesaid, or between the heads of any such guard rail and any other rail fixed and used alongside thereof as aforesaid, or between the heads of any such wing rails where no other rail intervenes as aforesaid is either less than one and three-quarters of an inch or more than five inches in width) not being at all times during the month of April, May, June, July, August, September, and October, filled in with packing.

Such injury shall be deemed and taken to have been caused by reason of a defect within the meaning of sub-section I of section 3 of this Act. But nothing in this section contained shall be taken or construed, as in any respect or for any purpose restricting the meaning of said sub-section. R. S. O. 1887, c. 141,

S. 4.

6. A workman shall not be entitled under this Act to any right Exceptions to preof compensation or remedy against the employer in any of the ceding provisions.

following cases, that is to say

(a) Under sub-section 1 of section 3, unless the detect therein mentioned arose from or had not been discovered or remedied owing to the negligence of the employer or of some person entrusted by him with the duty, of seeing that the condition or arrangement of ways, works, machinery, plant, buildings or premises are proper. R. S. O. 1887, c. 141, s. 5, (1); 52 V. c. 23 s. 8.

(b) Under sub-section 4 of section 3, unless the injury resulted from it propriety or defect in the rules, by-laws, or instructions thereis, sentioned; provided, that where a rule or by-law has been approved or has been accepted as a proper rule or bylaw either by the Lieutenant Governor in Council or under and pursuant to any provision in that behalf of any Act, of the Legislature of Ontario or of the Parliament of Canada it shall not be deemed for the purposes of this Act to be an improper or defec-

tive rule or by-law. R. S. O. 1887, c. 141, s. 5, (2).

(c) In any case where the workman knew of the defect or negligence which caused his injury or failed without reasonable excuse to give or cause to be given, within a reasonable time information to the employer or some person superior to himself in the service of the employer, unless he was aware that the cinployer, or such superior already knew of the said defect or negligence. Provided however that such workman shall not by reason only, of his continuing in the employment of the employer with knowledge of the defect, negligence, act, or omission, which caused his injury, be deemed to have voluntarily incurred the

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risk of the injury. R. S. O. 1887, c. 141, s. 5, (3) 52 V. c. 23, s. s.

7. The amount of compensation recoverable under this Act, Limit of amount of shall not exceed either such sum as may be found to be equiva-compensation. lent to the estimated earnings during the three years preceeding the injury of a person in the same grade employed during those years in the like employment, within this Province, or the sum of fifteen hundred dollars, whichever is larger: and such compensation shall not be subject to any deduction, or abatement, by reason, or on account, or in respect of any matter or thing whatsoever, save such as is specially provided for in section 12 of this Act. R. S. O. 1887, c. 141, s. 6; 52 V. c. 23, s. 10.

8. When in any action under this Act compensation is awarded Distribution of comin the case of the death of a workman, for an injury sustained by pensation. him in the course of his employment, the amount recovered after deducting the costs, not recovered from the defendant may, if the Court or Judge before whom the action is tried so directs, be divided between the wife, husband, parent, and child of the deceased in such shares as the Court or Judge with or without assessors, as the case may be, or, if the action is tried by a jury, as the jury may determine. 52 V. c. 23, s. 14.

9. An action for the recovery under this Act of Compensation Limit of time for refor an injury shall not be maintainable against the employer of covery of compenthe workman, unless notice that injury has been sustained is given within twelve weeks and the action is commenced within six months from the occurrence of the accident causing the injury, or in case of death, within twelve months from the time of death; provided always that in case of death the want of such notice shall be no bar to the maintainance of such action if the Judge shall be of opinion that there was reasonable excuse for such want of notice. R. S. O. 1887, c. 141, s. 7; 52 V. c. 23,

10. No contract or agreement made or entered into by a workman shall be a bar or constitute any defence to an action for the tute a defence to recovery under this Act, of compensation for any injury.

Contract by workman when to constiaction for compensation.

- (a) Unless for such workman entering into or making such contract or agreement, there was other consideration than that of his being taken into or continued in the employment of the defendant; nor
- (b) Unless such other consideration was in the opinion of the Court or Judge before whom such action is tried ample and adequate; nor
- (c) Unless in the opinion of the Court or Judge, such contract or agreement, in view of such other consideration was not on the part of the workman, improvident, but was just and reasonable; And the burthen of proof in respect of such other consideration and of same being ample and adequate, as aforesaid, and that the centract was just and reasonable, and was not improvident as aforesaid, shall, in all cases rest upon the defendant; provided always that notwithstanding anything in this section contained, no contract or agreement whatsoever made, or entered into by a workman shall be a bar or constitute any defence to an action

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for recovery under this Act, of compensation for any injury happening, or caused by reason of any of the matters mentioned in section 5 of this Act. R. S. O. 1887, c. 141, s. 8.

11. Notwithstanding anything contained in this Act, an action Liability or personal under sections 3, 4 or 5 shall lie against the legal representatives representative.

of the deceased employer. 52 V. c. 23, s. 15.

12. There shall be deducted from any compensation awarded Money payable unto any workman, or representatives of any workman, or persons der penalty to be deducted from comclaiming by, under or through a workman in respect of any cause pensation. of action arising under this Act, any penalty or damages or part of a penalty or damages, which may in pursuance of any other Act, either of the Parliament of Canada, or the Legislature of Ontario, have been paid to such workman, representatives or persons in respect of the same cause of action; and where an action has been brought under this Act by any workman or the representatives of any workman or any persons claiming by, under or through, such workman, for compensation in respect of any cause of action arising under this Act, and payment has not previously been made of any penalty or damages or part of a penalty or damages, under any such Act either of the said Parliament, or of the said Legislature, in respect of the same cause of action such workman, representatives or persons shall not so far as the said Legislature has power to enact, be entitled thereafter to receive in respect of the same cause of action any such penalty or damages or part of a penalty or damages under any such last mentioned Act. R. S. O. 1887, c. 141, s. o.

13. (a). Notice in respect of an injury under this Act shall Form and service of give the name and address of the person injured and shall state in ordinary language the cause of the injury, and the date at which it was sustained and shall be served on the employer, or if there is more than one employer, upon one of such employers.

(b) The notice may be served by delivering the same to or at the residence or place of business of the person on whom it is

(c) The notice may be served by post, by a registered letter addressed to the person on whom it is to be served at his last known place of residence or place of business and if served by post shall be deemed to have been served at the time when a letter containing the same would be delivered in the ordinary course of post, and in proving the service of such notice it shall be sufficient to prove that such notice was properly addressed and registered.

(d) Where the employer is a body of persons corporate or unincorporate the notice shall be served by delivering the same at or by sending it by post, in a registered letter addressed to the office or if there be more than one office, any one of the

offices of such body. R. S. O. 1887, c. 141, s. 10. (1-4).

(e) The want of sufficiency of the notice required by this section or by section 9 of this Act shall not be a bar to the maintainance of an action for the recovery of compensation for the injury if the Court or Judge before whom such action is tried or in case of appeal, if the Court hearing the appeal is of opinion

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that there was reasonable excuse for the want or insufficiency and that the defendant has not been thereby prejudiced in his defence. 52 V. c. 23, s. 12.

(t) A notice under this section shall be deemed sufficient if in the form or to the effect following:—

To A. B. of (here insert employer's address) or To the-

-Company (or as the case may be).

Take notice that on the of (insert address of injured person) a workman in your employ sustained personal injury (add, of which he died, if such be the case), and that such injury was caused by (state shortly the cause of injury e. g. the fall of a beam).

Date.

Yours, etc.,

X. Y.

R. S. O. 1887, c. 141, s. 10. (6).

14. If the defendant in any action against an employer for Defence of want of compensation for an injury sustained by a workman in the course notice. of his employment intends to rely for a defence on the want of notice or the insufficiency of notice or on the ground that he was not the employer of the workman injured, he shall, not less than seven days before the hearing of the action or such other time as may be fixed by the rules, regulating the practice of the Court in which the action is brought, give notice to the plaintiff of his intention to rely on that defence, and the Court may in its discretion and upon such terms and conditions as may be just in that behalf, order and allow an adjournment of the case for the purpose of enabling such notice to be given; and subject to any such terms and conditions any notice given pursuant to and in compliance with the order in that behalf shall, as to any such action and for all purposes thereof be held to be a notice given pursuant to and in conformity with sections 9 and 13 of this Act. 52 V. c. 23, s. 13.

15. In an action brought under this Act, the particulars of de-Particulars of demand or statement of claim shall state in ordinary language mand. the cause of the injury and the date at which it was sustained, and the amount of compensation claimed; and where the action is brought by more than one plaintiff the amount of compensation claimed by each plaintiff, and where the injury of which the plaintiff complains shall have arisen by reason of the negligence. act, or omission of any person in the service of the defendant, the particulars shall give the name and description of such person. R. S O. 1887, c. 141, s. 11.

16. (a). Upon the trial for the recovery of compensation Appointment of Asunder this Act before a Judge without a jury, one or more assessors may be appointed by the Court or Judge for the purpose of ascertaining the amount of compensation and the remuneration (if any) to be paid to such assessors shall be fixed and determined by the Judge at the trial.

(b). Any person who shall as hereinafter provided, be appointed to act as an assessor, shall be qualified so to act.

(c) In such action a party who desires assessors to be appointed, shall, ten clear days at least before the day for holding

service of injury,

the Court, at which the action is to be tried, file an application stating the number of assessors he proposes to be appointed, and the names, addresses, and of the persons who may have expressed their willingness in writing to act as assessors. If the applicant has obtained the consent of the other party, to the persons named being appointed, he shall file such consent with his application.

(d) Where the application for the appointment of assessors has been made by one party to the action only, he shall, eight clear days before the day for holding the Court at which the action is to be tried, serve a copy of the application so filed upon the other party who may then either file an application for assessors or file

objections to one or more of the persons proposed.

(e) An application for the appointment of assessors may be in the form following, or to the like effect, namely:—

The plaintiff (or defendant) applies to have an assessor (or assessors) appointed to assist the Court in ascertaining the amount of compensation to be awarded to the plaintiff, should the judgment be in his favour, and he submits the names of the following persons, who have expressed their willingness in writing to act as assessors should they be appointed.

(Here set out the names, addresses and occupations of the persons above referred to). (If the party consents to the appointment add the following:—

The defendant (or plaintiff) consents to the appointment of any of the persons, above named to act as assessors, in this action, as appears by his consent thereto filed herewith.

Dated this ——day of———189—

The above named plaintiff (as the case may be).

(f) Where separate applications are filed by the parties, no objections to the persons proposed shall be made by either party, but the Court or Judge may appoint from the persons named in each application one or more assessor or assessors, provided that the same number of assessors be appointed from the names given in such applications respectively.

(g) In such action brought in a Division Court, the application for the appointment of assessors, together with any objections made to the persons proposed, shall be forwarded by the

Clerk of the Court to the Judge.

(h) Where application for the appointment of assessors is granted the Court or Judge shall appoint such of the persons proposed for assessors as by the Court or Judge may be deemed fit, subject to the provisions contained in this Act.

(i) In such action where an application for the appointment of assessors has been filed the Court or Judge may, at any time prior to the trial thereof nominate one or more additional persons to act as assessors in the action. Where no application for

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assessors has been made, the Court or Judge may appoint one or more persons to act as assessor or assessors in the action

before, or on the trial of the action.

(j) If at the time and place appointed for the trial, all or any of the assessors appointed shall not attend, the Court or Judge may either proceed to try the action with the assistance of such of the assessors, if any, as shall attend or may adjourn the trial generally, or upon any terms which the Court or Judge may think fit, or may appoint any person who may be available and who is willing to act, and who is not objected to or who, if objected to, is objected to on some insufficient ground or the Court or Judge may try the action without assessors.

(k) Every person requiring the Court or Judge to be assisted by assessors shall at the time of filing his application, deposit therewith the sum of \$4 for every assessor proposed, and such payments shall be considered as costs in the action, unless otherwise ordered by the Court or Judge: Provided that where a person proposed as an assessor, shall have in writing agreed and consented that he will not require his remuneration to be deposited, no deposit in respect of such person shall be required.

(l) Where an action shall be tried by a Court or Judge with the assistance of assessors in addition to or independently of any assessors proposed by the parties, the remuneration of such assessors shall be borne by the parties, or either of them as the

Judge or Court shall direct.

(m) If after an assessor has been appointed the action shall not be tried, the Court or Judge shall have power to make an allowance to him in respect of any expense or trouble that he may have incurred by reason of his appointment, and direct the payment to be made out of any sum deposited for his remuneration.

(n) The assessors shall sit with and assist the Court or Judge when required with their opinion and special knowledge for the purpose of ascertaining the amount of compensation if any, which the plaintiff shall be entitled to recover. R. S. O. 1887, C. 141, S. 12.

17. (a) Where several actions shall be brought under this Act, Consolidation of ecagainst a defendant, in the same court in respect of the same tions. negligence, act, or omission, the defendant shall be at liberty to apply to the Judge that the said actions shall be consolidated.

(b) Applications for consolidation of actions shall be made upon notice to the plaintiffs affected by such consolidation.

(c) In case several actions shall be brought under this Act, against a defendant in the same court in respect of the same negligence, act, or omission, the defendant may upon filing an undertaking to be bound so far as his liability for such negligence, act or omission by the decision in such one of the said actions as may be selected by the Court or Judge, apply to the Court or Judge for an order to stay the proceedings in the actions other than in the one so selected, until judgment is given in such selected action.

(d) Applications for stay of proceedings shall be made upon notice to the plaintiffs affected by the stay of proceedings or

ex parte.

(e) Upon the hearing of an application for consolidation of actions or for stay of proceedings, the Court or Judge shall have power to impose such terms and conditions and make such order

in the matter as may be just.

(f) If an order shall be made by a Court or Judge, upon an ex parte application to stay proceedings, it shall be competent to the plaintiffs affected by the order to apply to the Court, or Judge (as the case may be) upon notice or ex parte to vary or discharge the order so made, and upon such last mentioned application such order shall be made as the Court or Judge shall think fit, and the Court or Judge shall have power to dispose of the costs, occasioned by such order as may be deemed right.

(g) In case a verdict in the selected action shall be given, against the defendant, the plaintiffs in the actions stayed, shall be at liberty to proceed for the purpose of ascertaining and recov-

ering their damages and costs.

(h) A defendant may by notice to the opposite party to be given or served at least six days before the day appointed for the trial of the action, admit the truth of any statement of his liability for any alleged negligence, act, or omission as set forth or contained in the plaintiff's statement or particulars of claim in the action, and after such notice given the plaintiff shall not be allowed any expense thereafter incurred for the purpose of prov-

ing the matter so admitted.

(i) Where two or more persons are joined as plaintiffs under sub-section I of this section and the negligence, act, or omission, w. in is the cause of action shall be proved, the judgment shall be for all the plaintiffs but the amount of compensation, if any, that each plaintiff is entitled to, shall be separately found and set forth in the judgment and the amount of costs awarded in the action shall be ordered to be paid to such person, and in such manner as the Court or Judge may think fit; should the defendant fail to pay the several amounts of compensation and the costs awarded in the action execution may issue as in an ordinary action and should the proceeds of the execution be insufficient after deducting all costs, to pay the whole of the amounts awarded a dividend shall be paid to each plaintiff calculated upon the proportion of the amount, which shall have been awarded to the respective plaintiffs to the total amount realized after the deduction of all costs of the action as aforesaid. R. S. O. c. 141, S. 13.

18. Where the time for doing any act, taking any proceeding, Computation of time. or giving any notice under or required by this Act, expires on a Sunday, such act, or proceeding or notice shall so far as regards the time of doing, taking or giving the same, be held to be duly and sufficiently done taken or given, on the day next following such Sunday. ス. S. O. 1887, c. 141, s. 14.

19. In an action brought in any Court to recover compensa- Forms and rules, tion under this Act, the forms and methods and rules, and orders in force in Court shall, subject to and save as otherwise provided by the terms and provisions of this Act, apply to and regulate all matters of pleading, practice and procedure in such action, and notwithstanding anything in this Act contained, the forms

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and method, pleadings, practice and procedure in any such action shair conform to, and be regulated by any rules or orders in that behalf hereafter lawfully and duly made or prescribed with respect to actions brought in any such Court. R. S. O. 1887, c. 141, S. 15.

20. All Acts and parts of Acts consistent with this Act, are Saving clause. hereby repealed: but such repeal shall not affect, nor shall any provision of this Act prejudice anything heretofore done or suffered, or any right heretofore acquired or accrued under or in pursuance of said Acts or parts of Acts so repealed and any proceeding in respect of any such right, and any action, suit, or proceeding under or in pursuance of said last mentioned Acts or parts of Acts, shall be instituted, continued, completed and determined and dealt with in all respects, and for all purposes as if this Act had not been passed. R. S. O. 1887, c. 141, s. 16; 52 V. c. 23, s. 16.



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# No. 3075. A BY-LAW RESPECTING THE CONSTRUCTION OF BUILDING SCAFFOLDINGS.

[Passed June 6th, 1892.]

The Municipal Council of the Corporation of the City of Toronto enacts as follows:

I. All scaffoldings used by bricklayers or other builders in the erection, repairing, altering or improving of buildings, chimneys or other structures, shall be built and constructed as follows:

#### BUILDERS' SCAFFOLD.

Standards or uprights to be of live, sound Norway pine, tamarac or spruce (tamarac preferred). Distance between each standard eight or ten feet, and butts of said standard placed in the ground to the depth of not less than two feet six inches, and when placed upon stone flagging or granolithic sidewalk, to be put in a good sound cement or other barrel or a box two feet square by two feet six inches high, and filled with sand or other suitable material. The standards to be not less than four and a half inches at butt, and two and a half inches at the top diameter, and in a very high scaffold, to be increased in size.

Ledgers same material as standards, not less than three inches diameter at small end, and no ledger to be taken off the standards that would allow a greater distance from the ground than ten feet. Putlogs to be of ironwood, white oak, or other suitable material, the said putlogs to be butted, flattened or squared at the end which enters the wall, and not to be removed according as the scaffold rises. One course of planking, the entire length of scaffold, must remain on each tier of the said putlogs. The putlogs not to be less than three and a half inches in diameter clear of bark. Three putlogs to be placed under planks twelve feet in length, that is to say, one putlog at each end, and one in centre. (When planks sixteen feet long are used five putlogs shall be used.) Planks to be two inches in thickness, and of sound pine, spruce or hemlock, ten or twelve inches in width.

Scaffolds to be stayed from ledgers on to the joists through the openings, and in the absence of openings, to be stayed by other sufficient means.

Racking braces to consist of poles, and tied with ropes. Ropes not to be less than sixteen feet in length, and five-eighths of an inch thick, except in case of small scaffolds, when rope one-half inch thick may be used.

Ladders in all cases to reach five feet above the landing stage, so that plenty

of hold will be afforded men when landing off.

When bricks are laid from the inside of fire-proof buildings, there shall be a temporary floor of two-inch plank laid on the girders or temporary joists all around the inside of walls and not less than six feet wide, and when bricks are laid from the inside of buildings not fire-proof, which have joists not over four-teen inches apart, then the temporary floor may be of one-inch boards six feet wide and placed all around the building.

In all cases where the inside scaffolding is built from the foundation the same as the outside scaffolding, the temporary floors above mentioned shall not Le

required.

When trestles are used the height to be from four to six feet and to be made substantial, of good material; and when a scaffold is formed by putting trestles

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one upon another, it shall not be over eighteen feet in height, that is to say, not more than three trestles shall be used of the height of six feet each.

Where required all overhead protections to be placed fully under scaffolds. When building out to the street line, boards or planks to be placed where the workmen pass under.

II. All scaffolding used by carpenters, in the erection, repairing, altering or improving of buildings, chimneys or other structures, shall be built and constructed as follows:

#### CARPENTERS' SCAFFOLDING.

1. All uprights of said scaffolding to be  $4 \times 4$ , sound and free from objectionable knots, the brackets nailed to them and to the building, and to be one inch in thickness, and not less than ten inches wide, properly nailed to building and upright; and when there is no opening to nail said bracket, then a piece one inch thick and six inches wide to be notched to secure the bracket, and nailed solid to the wall and to the upright. The boards laid on this to walk on to be two-inch plank, sound and free from knots, or else two one-inch boards laid on on top of the other.

2. When bracket scaffold is put up, the leg to be sound and not less than 2×6 on edge, set at the proper angle, to prevent the bracket from tipping from the wall.

3. When scaffolding projects from windows, the scaffold to be one inch thick by not less than ten inches wide, and braced on the angle with a board not less than one inch thick and six inches deep, both brace and bracket well nailed to window, and 'he brace well nailed to bracket also.

III. The City Commissioner shall prosecute all persons who may proceed with the erection of buildings using scaffolding which is not constructed in accordance with this By-law, and in the event of the City Commissioner finding a scaffold which in his estimation is unsafe, and after due notice to the contractor the same is not made satisfactory, the said Commissioner may take such proceedings against him as he has power under this or any other By-law.

IV. Any person convicted of a breach of any of the provisions of this By-law shall forfeit and pay at the discretion of the convicting magistrate a penalty not exceeding the sum of fifty dollars for each offence, exclusive of costs, and in default of payment of the said penalty and costs forthwith, the said penalty and costs, or costs only, may be levied by distress and sale of the goods and chattels of the offender, and in case of there being no distress found out of which such penalty can be levied, the convicting magistrate may commit the offender to the Common Gaol of the City of Toronto, with or without hard labor, for any period not exceeding six calendar months, unless the said penalty and costs be sooner paid.

# No. 3082. A BY-LAW TO AMEND BY-LAW No. 3075 RESPECTING THE CONSTRUCTION OF BUILDING SCAFFOLDING.

[Passed July 7th, 1892.]

The Municipal Council of the Corporation of the City of Toronto enacts as follows:

I. Section 3 of By-law No. 3075 is hereby amended by striking out all the words in the section after the word "By-law" in the third line thereof.

# MONTREAL BY-LAW CONCERNING SCAFFOLDS. BY-LAW No. 107, Sec. 51.

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All scaffolds erected for use in the erection or repair of any building shall be well and safely supported and of sufficient strength and size and properly secured so as to insure the safety of persons working thereon or passing under the same, against the falling thereof or of such materials as may be used or deposited thereon; any scaffold which may be otherwise erected shall be deemed a nuisance; and any person who shall erect or use, or cause to be erected or used, any scaffold contrary to the provisions hereof shall be liable to the penalty provided in section 103 of this By-law.

Section 103 provides that for each offence against any by-law the offender "shall be liable to a fine, and in default of immediate payment of said fine and costs to an imprisonment, the amount of said fine and the term of said imprisonment to be fixed by the Recorder's Court at its discretion."



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#### GLOSSARY OF TERMS USED IN BUILDING.

Aisle (pronounced ile)—a passageway between seats; the space between the outside walls and the columns in a cathedral church.

*Alcove*—a recess in the side of a room.

Alto relievo—carving projecting considerably above the surface; also "high relief,"

Ambo-same as pulpit.

Anchor—a term applied to ties and fastenings.

Andirons (or fire dogs)—irons for the support of fuel in open fire places.

Angle bead—a small round moulding secured to outside angles of walls.

Angle brace—a bracket or framing in the angle of two surfaces.

Angle staff—see angle bead.

Annex—a building subordinate to a main building.

Antique—referring to ancient style.

Apse—semi circular end of a room or chancel.

Arabesque—ornamentation of flat surfaces.

Arc—a geometrical term used for a portion of a circle.

Arcade—(1) a series of arches and columns.

(2) a corridor.

Arch—a construction of bricks or other materials so arranged as by mutual pressure to support each other and to become capable of sustaining a superincumbent weight.

Archivolt-same as soffit.

Area—a space, a court yard or sunken court.

Arris—the line or edge formed by the meeting of two plain surfaces.

Ashlar—cut or hewn stone used in the face of a wall, generally with vertical and horizontal joints.

Astragal—semi circular moulding or bead.

Attic—a sub-story rising above the cornice of a building.

Auditorium—a hall for assemblies.

Back arch-a relieving arch.

Ball cock—a water cock in the form of a ball, placed inside a cistern to regulate the inflow of water.

Balderchino—a canopy supported on columns over an altar, tomb or throne.

Ball flower—a gothic ornament like a ball enclosed in three leaves.

Balloon frame—Rough framing of a wooden building.

Baluster—perpendicular standard supporting a rail, also banister.

Balustrade—a range of balusters with upper and lower rail.

Barge Board—board used as a finish on the face of a gable, plain or ornamental.

Barrel drain—a brick conduit built in cylindrical form.

Barrel vault—a long semi circular vault or roof. Base—lower part of a wall or pillar.

Bas relief or basso relievo—carvings raised but little above the surface; also "low relief."

Bat-a broken brick.

Batten-a thin strip of wood.

Batter—the sloping face of a wall built wider at the bottom than at the top.

Battlement—indentations in the top of a parapet wall.

Bay—a division in the architectural arrangement of a wall.

Bay window—a window forming a recess in a room. Bead—a small circular moulding.

Bead and butt—a panel moulded with a bead, abutting against a square surface; also "bead and flush."

Bead and quirk—a bead sunk below the surface with the angles of the surface cut off.

Beamfilling—building round the ends of beams or joists in a wall.

Bearer—a strut or post or horizontal piece supporting a shelf or other boards.

Bearing wall or partition—portion of a wall or partition that carries a superstructure.

Bed moulding—strictly speaking a moulding immediately under the cornice of a classic building.

Belvedere—a high turret. Bevel—a sloping surface.

Billet—a block of wood.
Binder—same as "Header."

Binding joist—same as "Trimmer."

Block plan—a plan showing the outline of a building.

Blocking course—a plain course of stone above a cornice, its face being in the same plane as the face of the walls below.

Bond—in brick or stone work denotes arrangement of headers and stretchers. Bond, English—in brickwork a course of headers and a course of stretchers laid alternately.

Bond, Flemish—in brickwork, headers and stretchers laid alternately in the same course.

Bond stone—same as "Header."

Bond timber—pieces of wood built into a wall on which to secure the other woodwork.

Bow Window—a semi circular bay window.

Box frame—the trame of a window made hollow for the sash weights.

Box girder—a hollow built girder.
Brace—a stiffening piece in framing.

Breaking joint—a term applied to the joints of masonry or other work which are not continuous.

Breast of a chimney—the projecting portion of a chimney stack in a room.

Breast of a window—that portion of a wall below the window.

Bressummer—a heavy beam.

Brick nogging—brickwork laid in the interstices of traming.

Bridging—pieces of wood secured between joists for the stiffening of floors. Broach—the masonry at the foot of an octagonal spire above the square tower. Bull's nose—a rounded angle.

Bush hammered—the worked face of a stone formed by a bush hammer. This hammer is formed of several metal points with which the stone is pounded. Butting joint—the junction of two pieces cut at right angles with the surface.

Cable moulding—a moulding like a rope.

Cant—the surface left when the angle of a square is cut off.

Cantilever—a bracket.

Cap or capital—the highest member of a column.

Carriage of a stair—the rough timbers supporting a staircase; also "stringers." Case—see box frame. Casing—boxing in of pipes, etc.

Casement—(1) a sash hinged like a door.

(2) also a classic moulding (see "Scotia").

Cauking or caulking—filling of joints.

Cavetto-a hollow moulding.

Centre—rough framework for the support of an arch in construction.

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Centre of gravity—that point at which all the weight of a mass might be collected without disturbing the equilibrium of the body.

Chace mortise—a mortice cut so as to admit of a transverse piece of timber being

*Chace—-*a groove.

let in between two parallel pieces. Chamfer—like a bevel, the cutting off of a right angle to an angle of 45°.

Chancel—the choir of a church containing an altar.

e cornice of a Cheeks of a mortice—the two solid pieces on each side of a mortice.

*Chevron*—a zig-zag moulding of the Norman style. Chiseled work—Stone work finished with a chisel.

*Choir*—part of the chancel of a church. *Chord*—a line joining the extremities of an arch.

Cima recta— Greek undulating moulding. Cima reversa—like a cima recta reversed.

Cinque foil—a Gothic ornament of five semi circular curves with four cusps or points at the junctions of the curves.

Circumference—the outside line of a circular body.

Clamp—a piece of metal or other material by which two stones or other substances are fastened together.

Clapboard—thin boarding covering framework of a building.

Clearstorey—the upper portion of a church wall above the aisle roof.

Cleat—a small piece of wood secured to timbers for the support of other woodwork.

Cloister—a covered and paved walk in connection with a cathedral.

Closer—a portion of a brick less than the width of a header inserted as a header near the end of a course.

Cofferdam—timber framing employed in bridge building, sunk into the water. Coin or quoin—blocks of stone finishing the angles of a building.

Collar beam—a horizontal beam in roof construction secured to the rafters midway between the eaves and ridge.

Column—a perpendicular pillar.

Common rafter—ordinary rafters of a roof.

Concave—a hollov ad surface.

Convex—an outward curved surface.

Cope stone—see coping. *Coping*—cap stones of a wall.

Corbie steps—steps up a gable. *Corbel tab'e*—a row of corbels or ornamental brackets.

Counter sinking—any cutting below the general surface.

Crowning—forcing up the centre of a joist by drawing in the ends with a screw rod.

Cusps—the points of intersection of curves in Gothic architecture.

**Dado**—lower portion of a wall faced with panelling or other decoration.

Dais—a raised portion of a floor or low platform.

Deafening—any method of construction for the purpose of deadening sound.

Deflection—a term applied to the bending of a beam from a straight line under pressure.

*Dentil*—an ornament of square blocks.

Diaper work—decoration of a flat surface, generally carved or sunk, sometimes painted.

Discharging arch, or relieving arch—an arch formed over a lintel to relieve it of the superincumbent weight.

Dog legged stairs—a stair case divided into two flights, the outer string of the upper flight being directly over the handrail of the lower.

Dog tooth moulding—a moulding peculiar to Early English architecture.

Door jamb—the lining on the ingoing of a door.

Dormer—a window in a sloping roof.

Dormitory—a large sleeping room.

Dovetail—a wedge shaped joint for woodwork.

Dowel—a wooden or metal pin.

Dragon piece—a short piece of timber used to strengthen the foot of a principal rafter or to tie together the intersecting eaves-plates at the angles of a building.

Dressings—the finished stonework of windows, doors, etc.

Drips—the formation at changes of levels in flat or sloping roofs.

Drip stone or moulding—a projecting moulding to throw off the water over a window or doorway, called also "Labels."

Dry rot—a disease of vegetable growth that attacks timber in damp situations. Droved ashlar, margins, etc.—a tooled margin on stone; also draved.

Eaves—edges of a roof.

Echinus—egg and anchor moulding.

Elliptic arch—arch in the form of an ellipse.

Embrasure—(1) the splay or bevel of a door or window jamb: (2) a cutting in a thick wall.

Engaged column—a column attached to a wall.

English bond—see Bond.

Entasis—the swelling or curved vertical line of a shaft or column.

Entresol—an intermediate storey, called also "Mezzanine."

Equilateral arch—a pointed gothic arch, the height of the apex above the spring being equal to the width of the span at the spring.

Escutcheon—a key hole plate; also "scutcheon."

Extrados—the upper surface of an arch.

Eye of a volute—the central spot of a spiral curve.

Facade—principal or front elevation of a building.

Facing—the outer surface of a wall, etc.

Fanlight—a transom light.

Fan tracery—radiating ornament, the decoration applied to Fan vaulting, a feature of the perpendicular style.

Fenestration—the spacing of windows in a wall.

Fillet—a small band or square member in mouldings.

Finial—the finish or vertical ornament of a turret, roof or spire.

Flags—paving stones

Flashings—metal coverings round the edges of gutters, etc., on roofs or against brickwork.

Flemish bond-see Bond.

Floating-a term in plastering, the finishing work.

Flush—a term indicating the meeting of two surfaces on the same plane.

Flutes or Flutings-semi-circular sinkings on flat surfaces.

Flying buttress—an arched buttress.

Foils—the curves between the cusps in gothic architecture.

Footings—the lowest courses of foundations.

Formeret—the principal rafter against a wall at the end of a building. Free stone—sand stone.

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Freize—(1) in classic architecture that part of an entabalature between the

architrave and comice; (2) the upper portion of a wall in a room, usually so

tring of the

ture.

called for decorative purposes.

Fresco painting—painting on flat s

Fresco painting—painting on flat surfaces of a particular character. Fret—an ornament of classic art.

Furrings—woodwork beneath the plastering, fastened against the walls or on the underside of joists, etc., to receive the lathing.

Gable—the pointed upper part of a wall.

Girt—surface of a moulding.

Grill—open screen work in any material.

Groin—the line of intersection of the vaults in vaulted ceilings.

Grounds—strips of wood forming the projections to which to fasten finished woodwork.

Grout—liquid mortar or cement.

Gurgoyle or Gargoyle—ornamental spout generally of stone in gothic architecture to conduct rain water from the roofs or gutters.

Half space—a platform on a staircase.

Half-timbered work—16th century domestic architecture in which framing timbers appear on the surface forming panels usually filled with brick nogging. Hammer beam—the lower beam projecting horizontally into a building (a hall or

church) acting as a base and tie at the foot of a principal rafter.

Hammer dressed—Method of finishing stone surfaces with a hammer, leaving the face rough and broken.

Hanging stile—the uprights to which doors or casements are hinged.

Hasp—a fastening of metal to secure doors.

Hatchway—an opening in a roof or floor.

Header—a brick or stone acting as a bond between the face and back work of a wall.

Heptagon—a geometrical figure of seven sides.

Herring bone, strutting or bridging—cross pieces of wood secured between joists for stiffening floors. (see Bridging).

Herring bone masonry—masonry in which the stones are laid diagonally.

Hexagon—a six-sided figure.

High relief—(see Alto relievo.)

Hip—the line formed by the angle of two meeting roofs.

Hip rafter—the ratter at the intersection of two roof slopes.

Hip roof—a roof constructed of equally inclined planes rising to the same pitch and height.

Hood-mould—(see Drip.)

Impost—head of a pier.

Inverted arch—an arch constructed with the apex downwards, used chiefly in foundations.

Intrados—soffit of an arch or dome.

Isle, ile—(see Aisle.)

Jack rafter—(same as Dragon piece.)

Jamb-sides of an opening.

Joggle—a notched joint in wood or stone work.

Key stone—the apex stone of an arch.

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King post—the central post in roof framing hanging from the apex to the principal rafters and supporting the tie beam.

Knotting-preparing the knots in woodwork with shellac for painting.

Label-see Dripstone.

Lancet arch—an early English arch of lancet form.

Lattice girder—a metal girder of lattice work.

Lintel—a beam or stone spanning an opening. \*

Low relief-see Bas relief.

Louvre boards—horizontal boards placed sloping outwards, forming unglazed apertures, such as belfries or ventilators in brewery roofs, etc.

Mansard roof—a roof, the incline of which is in two planes, the lower very steep and the upper one of a slight rise only. The lower is often used and finished as an attic storey. It derives its name from its inventor, Francis Mansard, a French architect, born 1598.

Morquetry or Parquetry—inlaid work of different woods.

Metope—In Doric architecture, the square space between the triglyphs.

Mortise—a hollow cavity in woodwork into which is let a tenen of another piece to form a joint.

Mullion—a vertical division between the lights of a window or opening in a screen.

Narthex—the vestibule of a church.

Nave--(1) the body of a church; (2) the centre passage between the seats in a church.

*Needle*—a horizontal timber used as a support.

Nevel—main posts in a railing, principally in a staircase.

Niche—a recess for a statue.

Nogging—see Brick nogging.
Nonagon—a nine-sided figure.

Nosing-projecting moulding of a step or horizontal board.

Obelisk—a quadrangular shaft of stone set on end, the diameter of which is less near the summit than at the base; the summit is truncated.

Octagon—an eight-sided figure. Ogee—a mould of double curve.

Oriel window—a projecting window in an upper floor.

Panel—a raised or sunk portion in a general surface surrounded by mouldings. Pantile—a curved roof tile.

Parallelogram—a four-sided rectangular figure.

Parapet—a low wall above the eaves of a building.

Parquetry—inlaid work of wood.

Party wall—a wall the centre line of which is over the dividing line of two properties or lots of ground. The owner of each lot has certain rights in connection with its use.

Patera-a circular, flat ornament.

Pedestal—a base.

Pediment—a gable or triangular portion above the cornice of a classic building. Pendentive—hanging work of stone, timber or any other material.

Penehammer—in masonry, a tool for pounding the inequalities on a flat surface. Pentagon—a five-sided figure.

Piend check—the repate formed on the bottom of a riser.

Pile—a timber driven into the earth to make a foundation in loose ground. Pinnacle—the finial of a buttless.

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Plate—a piece of timber laid horizontally on a wall to form a bed for the ends of other timbers.

Plinth—the projecting sub-base of a base.

Plugs—small pieces of wood driven into walls to fasten woodwork to the wall. Pointing—the act of filling the face of joints of brick or stone with mortar.

Poleplate—a purlin at the foot of the principals in a roof on which to rest the common rafters.

Principal—the framed main supports of a roof.

Pugging—deafening with clay.

Purlin—a horizontal piece of timber in roof construction for the support of common rafters. It gets its support from the principals.

Putlogs—short timbers in scaffolds at right angles to the walls.

Quarters—scantlings of timbers in partitions or other framing.

Quarter pace—the quarter landing of a staircase.

Quatre foil—a gothic ornament of four semi-circular curves with three points or cusps at the junctions of the curves.

Queen posts—the two suspended posts of a principal roof truss, supporting the tie beam, acting like a king post but used for larger spans.

Quirk - a curved sinking. See Bead and Quirk.

Rabbet or rebate—a rectangular cutting along the edge of a piece of wood or other material.

Rebate joint—a joint formed by a rebate.

Radius—in geometry, a line drawn from the centre to the circumference of a circle.

Rafter—the timbers of a roof laid from the eaves to the ridge.

Raking fiece—a piece of wood on a slope.

Ramp—the sudden rise in a handrail or wall or other substance.

Random work, random rubble-masonry with irregular courses or jointings.

Relieving arch-same as Discharging arch.

Rendering - a plastering coat.

Reredos—a screen or ornamental wall behind and above an altar.

Responds—half piers or buttresses.

Retaining wall—a wall built to support an embankment of earth.

Reveal—the ingoing or recess at right angles to the face of a wall.

Ridge piece—the horizontal board against which the upper end of the rafters abut.

Riser—the vertical part of a step.

Rockfaced work -- stonework left rough on the face.

Roll and fillet—a bead having a square projection on its face.

Rolled beams—iron or steel beams rolled between rollers not cast.

Rood screen—a screen at the junction of a chancel with a nave.

Rose window—a circular window.

Rough cast—rough plastering on the exterior of a wall made by throwing small pebbles against the finished coat of plaster.

Rough string—the rough carriage of a stair.

Saddle—a thin piece of wood bevelled on two edges placed on the floor beneath a door.

Sagging—the bending of a body by its own weight, the ends being supported.

Sash—the glazed part of a window, also sash frame.

Sash lock or fastener—a metal fastening for a window.

Scant'ing-small timbers.

Searf-method of joining timbers lengthwise.

Scotia-a hollow moulding.

Screed—in plastering, a strip of wood the exact thickness of the coat of plaster, nailed for the plasterers to work to.

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Scribing—fitting pieces of wood together.

Segment—part of a circle.

Set off—a break in the face of a wall, etc. Setting coat—the last coat in plastering.

Skewback—the inclined abutment of an arch.

Skirtings—base boards of a room.

Sleepers—timbers laid on the ground to receive the joists of a floor.

Soffit—underside of doorway, staircase, etc. Span—the opening beneath an arch or lintel.

Spandrel—the triangular space over the haunch of an arch.

Specific gravity—the weight of any body as compared with the same magnitude of rainwater, a cubic foot of which weighs 1000 oz.

Springer—the top of an impost.

Stile—the upright portions of a door or window sash.

Stilled arch—an arch that has its centre struck from a point above the impost cap, the curves of the arch being carried down vertically to the impost cap. Strap—a narrow iron band used to hold timbers together.

Stretcher—a stone or brick with its long side to the face of a wall.

String course—a projecting horizontal band of stone plain or moulded.

Struck joint—a finish given to a mortar or cement joint in brick work or masonry.

Tail joist—joist framed between the tail trimmer and the wall, also trimming joist.

Tail trimmer or trimmer—the beam or double joist to carry the hearth laid parallel with the face of chunney breast.

Tangent—a line that touches in part of its length the circumference of a circle. Templet—a pattern.

*Tenon*—a tongue of wood to fit the mortise.

Tension rod—a rod of metal used to counteract a thrust.

Throat—(1) of a chimney, the narrowing above the fireplace; (2) the groove cut under the projecting part of a sill to prevent the water running into the joint.

Tie beam—the horizontal main timber of a roof, etc., tying the ends of the principal rafters.

Torus—half-round moulding.

Tracery—(1) ornamental masonry or woodwork of windows; (2) raised decorations of flat surfaces.

Transept—the "arms" of a church.

Transom—a bar across a window or head of a door with a fanlight over.

Tread—the horizontal portion of a step.

*Trefoil*—a gothic figure of three semi connected circles.

Triforium—in cathedrals the arcade above the nave arches and below the clear story.

Trimmer—see Tail trimmer, etc.

Truss—a framing of timber to support a weight.

Trussed gir der—a beam strengthened by trussing.

Tuck pointing—filling joints in masonry or brickwork and forming a raised joint. Tympanum—the triangular space in the front of a pediment between the horizontal and sloping cornices.

oat of plaster,

Underpinning—rebuilding the lower portion of a wall without pulling down the upper part.

Valley—a junction of two inclines of a roof.

Valley board—board forming a valley.

Vane—a finial of a roof.

Vault—(1) an arched roof of masonry or brickwork; (2) a compartment securely built for the storing of valuable articles.

Veneer-a thin layer of any material covering a coarser material.

Volute-spiral curves on a flat surface.

Voussoir-keystone and other radiating stones of an arch.

Wainscoi-a base or dado.

Wall plate—a narrow timber laid on the top course of the face of a wall to receive the eaves boards, etc.

Winders-triangular steps of a staircase at a corner or bend in the flight.

Weepers or weeping drain pipes—a drain laid with unjointed pipes for the purpose of dispersing surface water.



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### NOTES ON ESTIMATING.

Cubic measure is length, breadth and height or depth multiplied together. Superficial or square measure is the multiplying together of the two dimensions of a surface.

Lineal, running or run measure is measurement in one length.

[See Tables.]

#### Excavating.

Take excavating to surface 12 in. deep per sq. yard,

Take excavating (ordinary) per cubic yard, Excavating over 6 ft, deep should 'a kept separate, and allowance made for staging if required.

Trenches for foundations if below general excavating, take extra at per, cubic yard. Allow for width of trenches 6 in. on each side of lowest course of footings.

Allow for casting away, or removal in other ways as specified, or for part filled in and ranned.

In rock foundations extra time must be allowed.

Strutting and planking when required per line-foot,

Allow extra cost for statting and planking over 6 ft. deep.

Clay puddling, per cubic yard. Concrete, per cubic yard.

Allow for wheeling or carting.

Levelling and consolidating ground, per cubic yard,

Post holes, at so much each, counted.

Drains, keep different sizes separate, and include cement jointing, etc., per lineal foot.

Junctions, bends, V's, T's, etc., count us τ foot lin'l of pipe, except special ones,

Cement, per barrel. Sand, per load.

" River or other special quality, per barrel, Mortar, per load.

Coacrete, 6 in. th ck, per sup, yard.

Concrete, over 6 in. thick, per cubic yard.

#### Masonry.

Stonework is generally measured by the toise, A toise varies in different localities as for instance:

A toise in Montreal = 86 Cubic feet,
" " Toronto = 54 " "
" " Hamilton = 70 " "
" " Perth Ont. = 36 " "

Take masonry, generally per toise. Allow for hoisting, adding an extra price for every 20 ft, over the first 40 ft. of height.

Ashlar, or other face work, per sup'l ft. Backing, except very thick walls, per cubic

ft. or toise. Door and window openings to be taken as

solid, to allow for labor and cutting.
Cut Stone, per superficial ft.

Each kind of stone to be kept separate, and each kind of work on each as specified.

Allow for different treatment, as hammerdressed, bush hammered, grooved, throated, sunk, chamfered, etc., etc.

Cut stone must be measured to suit courses of brick work,

Treads and risers, per superficial ft.
String courses, per superficial ft.
All stone under 3 in. thick, per sup'l ft.
Chamfers, beads, margins, etc., per lin'l ft.
Mitres, stopped ends, stop chamfers, counted.

Landings, pavings, etc., per superficial ft, Nosings, and other edge work, per lineal ft. Rebated joints, etc., per lineal ft.

Pointing as specified, per lineal ft.

Allow for templates

' 'hoardings.
' 'scaffolding where required.

Rubble masonry when built in courses requires per cubic yard about 35 cubic feet of stone, including waste, and 6½ cubic feet of mortar.

Random and uncoursed rubble 33 cubic feet of stone and 9 cubic feet of mortar 1 cubic yard ashlar requires, including waste, 29¾ cubic feet of stone and 2¾ cubic feet of mortar.

I lineal yard pointing to ashlar work requires from one-twentieth to one-thirtieth of a cubic foot of mortar or cement. Size For

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#### Brickwork,

Sizes of bricks vary in different places, but usually,—

For face work allow 7 bricks per sq. foot, solid "20" cub. ft, "cub. yd.

Cub. yd, of brickwork requires about 5 cub. ft. of mortar.

1 Rod of brickwork = 272 sup. ft. of 1½ brickwork.

Take brickwork by cubic feet, and count per thousand.

Face work by sup'l feet, and count per thousand.

Make no deductions for flues less than 2 feet square, or for cavities in hollow walls.

Také doorways and windows as solid, except where there are many, in which case deduct half.

Circular brickwork over 25 ft. radius, measure first as ordinary brickwork, and then measure one face and allow for that amount extra labor.

Brickwork built fair, both sides to be kept separate.

Backing to stone work, per cub. yard.

Allow for eutting to fit against stone, measure foctings, averaging them by adding together lengths of top and bottom courses and dividing by 2.

Moulded bricks, per lin'l foot, and per 1,000.

Brick on edge, per sq. yard.

Brick nogging, measuring over woodwork, per sq. yard.

Brick nogging requires ¾ cub. ft. mortar when flat.

Brick nogging requires ½ cub. ft. of mortar when on edge.

Pointing, per sq. yard.

requires if flat 1/6 cub. ft. lime mortar, or 1/8 bushel cement.

requires if tuck, ½ cub. ft. putty or ½ cub. ft. mortar.

#### State Mason.

Slate slats, per superficial foot.

Allow for bedding in cement, or screwing with copper screws, as specified,

Slate skirtings, per lineal loot.

Rounded edges, rebates, grooves, etc. etc., per lineal foot.

Rounded corners, perforations for pipes, sinkings, holes for basins, etc. etc., counted.

#### Carpenter and Joiner

One square = 100 superficial feet.

Rough timbers, as joists, beams, studding, roof timbers, rafters, etc., per cubic feet.

Care must be taken in calculating the correct number of joists required.

Timbers under 3 in, square, per lineal foot, Timbers under 2 in square, per sup, ft.

Keep separate the various sizes of timbers; flooring, root boarding, shingling, etc. per square.

Allow for waste in cutting, 25 per cent.

Eaves boards, gutter boards, flashing boards, barge boards, bracketting, facias and the bearers to these, per sup. ft.

In framed work allow for length of tenon.

Allow for laps to plates, etc., 6 in. in every 20 feet.

Take bolts, straps, plates, etc., to principals. Ribs, chamfers, mouldings, rebates, strutting or bridging to floors, eaves and valley fillets, hip and ridge rolls, etc., per lin'l ft.

Note:—In drawings the actual length of hip rafters rarely appears. They must be set off to scale before figuring.

Ends of rafters, hip knobs, cleats, scarfings, counted.

For scarfings where they are not resting on a principal, allow four times the depth of the beam to every 20 ft, run extra.

Keep different kinds of work separate, and all circular work separate.

Skirtings, mitred boards to hearths, grounds, nosings, architraves, wall strings, and outer strings of stairs, hand, and other rails, cornices, mouldings, door frames, etc. etc., per lineal foot.

Window sashes, casements, and frames, shutters, and boxings, window boards, and bearers, panelling, dados, doors, jamb linings, w.c. fittings, bath fittings, treads, nosers, and winders, seats, shelving, and bearers, and ceiling lights, casings, etc., per sup. foot.

Newels, mitrings, housings, fitted ends and bosses, pendants, etc. etc., counted.

Mantles, arcades, over-doorways, require to be figured in detail.

Allow for attending on other trades and making good after them.

Allow for clearing up waste and rubbish, and removing shavings from between joists, before laying flooring.

Allow for additional scaffolding as required.

#### Roofer.

Felt and gravei other similar roofing material, per square 100 feet.

Allow for lapping of feet. d laying double at edges, also for wood strips for securing.

#### Stater.

Slates, including nailing, per square of 100 ft. Allow for cuttings against dormers, chimneys, skylights, hips, valleys, etc.

Take double course at eaves, valleys and other edges.

Slate hips, ridges, etc., per lineal foot. Felt, if required, per square.

Allowances for laps, etc., as in rooter.

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#### Galvanized Iron.

For flat or inclined roofs, per square.

Allow about 6 in. extra for each drip, raised joint, etc.

Gutters, allowing for turning up under slate say o in. on each side, and for laps 4 in. in every 7 ft., per sup. foot.

Flashings allow 4" in every 7 it.

step per lir.eal ft. Ridges, hips and valleys and allowance for laps as above, per lineal foot.

Oak wedging for flashings.

Copper nailing as specified, per lineal foot. Rainwater pipes, eaves, gutters, etc., per lineal foot.

Returned ends, stopped ends, outlets, heads, knees, feet, junctions with drains, cleats, etc., counted.

#### Plasterer.

Keep separate internal and external work. Materials required for plastering per yard super:

Brown	(lime (u	nslak	ed)		15 cul	bic ft.
coat		-			3	**
only	hair		-		r lb.	
3/8" thick	water	-	-		2 ; 1	
	flime (i	ur slal	ked)		22 cu	hie ft.
and	sand		-		23	11
finish coat	hair	-	-		12 lb.	
訁" thick	water		-	1.	8 gall	on.
_	(lime (u	nslak	ed)		25 cu	bic ft.
2 coats	sand		-		38	4.6
5/8" thick?	hair	-			18 lb.	
	water		-	2.0	oo gal	lon.
3 coats	(lime (u	nslak	ed)		32 cu	bic ft.
3 coats	ınd		-		38	4 6
3/4" thick	hair	-	-		18 lb.	
	water		•		6 gall	
Finish v	vith [1	ime (	unsla	eked)	. 10 C	ab. ft,
putty a	ind {	plaste	r of I	aris	.03	4.6

plaster 1/8" thick (water 1,00 gallon. For rubble or rough brick walls the above quantities should be increased.

The first or "pricking up" coat on laths requir s about 1-10th more coarse stuff than "rendering only," or brown coat only.

Laths, containing nominally 400 feet lineal will cover about 41/8 super yards.

Nails—a bundle of 3-ft. laths takes 500 nails. 4-ft.

Hair-a bushel of dry hair weighs from 14 to 15 lbs. In best work allow I lb. of hair to 2 cubic feet

of mortar. Ordinary work, 1 lb. hair to 3 cubic feet of mortar.

Lime whiting once done requires 1 cubic foot slaked lime per 100 sq. yards; twice done, 1% cub. ft. of lime.

Take plastering (generally) to walls, partitions, ceilings, soffits of stairs, etc., two or three coat work as specified, and lathing as described, per sup. yard.

Small quantities, per sup. foot. Cornices under 12 in. girth, per lineal foot. Cornices over 12 in. girth, per sup. foot. Cornices, if bracketted, allow for lathing. Enriched mouldings, per lineal foot. Ornaments, bosses, etc., counted.

Mitres, stopped and returned ends, counted. Moul ed ribs, strings, straight or forming panels, per lineal foot.

NOTE.—Laths — In specifying lathing the terms "lath," "lath and a half," "double lath," refer to the thickness of the laths.

> single laths  $= \frac{1}{8}$  in. to 3-16 in. tnick. lath and a half= 1/4 in thick. double laths=3% in. thick.

#### Plumber and Gasfitter.

Lead in sinks, cisterns, etc., per sup. foot. In measuring lead care should be taken to measure exactly, as small errors become serious when quantity is reduced to

Copper nailing, soldered angles, per lineal

fron cisterns, giving capacity, counted. Slate cisterns, per sup. foot.

Allow for all drillings for pipes, balls and ball cocks, wastes, closet valves, ball levers,

Pipes, tin cased, lead, iron, of all kinds, per lineal foot.

Gas tubing, per lineal foot.

Allow for joints soldered or screwed, wiped, caulked; bends, faucets, offsets, valves, elbows, T's, Y's, straps, etc.

Brackets and pendants a sum is generally mentioned.

Allow for burners, glass shades, etc.

Lavatory, urinal w. c. bath fittings all according to description.

Ascertain if joiner is to attend on plumber and do all cutting and making good after. Pumps and fixtures described and counted. Allow for special requirements for each kind.

#### Tiler.

Plain tiling, per square. Allowances as for slate. Hips, ridges, etc., per lineal foot. Finials, etc., counted.

#### Hardware.

According to specification, counted.

Heav we Alloy SC Ascer gi

Keep the

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CO Grati ta ba et Iron sh

> ta Hole For fo

Keep

Mea er Si Allo w Emil 10 Lea

Plat li p Allo b

Beve

Kee I lb C 3

# 1b I lb

walls, partis, etc., two or , and lathing

lineal foot, up, foot, r lathing, foot, d, ods, counted,

lathing the alf," "double of the laths.

t or forming

in. thick. k.

ter. sup. foot.

be taken to rrors become reduced to

es, per lineal Dunted,

balls and ball ball levers

ll kinds, per

wed, wiped, s, valves, el-

is generally

etc. sall accord-

olumber and d after. l counted. r each kind. fron Founder.

Keep separate wrought and cast iron, also the different articles.

Heavy work per cubic foot, and calculate weight in tons at so much a lb. as specified. Allow for hoisting and fixing, cartage, extra

scaffolding, etc.

Ascertain what assistance is specified to be given by other trades.

For vaults get special prices.
Allow for patterns for all eastings.

Allow for drilling, bolting and all other work to be done after placing in position.

Rivetted girders, fish-plates, pavement lights, coal plates and frames, per sup, foot.

Gratings, bars, rails, straps, bolts not already taken by carpenter, saddle bars, guard bars, balusters, gates, railings, hand rails, etc., per lineal foot.

Iron roof principals, rafters, purlins, with all shoes, cleats, brackets, ties, T irons, etc.,

take in detail and keep separate. Holes, perforations, etc., counted.

For girders, etc., consult price lists of iron founders.

#### Glazier

Keep different kinds of glass separate.

Measure all glass in inches and turn into superficial feet, measuring to extreme dimensions.

Allow for sprigging, puttying, or bedding in washleather or india rubber as required.

Embossing, enamelling post lights, per sup. foot.

Lead lights, per sup, foot, Bevelling, per lineal foot.

Plate glass, the price of, is apt to vary; price lists should always be obtained from importers,

Allow for cleaning at completion and for breakages,

#### Painting.

Keep separate different classes of work.

I lb. white paint mixed with oil, etc., will cover on wood about 4½ sup. yards, first coat; second coat same quantity will cover about 6½ sup. yards, and each additional coat about 6½ sup. yards.

\* lb. red lead paint mixed and applied as a first coat on iron, about 5½ sup. yards. I lb. oxide of iron paint mixed and applied

as a first coat on iron, from 8 to 12 sup.

I gallon tar with 1 lb. pitch included, applied hot, will cover about 12 sup. yards first coat on wood and 17 sup. yards each additional coat.

I lb. of putty for stopping is required for every 20 to 25 yards of surface.

Take painting (generally) per sup. foot.

Allow for knotting, stopping, etc.

Ascertain if priming is included when two, three or four coat work is specified.

Allow extra when work is finished in two or more colors.

Count windows and double them for painting both sides.

Oiling, rubbing, staining, sizing, varnishing, French polishing, graining, etc., per sup. foot.

Railings, gates, gratings, measure round of bars.

Measure 1 and of bars, open framing, etc. Skirtings, cornices, strings, etc., per lineal ft.

#### Paper Hanging.

Generally, per sup. foot.

Allow for waste.

To find the number of pieces (12 yards) required for a wall, take the superficial area deducting doors and windows and divide by 54. This rule applies to an average width of 20 inches to the piece.

Allow for filling cracks in old walls. Measure borders, per lineal yard.

Bell Hanger.

Specification should state positions of bells and pulls,

Bells and pulls, counted,

Allow for fixing with all cranks, wire, springs, plates.

Zinc or copper tubing, per lineal foot. Ascertain if various trades are to do cutting

required in their own work.
For electric bells and fittings get special

prices from engineers.

Pneumatic bells, get special prices from en-

Speaking tubes, per lineal foot.

Allow for mouth pieces and all fixing as required.

#### Electric Lighting.

Get special estimates from engineers.

#### Heating Apparatus.

Get special estimates from manufacturers.

#### NOTES ON HOSPITAL WARDS-MINIMUM DIMENSIONS.

 Space between beds
 7 ft. 6 in.

 Superficial area per bed
 90 sq. ft.

 Height of ward
 14 ft.

 Cubic space per bed
 1260 cub. ft.

 Width of ward
 24 ft.

 No. of beds to a ward (not more than)
 32. [Galton.]

### WEIGHT OF SUBSTANCES PER CUBIC FOOT.

WEIGHT OF SUB	OIMI	CES FER CODIC FOOI.
A	Average Weight	Average Weight
	in lbs.	in lbs.
Aluminium	162	Mahogany, Spanish, dry 53
Ash, American white, dry	38	" Honduras dry 35
Asphaltum	87	Masonry, granite, or limestone,
	-	(well dressed) 165
Brass, (copper and zinc) cast	504	Masonry, mortar, rubble 154
" rolled	524	" dry rubble 138
Brick, best pressed	150	(/ 1 . 11 1 1
" common, hard	125	14
" soft, inferior	100	
Brickwork, pressed brick	140	Mud, dry, close80 to 110
" ordinary	112	Mud, wet fluid, maximum 120
Cement, ordinary	56	Oak, live, dry 59
" English, Portland	90	" white, dry 59
Cherry, dry	42	other kinds32 to 45
Chestnut, dry	41	Pine, white, dry 25
Coal, broken, loose	49	" yellow, (Northern) 34
" solid	84	" " (Southern) 45
Coke, loose	62	Salt, Liverpool, fine for table use 49
Copper, cast		Sand, of pure quartz dry loose,
" rolled	542	90 to 106
" rolled	548	Sand, well shaken99 to 117
Earth, dry, loose	76	" perfectly wet120 to 140
" " moderately ram-		Sandstone, fit for building 151
med	95	Clata
Elin, dry	35	Sne w, freshly fallen 5 to 12
Gold, cast, pure, 24 carat	1204	Sn w, moistened and compact
Granite	170	
Hemlock, dry	25	by rain
Hickory, "	53	Spruce, dry
_		Steel
Ice	58.7	Sulphur
Iron, cast	450	Tar 62
" wrought, purest	485	Tin, cast 459
average	480	Turf or peat, dry unpressed
Lead	711	20 to 30
Lime, quick ground, loose	53	Walnut, dry, (black) 38
" " thor-		Water, pure rain or distilled, at
oughly shaken	75	60° Fahr
Limestones and marbles	168	Water, Sea 64
" " loose in		Zinc, or spelter
irregular fragments	96	Green wood, add from one fifth
Maple, dry	49	to one half more than dry
Contact Contac		
APPROXIMATE BREAKING	STRE	NGTHS OF WIRE IN TONS, PER
		INCH.
Dright iven were	25 tons	per square inch, tensile resistance.
Bright iron wire,	35 "	
Bessemersteel wire,	10	<i>ii ii ii ii ii</i>

Annealed iron wire, 25	tons	per	square	inch,	tensile	resistance.
Bright iron wire,35	"	• 66	- 66	"	66	66
Bessemersteel wire,40	66		66	66	66	66
Mild Siemens steel, wire60	"	66	46	"	66	66
Crucible cast steel wire80		ο.	66	"	66	46
Patent plough crucible wire10			66	46	66	44

## VARIOUS TABLES

#### TABLE OF CUBES.

Cube of	Εī	is	1	Cube of	7	is	343
11	2	11	8	11			
11	_			11	9	11	729
1.11				11	10	11	1000
11			125	11	11	11	1331
**	6	11	216	11	12	11	1728

#### ALGEBRAIC FORMULAE.

- 1.  $(a+b)(a+b)=a^2+2ab+b^2$ .
- 2.  $(a+b)(a-b)=a^2-b^2$ .

Average Weight in lbs,

stone,

ressed

.. 80 to 110

... 32 to 45

le use

loose,

...90 to 106

.99 to 117

120 to 140

.. 15 to 20

ssed ..20 to 30 .... 38

38 1, at ... 62½ ... 64 ... 43.7

fifth 'y

, PER

ance.

35

165 154 138

103

120

59

**5**9

25

34

45

49

151

.... 175 ...5 to 12

- 3.  $(x+a)(x+b)=x^2+xa+bx+ab$ .
- 4.  $(a+b)(a^2+ab+b^2)=a^3+b^3$ .
- 5.  $(a-b)(a^2+ab+b^2)=a^3-b^3$ .
- 6.  $(a+b)^3=a^3+b^3+3a b+3ab^2$ .
- 7.  $(a-b)^3 = a^3 b^3 3a^2b 3ab^2$ .
- 8.  $\frac{x^2 + xy + y^2}{x y} = x^3 y^3$
- 9.  $\frac{x_2 + xy + y_2}{x + y} = x_3 + y_3.$
- 10.  $(a+b+c)(a+b+c)=a^2+b^2+c^2+2ab+2ac+2bc$ .

# FORCE AND VELOCITY OF WIND, AND ITS PRESSUREUPON ROOFS, SPIRES, ETC.

	VELOCITY	7.	FORCE OR	
MILES PER		FEET PER	PRESSURE	DESCRIPTION
HOUR	MINUTE	SECOND	PER FT. SUP.	
1	88	1.47	.005	Barely perceivable.
2	176	2.93	.02	
3	264	4.4	.044	Just perceivable.
4 5	352	5.87	.079 (	Light breeze.
	440	7.33	.123∫	Light breeze.
10	880	14.67	.492)	Pleasant breeze.
15	1320	22.0	1.107∫	ricasant biceze.
20	1760	29.3	1.97	Brisk gale.
25	2200	36.6	3.067∫	Erion Bure.
30	2640 3080	44.0	4.429	High wind.
35 40	3520	51.3 58.6	6.027	
45	3960	66.0	7.87	Very high wind.
50	4400	73.3	9.9 J 12.304	Storm.
- 60	5280	88.o	17.733)	Storm.
70	6160	102.7	24.153	Great storm.
· 8o	7040	117.3	31.49	
100	8800	146.6	49.21	Hurricane.

## Weights of Materials.

The weight of	of wrought	iron 2	X	0.038	=	cast iron	•
" C				0.028	=	zinc.	
"	46			1.010	=	steel.	
"	"			1.085	==	brass.	
46	"			1.144	=	copper.	
"	"			1.468	==	lead.	

#### THE WEIGHT OF WROUGHT-IRON BOLT HEADS, NUTS AND WASHERS.

Diam.	Hexagon H'ds & Nuts.	S'qre. Heads and Nuts.	Round Washers.	Diam. of Bolt.	Hexagon H'ds & Nuts.	Sq're Heads and Nuts.	Round Washers,
inches.	per pair.	per pair.	per pair	inches.	per pair.	per pair:	per pair,
1/4	20 to a lb.	16 to a lb.	20 to a lb.	13/8	3.00	3.60	3.22
1/4 3/8	10 "	81/3 "	10 "	1 1/2	3.75	4.42	4.22
1/2	5 "	41/6 "	5 "	1 5/8	4'75	5.40	5.22
5∕8	234 "	21/2 "	3 "	1 3/4	5.75	7.00	6.20
1/2 5/8 3/4 7/8	2 "	·56 lb.	63 lb.	1 7/8	7.27	8.72	8 00
7/8	.77 lb.	·83	.77	2	8.75	10.20	9.60
1	125	1.31	1.25	2 1/2	17.00	21.00	19.00
I 1/8	1.75	2.10	1.72	3	28.80	36.40	32.20
1 1/4	2.13	2.26	2.52				

#### ROUND AND SQUARE IRON-WEIGHT OF A LINEAL FOOT.

Diam. or Side in inches.	Round in lbs.	Square in lbs.	Diam.or Side in inches.	Round in lbs.	Square in lbs.	Diam.or Side in inches.	Round in lbs.	Square in Ibs.	Dizm.or Side in inches.	Round in lbs.	Square in lbs.
1/8 1/4 5-16 3/8 7-16	*041 *165 *258 *372 *506 *661 *837	°053 °210 °329 °474 °645 °842 1°066	1 5/8 1 3/4 1 7/8 2 2 1/8 2 1/4	9°300 10°581 11°945 13°392	8.894 10.315 11.841 13.472 15.209	3¾ 3½ 4 4½ 4½ 4¾ 4¾	34.761 37.199 39.720 42.324 45.011 47.780 50.632	47'363 50'573 53'889 57.310 60 835 64'467	6 1/4 6 1/2 6 3/4 7	91'303 95.230 103'331 111'763 120'525 129'618	142'300 153'457 165'035
11-16 34 13-16 78 15-16 1 11/8 11/4 13/8	1.033 1.250 1.488 1.746 2.025 2.325 2.645 3.340 4.133 5.001	1'316 1'592 1'895 2'223 2'579 2'960 3'368 4'263 5'263 6'368	2½ 25/8 23/4 27/8 31/8 31/4 33/8	14'921 16'533 18'228 20'205 21'865 23'807 25'833 27'941 30'131 32.405	21.050 23.208 25.471 27.839 30.312 32.891 35.575 38.364	4 3/8 4 3/4 4 3/8 5 1/8 5 1/8 5 1/4 5 3/8 5 1/2		75'992 80'044 84'201 88'464 92'832	8 8½ 9 9½ 10 10½ 11 11½	148'796 169'297 191'121 214'267 238.736 264'527 291'641 320'078 349'837 380'919	215.556 243.352 272.812 303.967 336.866 371.328 407.535 445.425

Diame Circle a and S Squ

3-3-3-5-3-7-7-7-7-7-7-11-3/4 13-7/

1/8 1/2 5/8 3/4 7/8

1 1/8 1 1/4 1 3/4 1 1/2 1 3/4 2 2 1/4 2 1/2 2 3/4

3 3¼ 3½ 3¾ 3¾

4 ½ 4½ 4¾ 5 5½ 6

## ROUND, OCTAGONAL, AND SQUARE STEEL-THE WEIGHT OF A LINEAL FOOT.

Diameter of Circle and Oct. and Side of Square.	Round in lbs.	Octagonal in lbs.	Square in lbs.	Diameter of Circle and Oct. and Side of Square.	Round in lbs.	Octagonal in lbs.	Squa e in Ibs.
1/8	.0417		0532	I	2.673	2.819	3,403
3-16	<b>'0</b> 940		.1199	I 1/8	3,385	3.268	4.302
1/4	1670	, ,	2127	1 1/4	4.126	4:405	5.312
5-16	<b>'2</b> 610	2753	*3323	1 3/8	5.023	5.330	6.433
3/8	·37¢8	*3964	4785		6.013	6.343	7.656
7-16	5.15	*5396	6513	1.58	7.057	7.444	8.985
1/2	.6681	.7047	.8507	1 34	8.182	8.633	10'421
9-16	.8456	.8919	1.077	1 7/8	9.396	9,010	11'963
5/8	1'044	1.101	1.329	2	10.690	11.276	13.611
11-16	1.563	1 332	1.608	2 1/4	13.530	14.271	17.227
3/4	1.203	1.286	1914	2 1/2	16.703	17.618	21.267
13-16	1 764	1.861	2.246	2 3/4	20,511	21.318	25.734
7/8	2.046	2,128	2.605	3	24.053	25.371	30.625
15-16	2.349	2.478	2.99				

Note.-The diameter of Octagon Steel is measured across the sides.

WASHERS.

Round Washers,

per pair.
3.25
4.25
5.25
6.50

8 00

#### FLAT STEEL.—THE WEIGHT OF A LINEAL FOOT.

9.60		FLAT STEEL.—THE WEIGHT OF A LINEAL FOOT.								
19.00	dth ches.				Thickness	in inches.				
32.20	Width in inches.	1/8	1/4	3/8	1/2	5/8	3/4	7/8	ī	
оот.	1/8	lbs.	lbs. 1063	1bs.	lbs. '2127	lbs. 2658	lbs. *3190	lbs.	lbs. '4253	
und Square in lbs.	1/2 5/8 3/4	'2127 '2658 '3190	'4253 '5317 '6380	6380 7975 9570	.8507 1.063 1.276	1.063 1.329 1.95	1.395 1.314	1.489 1.861 2.233	1.401 2.152 2.252	
459 111 356	7/8	'3722 '4253	7443 8507	1.112	1.489	1.861	2.233	2.605 2.976	2 977 3'403	
'303 116'251 .230 121'250 '331 131'565	11/4	'4785 '5317 '5849	1.063	1.436 1.595	1'914 2'127	2 393 2.658	2.871 3.100	3'350 3'722 4'094	3.828 4.253 4.679	
.763 142.300	1 1/2 1 3/4	6380	1.170 1.276 1.489	1.755 1.914 2.233	2.339 2.552 2.977	2°924 3°190 3°722	3.509 3.828 4.466	4.466 5.511	5°104 5 955	
'525 153'457 '618 165'035 '796 189'453	2 2 1/4	.8507 .9570	1'701	2.221	3.403 3.828	4°253 4°785	5°104 5 742	5'955 6'699 7'444	6·806 7·656 8·507	
297 215.556 121 243.352	2 3/4 3	1.140	2.127	3.190 3.828	4°253 4°679 5°104	5.317 5.849 6.380	6·380 7·018 7·656	8 188 8 932	6.328 0.328	
.736 303.967	31/2	1.385 1.480	2.76 <b>5</b> 2.977	4·147 4·466	5.530	6.912 7.444	8.294 8.932	9.677 10.421 11.165	11.059 11.910 12.760	
.527 336.806 .641 371.328	41/4	1.201	3.190 3.403 3.190	4.785 5.104 5.423	6 806 7'231	7'975 8'507 9.039	9°570 10°208 10,846	11.02	13.611 14.462	
.078 407.535 .837 445.425 .919 485.000	4 1/2 4 3/4	1'914	3.828 4 04 I	5.242 6.061	7.656 8 o82	9.570	11'484	13.398	16.163 12.313	
919 405 000	5 1/2	2.339	4.679 5.104	7.656 7.656	8.204 9.328 10.308	10.634 11'697 12'761	12'760 14'037 15'313	14.887 16.376 17.86 <b>5</b>	18.412	

ROUND CAST IRON.—THE WEIGHT OF A LINEAL FOOT.

Diam, in inches.	Weight in lbs.	Diam. in inches.	Weight in lbs.	Diam. in inches.	Weight in lbs.
1 1/4 1 1/2 1 3/4 2 2 1/4 2 1/2	2'454 3'835 5'522 7'517 9'818 12'425 15'340	5½ 5¾ 6 6¼ 6½ 6¾ 7	74°245 81°148 88°357 95°874 103°697 111°827 120°264	10 10¼ 10½ 10¾ 11 11¼ 11½	245'437 257'86, 270'595 283'634 296'979 310'632 324'591
23/4 3 31/4 31/2	18·561 22·089 25·924 30·066	7 ½ 7 ½ 7 ¾ 8	129°008 138°059 147°416 157°080	1134 12 13 14	338·857 353·430 414·789 481·057
3¾ 4 4¼ 4½	34.515 39.270 44.332 49.701	8 ½ 8 ½ 8 ½ 8 ¾ 9	167°051 177°329 187°913 198°804	15 16 17 18	552°234 628°320 709°314 795°217
4 <sup>3</sup> / <sub>5</sub> 5 <sup>1</sup> / <sub>4</sub>	55°377 61°359 67°649	9½ 9½ 9¾	210.002 221.204 233.319	20 22 24	981.750 1187.91 1413.7

## ROUND AND SQUARE BRASS.—THE WEIGHT OF A LINEAL FOOT.

Diam. or Side	Round in lbs.	Square in lbs.	Diam. or Side in in.	Round in lbs.	Square in lbs.
1-16	0112	°0142	1 5-16	4*933	6.581
3-16	°0447 °1012	°0570 °1282	1 3/8 1 7-16	5.414 2.414	6.893 7.534
5-16	°1790 °2796	*2279 *3560	I ½ I 9-16	6.443 6.991	8.303
3/8 7-16	'4047 '5486	·5127 ·6978	1 5/8 1 11-16	7·563	9.627 10.382
1½ 9-16	7159 9060	'9115 1'154	I 34 I 13-16	8.789 9.407	11'165
5/8 11-16	1.323	1 '424 1 '723	1 7/8 1 15-16	10'117 11'799	12.817 13.686
3/4	1.801	2.051 2.407	2 1/8	11.454 12.932	14°583 16'463
13-16	2.194	2.421 3.504	2 1/4 2 3/8	14.496 16.152	18.457 20.565
15-16	2.25 2.863	3.646 4.119	2 1/2 2 5/8	17.896	22.786
1 1-16	3.233 3.624	4.614	23/4	21.655	25.122
1 3-16	4.038 4.474	5.694 2.141	27/8	23.670 25.771	30.132

00	ЭТ.	
	Weight in lbs.	
1	245.437 257.86 270.595 283.634 296.979 310.632 324.591 338.857 353.430 414.789 481.057 552.234 628.320 709.314 795.217 981.750 1187.91	
L	Fоот.	St. Philippen

CAST-IRON PIPES.—THE WEIGHT OF A LINEAL FOOT,

 1 001.
Square in lbs.
6.281 6.893 7.534 8.203 8.902 9.627 10.382 11.165 11.977 12.817 13.686 14.583 16.463 18.457 20.565 22.786 25.722 27.572 30.135 32.813

Bore in	inches.		,	1 0	4,	2/2	**	(1)	4,00	31/2	334	4	717	†/I ~	73/2	t t u	71 1	, J. J.	7.9	1/2	71/	.∞	81%	0	0.2	10	101	7/21		
	13%	1150	51.542	11000	010	50.70	705.70	50.200	09.950	73.631	77.313	80.994	84.676	88.357	92.030	95.721	103.084	110.447	117.810	125.173	132.536	139.899	147.262	154.626	161.989	169.352	176.715	18:078	198.804	212 521
	1,7	lhe l	39.834	42.052	46.010	10.01	17.00/	77.22	55.77	58.291	61.359	64.427	67.495	70.563	73.631	76.699	82.835	88.971	95.107	101.243	107.379	113.515	119.651	125.737	131.923	138.059	144.195	1.0.330	162.602	174.874
	11/8	lhe	34.515	37.276	40.027	70.07	01111	40.004	40.020	51.002	53.843	56.604	59.365	62.126	64.888	62.649	73.171	78.693	84.216	89.738	95.260	100.783	106.305	111.827	117.350	122 872	128.394	133.917	144.962	1:6.006
	ı	lbs.	29.452	31.907	34.261	36.816	30.276	12.77	47.74	44.1/9	46.633	49.087	51.5.12	53.995	56 451	58.905	63.814	68.722	73.631	78.5.0	83 449	88.357	93.266	98.175	103.084	107.992	112.501	117.810	127.627	137.445
etal in inches.	% %	The.	24.697	26.845	28 002	21 1.40	22.287	701.10	20.4.00	3/.503	39.730	41.878	44.025	46.177	48.320	50.468	54.763	59.058	63.354	67.649	71.944	76.239	80.534	84.829	89.124	93.420	97.715	102.010	110.600	101.011
I hickness of Metal in inches.	34	lbs.	20.249	22.089	23.050	25.771	27.612	20.452	17.434	51.295	33.134	34.975	36.816	38.656	40.497	42.338	46.020	49.701	53.383	57.064	60.746	6 1.427	68.109	71.790	75.472	79.154	82 835	86.517	93.880	101.243
10	%% %	lbs.	16.107	17.641	19.175	20,700	22.243	23.777	17.7.	110.07	20.845	28.379	29.913	31.447	32.981	34.515	37.583	40.651	43.719	46.787	49.854	52.922	55.990	59.058	62.126	65.194	68.262	71.330	77.466	83.602
	1-2	lbs.	12.272	13.499	14.726	15.953	17.181	18.408	10 625	20.023	20,002	22.089	23.317	24.544	25.771	26.998	29.452	31.908	34.361	30.816	39.270	41.724	44.179	40.033	49.087	51.541	53.990	56.451	61.359	66.268
-	3%	lbs.	8.744	9.664	10.584	11.505	12.425	13.346	14.266	981 11	15.100	10.109	17.027	17.948	18.868	19.788	21.629	23.470	25.31I	27.152	29.992	30.033	32.074	34.515	30.355	38.190	40.037	41.878	45.539	49.241
	×	lbs.	5.522	0.130	6.750	7.363	7.977	8.590	9.204	0.817	70.61	10.431	11.045	11.058	12.272	12.685	14.113	15.340	10.507	17.794	19.021	20.249	21.4/0	22.703	23.930	25.157	20.305	27.012	30.000	32.520
Bore in			(1	7,7	21/2	234	3	3.74 	100	23/	t -	71	4 4;	4 (1)	4 ×		6,	0	;; ;;	1	0/72	200	2/0	7	2/6	10	10/2	11	21 ;	13

15 56'604 76'086 106'286 80'994 85'903 18 67'649 90'812 20 100'629 21 100'629 22 23 115'356 23 139'99! 28 28 28 28 28 28 28 28 28 28 28 28 28								Dore in
\$6.286 \$6.286 \$7.664 \$7.649	200	76	%	н	8 /I:	7,1	11/2	lnches.
50.086 60.286 63.967 67.649		lbs	lbs.	lbs.	lbs.	lbs.	lbe	
64.000	95.874	696.511	136.371	080.251	960.841	199.418	242.083	<u>'</u>
64.64.64.64.64.64.64.64.64.64.64.64.64.6		123.332	144.962	166.897	189.140	211.600	002.236	12
649.49		130.695	153.52	176.715	200.185	223 062	277.736	17
	114.282	138.059	162.142	186.532	211730	236.234	287.162	18
	120.418	145.422	170.732	022.901	240.100	248	988.100	-
		182.281	170 272	206.167	010.000	260:111	301 000	61
	132.000	160.148	187.012	70000	233 319	/// 007	510014	0
	_	241	10/913	212925	244 304	3/3 049	331.341	21
		10/ 511	195 563	208.522	255.408	285.32I	346.067	C1 C1
	5 114.062	172.873	100:502	009.200	. 66.452	70100	260,100	6
	-	18222	12.684	233.040	011.00	200.000	300./93	5.3
	_	/C= =0.	+00014	704 047	064 //-	309 605	3/5 519	77
• • •	15/ 255	100 000	222.274	255.255	288.542	322.137	390.246	25
		196.964	230.855	265.072	299 587	334.409	404.972	26
		204.327	239.455	274.890	310.632	346.680	419.698	27
		069.117	248.045	284.707	321.677	358.052	434.424	00
:	-	219'053	256.636	204.525	332.721	371.224	121.000	000
30 149'717	187.913	226.416	265.226	304 342	343.766	383.406	462.877	, ,

TABLE OF THE WEIGHT OF CAST-IRON FLANGED

WEIGHT OF CAST-IRON SOCKET PIPES.

	I		S	o c	o ru	(0)	<b>—</b>	~	0.	<b>→</b>	10	0.0	10	0		0 .	0.0	<u> </u>	
	Weight er Pine	1				H		9	I	24 c		250	25		-	¬ -	2,5	1	
	Weight per Pipe	; 1		ი - 0 -	. H		w.	4 rv 20 cs			9 3	_	-		() v		1 C		
			1 3	4 4	+ 4					9 9					13		-	-	
	Diameter and Number	of Holes.	Z									% \%						_	
	Dia	Ö	.si s		6%4	m, 1			H =	-	1		11/8	74.	7 -	7 %	3/3		
(NYSTROM.)	Diameter of Circle through	Holes.	inches.	4℃ ¾	734	× × ×	10	<b>-</b>	4	16	1734	10% 19%	2034	5 5	77 C	† 17	2 6	2	•
(NYS1	Thickness of	Flanges.	inches.	9-10 %	° %†	1%1	%		01-I	1 3-16	747	**************************************	17	01-5-10	Τ.,	13%	13%	1/3	
(Nys		Flanges.	inches.	1000	91/2	2/01	12	1.7	101/2	19	20	22	23.	24 1/2	22/2	2000	300	- 7	
		Metal.	nches.	8 %	0 74	70,2	% \\ \% \\	% %t	%† %	t 1%	120	% /% 	12	27	% _	-		•	
	Bore in in.			11 m	) 4	ru/	0 1	<b>√</b> ∞	6	2 [	12	Σ <u>1</u>	10	0 ;	_ ×	2 2	7,7	2	
Weight of Lead	Joint.	1.5	1.4	2.3	× 5	4 4			0.5	1.01	6.11	18.2	20 8	22.22	23.8	25.5	9.92	6.22	296
Size of W				74	16	01-63		9-16		5-16		100							
Size of	.: 8	74				•				in									
100	Joint.	×	; ;	X	; i	ر د	3	×3	33	×3	33	×	33	39	33	33	<b>3</b> :	3 :	3
	Joint.	1 1/2 × 1/4	* **	134	< ۰ 	< 		277 × × × × × × × × × × × × × × × × × ×		$\frac{2\sqrt{2}}{2}$ × $\alpha$		× × × × × ×				"			"
Weight		× 2/1 C	51 62	134	< ۰ 	<		2 77 × ×		×3		×;				1289			
weight of each	pipe.	30 1½ ×	g a gradual of a 100	134	137	< 	233	277 × × × × × × × × × × × × × × × × × ×		$\frac{2\sqrt{2}}{2}$ × $\alpha$		× × × × × ×	0 00				1365	1452	
th Thick- Weight	pipe.	30 1½ ×	51 62	121 134	137	210	233	314 27 ×	408	527 2½ ×	672	755 23 ×	0 00	876	1217		1365	1452	1794
h Thick- Weight	Socket Metal. pipe.	3 1/2 ×	51 62	121 134	137	210	233	314 27 ×	408	527 2½ ×	672	755 23 ×	0 00	876	1217		1365	1452	1794

VARIOUS METALS.—THE WEIGHT OF A SUPERFICIAL FOOT.

Thickness in inches.	Wrought Iron.	Cast Iron.	Steel.	Copper.	Brass.	Lead.	Zinc.	Thickness in inches.
	lbs.	lbs.	lbs.	lbs.	lbs.	lbs.	lbs.	
1-16	2 526		2.552	2.891	2.734	3.708	2.344	1-16
1/8	5.052	4.687	5.104	5.781	5.469	7.417	4.687	1/8
3-16	7.578	7.031	7 6 5 6		8.203	11.125	7.031	3-16
1/4	10.104	, ,,,,	10.208	11.563	10.938	14.833	9.375	1/4
5-16	12.630		12 760	14.453	13.672	18.542	11.719	
3/8	15.156		15.312	17.344	16.4c6	22.250	14.062	
7-16	17.682		17.865	20.234	19.141	25.958	16 406	
1/2	20.208	18.750	20.417	23.125	21.875	29.667	18.750	
9-16	22.734		22.969		24.609	33-375	21.094	
5/8	25.260	23.437	25.521	<b>28.90</b> 6	27.344	37.083	23.437	
11-16	27.786	25 781	28.073	31.797	30 078	40.792	25.781	11-16
34	30.312	28.125	30 625	34.688	32.813	44.500	28.125	
13-16	32.839	30.469	33 177	37.578	35.547	48 208	30.469	
7/8	35.365	32.812	35.729	40.469	38 281	51.917	32.812	
15-16	37.891	35.156	38.281	43.359	41.016	55.625	35.156	/ .
I	40.417	37.500	40 833	46.250	43.750	59.333	37.500	1 -

Add for each side in GALVANIZED IRON .096 lbs. per ft. sup.

### WROUGHT-IRON PIPES.—THE WEIGHT OF A LINEAL FOOT.

			Thickness	of Metal i	n parts of	an inch.			
Bore in inches.	1-16	1/8	3-16	1/4	5-16	3/8	7-16	1/2	Bore in inches
	Ibs.	lbs.	lbs.	lbs.	lbs.	lbs.	lbs.	lbs.	
1/4	.208	.497	.869	1.324	1.861	2.481	3.184	3.969	1/4
3/8	.289	.661	1.116	1.653	2.273	2.976	3761	4.629	3/8
1/2	.372	.827	1.364	1.984	2.687	3.472	4 340	5.291	1/2
58	.455	1.092	1.612	2.315	3.100	3.968	4.919	5.952	
34	.537	1.157	1.860	2.645	3.513	4.464	5.497	6.613	
7/8	.620	1.323	2.108	2.976	3.927	4.960	6.076	7.274	7/8
1	.703	1.488	2.356	3.307	4.340	5.456	6654	7.936	
1,4	.868	1.819	2.852	3.968	5.167	6.448	7.812	9.258	
1 1/2	1.033	2.149	3.348	4.029	5.993	7.440	8.969	10 581	
134	1.199	2.480	3.844	5.291	6.820	8.432	10,126	11.904	
2	1.364	2.811	4.340	5.952	7.646	9.424	11.284	13 226	2
21/4	1.539	3.131	4.836	6.613	8.473	10.416	12.441	14.549	
21/2	1.695	3.472	5.332	7 274	9.300	11,408	13 598	15 872	
234	1.860	3.803		7.936	10.126	12,400	14 756	17.194	
3	2.025	4 133		8.607	10.953	13.392	15.913	18 517	

14 5 9 2 6 0 4 7 1 5	1-16 1/8 3-16 1/4 5-16 3/8 7-16 1/2 9-16 5/8 11-16
0.	13-16 % 15-16 1
т.	
969 529 291 952	Bore in unches, 1/4 3/8 1/2 5/8 3/4

-					-						_																	
		Midt fani		91-1	18	3-10	74	91-5	3%	01-/	12/2	91-6	1%	11-16	3	12-16	2,72	15-16	, -	71 1	2 1 1	13/4	2/1	2/2	3,0	t %	,	2 2
		ı	lbs.	.2105	.4210	5100	.8420	1.053	1.203	† († )	1.084	1.895	2.105	2.316	2.526	2.737	2.947	3.158	3.368	3.780	4.210	4.631	5.052	5.473	5.804	6.315	6.736	7.157
		7/8	lbs.	.1842	.3684	2766.	.7368	.9210	1.280		1.474	1.058	1.842	2.026	2.210	2.394	2.579	2 763	2 947	3.315	3.684	4.052	4.421	4.789	5.157	5.526	5.894	6.262
		3%	lbs.	.1579	.3158	C/+:	.0315	+60/-	1.105	200	1.203	1.42I	1.579	1.737	1.895	2.052	2.210	2 368	2.526	2.842	3.158	3.473	3.789	4.105	4.421	4.736	5.052	5.368
L FOOT.		288	lbs.	.1316	.2631	1+40	.5263	0270	.0210		1.053	1.184	1.316	1.447	I.579	1.710	1.842	1.973	2.105	2.368	2.631	2.094	3.158	3.421	3.684	3.947	4.210	4.473
A LINEAL		1/2	lbs.	.1053	2105	20.0	.4210	.5203	7368	000	02400	.9473	1.053	1.158	1.263	1.368	1.4,7	1.579	1.684	1.895	2.105	2.316	2.526	2.737	2.947	3.158	3.368	3.579
OF	inches.	7-16	lbs.	1260.	.1842	.07	.3094	2004.	.6447	2367	005/	6070.	.9210	1.013	1 105	1.197	1.289	1.381	1.474	1.658	1.842	2.026	2.210	2.394	2.579	2.763	2.947	3.131
-WEIGHT	Phickness in inches.	3/8	lbs.	62/0	.1579		.3150	1594/	.5526	7212	515.	401/-	1687.	.8083	.9473	1.026	1.105	1.184	1.263	1.421	1.579	1.737	1.895	2.052	2.210	2.368	2.526	2.684
BAR IRON	I	5.16.	lbs.	.0050	.1310		12031	2012	.4605	2967	5203	0766.	.0578	.7236	1.7894	.8552	.9210	1986.	I.053	1.184	1 316	I.447	1.579	1.710	1.842	1.973	2.105	2.237
FLAT BA		74	lbs.	0250.	.1579		2621	21.00	.3684	01.61	4726	00/1-	.5203	.5789	.6315	1689.	.7368	1684	.842	.947	1.053	1.158	1.263	1.308	I.474	1.579	1.684	1.789
14		3-16.	lbs.	2030	69/0.	011	1072	2368	.2763	2158	25.50	-000-	.594/	.+3+2	.4736	.5131	.5526	.5920	.632	.710	.789	808.	.947	1.026	1.105	1.184	I 263	1.342
		8-H	lbs.	0.00	0250.	10:01	1316	1579	.1842	20105	2368	2621	1502.	+607.	.3158	.3421	.302+	.3947	.421	-474	.520	.579	.632	120.	.737	.789	.842	\$68.
		91-1	lbs.	0262	.0395	9020	8,500	.0789	.0921	.1053	1181	1216	20101	/+:-1.	1579	1710	1042	.1973	.210	.237	.203	5200	.316	.342	-368	.395	.421	-447
	isənə tay qay		91-1	71	3-16	17	91-5	100	7-16	7%	91-0	35	73	01-11	10	13-10	00/	15-10	-	1 18	**	78	1/2	38	は	1 %	(1)	100

FLAT BAR IRON-Continued.

ui d	ıbiW İəni	2 2 2 3 %	2000	$\omega \omega $	4 4 4 4 74 74 74	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
	-	lbs. 7.578 7 999	8.425 8.841 9.262 9.683	10.104 10.946 11.788 12.630	13.472 14.314 15.156 15.998	16.840 17.682 18.524 19.366 20.208
	3/8	1bs. 6.831 6.999	7.368 7.736 8.104 8.473	8.841 9.578 10.315 11.051	11.788 12.525 13.262 13.998	14 735 15.472 16.209 16.946 17.682
	34	lbs. 5.684 5.999	6.315 6.631 6.947 7.262	7 578 8.210 8.841 9.473	10.104 10.736 11.367 11.999	12.630 13.262 13.893 14.525
	3%	lbs. 4.736 4.999	5.263 5.526 5.789 6.052	6.315 6.841 7.368 7.894	8.420 8.946 9.473 9.999	10.525 11.051 11.578 12.104
	2/1	1b3. 3.789 4.000	4.210 4.42 4.631 . 4.842		6.736 7.157 7.578 7.999	8.420 8.841 9.262 9.683 10.104
in inches.	7-16	lbs. 3.315 3.500	3.684 3.868 4.052 4.236	4.421 4.789 5.157 5.526	5.894 6.262 6.631 6.999	7.368 7.736 8.104 8.473 8.473
Thickness in inches,	3%	1bs. 2.842 3 000	3.158 3.315 3.473 3.631	3.789 4.105 4.421 4.736	5.052 5.368 5.684 5.999	6.315 6.631 6.947 7.262 7.578
	91-5	1bs. 2.368 2.500	2.631 2.763 2.894 3.026	3.158 3.421 3.684 3.947	4.210 4.473 4.736 4.999	5.263 5.526 5.789 6.052 6.315
	74	lbs. I 895 2.000	2.105 2.210 2.316 2.421	2.526 2.737 2.947 3.158	3.368 3.579 3.789 4.000	4.210 4.421 4.631 4.842 5.052
	3-16	lbs. I.421 I.500	1.579 1.658 1.737 1.816	1.895 2.052 2.210 2.368	2.526 2.684 2.842 3.000	3.158 3.315 3.473 3.631 5.789
	8/	.947 1.000	1.053 1.105 1.158 1.210	1.263 1.368 1.474 1.579	1 684 1.789 1.895 2.000	2.105 2.210 2.316 2.421 2.526
	91-1	.474 .500	6573 605 505 505 505	.632 .684 .737 .789	.842 .895 .947 I.000	1.053 1.105 1.158 1.210 1.263
ni dil		11 11	1000	12 10 10 10 10 10 10 10 10 10 10 10 10 10	4444	2 12 12 12 0 7 7 16 14

17474

13.893 14.525 15.156

11.578 12.104 :2.630

9.202 9.683 10.104

8.473 8.841.

7.262

6.052

5.052

3.631

2.526

1.263 I.263

.:	Weight in lbs.	14.50 16.00 17.66 20.83 24.17 28.33 32.50 38.33 1 to be made Neight in 1bs. 36.375 36.375 36.375 36.375 36.375 36.375 36.375 36.375 36.375 36.375 36.375 36.375 36.375 36.375 36.375 36.375 36.375
EAL FOOT	Diam. of Link in in.	
OF A LIN	Weight ir. lbs.	5.33 14 6.16 15-1 7.16 13-8 8.16 13-8 10.50 13-8 11.83 17-8 13.16 2 diameter are assume B. W. Width Gauge. in ins 15 15 13-8 15 13-8 16 17 11-8 17 11-8 18 11-8 19 8-8
CHAIN S \VEIGHT OF A LINEAL FOOT.	Diam. of Link in in.	34 * 15 16 15 16 15 16 17 16 17 16 17 16 17 16 17 16 17 17 16 17 17 17 17 17 17 17 17 17 17 17 17 17
HAIN S	Weight in lbs.	5-16 .63   .63   .91   .91   .92   .92   .92   .93   .93   .94   .95   .94   .95   .94   .95   .
Ö	Diam. of Link in in.	5-16 3/8 7-16 3/8 9-16 5/8 11-16 Nore.—TI with studs. HOOI B. W. Gauge. 11 11 12 13 13 14
FOOT.	Square in lbs.	8.672 9.410 10.177 11.803 13.550 15.417 17.404 19.512 21.740 24.089 26.558 29.146 31.856 34.688 37.638 47.214 67.10 47.214 50.646 55.646
LINEAL	Round in Ibs.	6.811 7.390 7.993 9.270 10.642 12.168 13.668 15.325 17.075 18.916 22.891 27.244 27.244 27.244 27.244 27.244 27.244 27.268 31.972 31.972 33.777 43.550 48.5550
THE WEIGHT OF A LINEAL FOOT.	Diam. or Side in in.	11-11-12-13-13-13-13-13-13-13-13-13-13-13-13-13-
Тне Wе	Square in Ibs.	
Copper Bar.—	Round in lbs.	.047 .189 .296 .296 .579 .577 .958 1.182 1.182 1.198 2.318 2.318 2.318 2.318 2.318 2.318 2.318 3.027 3.417 3.821 4.269 4.730 6.255 6.255
COPPE	Diam. or Side in in.	3-16 3-16 3-16 3-16 3-16 3-16 3-16 3-16 3-16 3-16 13-16 1 1-16 1 1

COPPER PIPES —THE WEIGHT OF A LINEAL FOOT.

			Thickness	of Metal	in parts of	an inch.		
Bore in inches.	1-16	1/8	3-16	1/4	5-16	3/8	7-16	1/2
	lbs.	lbs.	lbs.	lbs.	lbs.	lbs.	lbs.	lbs.
3-16	.189	473	.851	I 324	1.892	2.224	3.311	4.162
1/4	.236	.268	<b>.</b> 993	1.214	5,158	2.838	3.642	4.241
5-16	*284	662	1.132	1.403	2.362	3.151	3.973	4.018
3/8	.331	757	1.522	1,805	1.69.2	3 406	4.304	<b>5</b> °297
7-16	.378	851	1'419	2'081	2.838	3.689	4.635	5.675
1/2	.426	.946	1.261	2'270	3.072	3'973	4.966	6.054
9-16	473	1.040	1.403	2.459	3,311	4.256	5.297	6.432
5/8	.20	1.182	1.845	2.649	3.547	4.240	5.629	6.811
11-16	.568	1.530	1.986	2.838	3.483	4.824	5.959	7.190
3/4	615	1.324	2,150	3'027	4.020	5.108	6.290	7°568
13.16	.662	1.419	2.51	3.516	4.257	5.395	6.622	7.946
7/8	.709	1.214	2.412	3.402	4.493	5.676	6.923	8.324
15-16	757	1.608	2.224	3'594	4.729	5.060	7.284	8.403
I	.804	1.403	2.696	3.784	4.966	6.243	7.615	0.081
11/4	'993	2.081	3.263	4.240	5.015	7:378	8.938	10.202
1 1/2	1,185	2.459	3.831	5'297	6.857	8.214	10.264	12,102
1 34	1.372	2.833	4.398	6.022	7.805	9.646	11.286	13.621
2	1.260	3'217	4.967	6.808	8.748	10.783	12,011	15.135
21/4	1.750	3.201	5.231	7.266	9.694	11'918	14.234	15.647
2 1/2	1.040	3.975	6.103	8.327	10.643	13.566	15.262	18.162
2 3/4	2.138	4.352	6.668	0.081	11'590	14.100	16.886	19.677
3	2,319	4.729	7.238	9.737	12.234	15.322	18'212	21'190

Weight per foot run in lbs. = 3'027 D2 - D'2, D and D' being the external and internal diameter in inches.

BRASS PIPES —THE WEIGHT OF A LINEAL FOOT.

		Thickness of Metal in parts of an inch.												
Bore in inches.	1-16	1/8	3-16	1/4	5-16	3/8	7-16	1/2						
	lbs.	lbs.	lbs.	lbs.	lbs.	lbs.	lbs.	lbs.						
3-16	178	'447	.802	1.222	1.490	2'428	3.135	3.937						
14	.536	537	<b>°</b> 940	I'432	2'015	2.684	3'445	4'295						
5-16	<b>.</b> 269	·626	1'074	1.911	2.260	2'953	3.758	4.653						
3/8	.311	714	1.500	1 790	2'459	3.510	4'069	5'009						
7-16	357	.802	1'342	1981	2.684	3.489	4.384	5.369						
1/2	'403	.895	1.478	2'148	2 908	3.758	4.698	5'727						
9 16	'447	'985	1.623	2'327	3'132	4'027	5'012	6'085						
58	492	1'076	1.745	2.206	3'356	4 295	5'324	6.445						
11-16	537	1'176	1 880	2.684	3.579	4.564	5.637	6.801						
34	.584	1'253	2'013	2.863	3.803	4.832	5'953	7.179						
13-16	.638	1'342	2'147	3'042	4'027	5'100	6'264	7616						
7/8	(6)	1'430	2'280	3.219	4'248	5.369	6.505	7'922						
15-16	704	1.200	2'404	3.388	4.462	5.625	6.888	8.227						
I	761	1.611	2'550	3'579	4'700	5'926	72.23	8.590						
1 1/8	850	1'790	2.819	3 939	5.162	6.493	7.830	9.308						

BRASS PIPES.--THE WEIGHT OF A LINEAL FOOT.-continued.

1/2 1/54 1/54 1/54 1/54 1/54 1/54 1/54 1/56 1

9.081

10 595

13.621 15.135 16.647 18.165 19.677 21.190

lbs. 3'937 4'295 4'653 5'009 5'727 6'085 6.445 6'801 7'179 7'616 7'922 8'227

8.590

9.308

		Thickness of Metal in parts of an inch.											
Bore in inches.	1-16	1/8	3-16	1/4	5-16	3/8	7-16	1/2					
	lbs.	lbs.	lbs.	lbs-	lbs	lbs.	lbs.	lbs.					
1 1/4	'940	1,060	3.089	4.312	5.643	6.98c	8.458	10.022					
13/8	1,050	2'150	3 376	4.703	6.040	7.219	9 082	10.238					
I 1/2	1,151	2.314	3.674	5.011	6'489	8.053	9.709	11'454					
1 5/8	1.556	2.224	3.890	5.369	6.933	8.288	10'333	12'168					
134	1.322	2.664	4'143	5.706	7:362	9.107	IC'942	15.89					
17/8	1.337	2.815	4'379	6.035	7.780	9614	11.238	13'553					
2	1.478	3.042	4.698	6.443	8.277	10 201	12,519	14'317					
2 1/4	1.655	3.400	5.235	7.159	9.174	11.276	13.467	15.749					
2 1/2	1.833	3.758	5.774	7 874	10.067	12:349	14.722	17.181					
23/4	2.012	4.116	6.300	8.290	10'964	13'422	15'973	18.812					
3	2'192	4.474	6.847	9:306	11.856	14.696	17.225	20'044					

Weight per foot run in lbs. = 2.8634 D2 - D'2, D and D' being the external and internal diameter in inches.

LEAD PIPES.—THE WEIGHT OF A LINEAL FOOT.

		Thick	ness of Metal	in parts of an	inch.	
Bore in inches.	1-16	1/8	3-16	1/4	5-16	3/8
	lbs.	lbs.	lbs.	lbs.	lbs.	lbs.
3-16 .	*243	.607	1.095	1.699	2.427	3.27
1/4 .	.303	.728	1.523	1.945	2.430	3.64
5-16	.364	.850	1.456	2.184	3.034	4.00
3/8	425	971	1.638	2 427	3'337	4.366
7-16	'485	1'092	1.820	2.670	3.640	4.73
1/2	•546	1.514	2.013	2.013	3'944	5'097
9-16	.607	1'335	2,184	3.122	4.248	5.460
5/8	667	1.20	2,366	3.398	4.221	5.82
11-16	.728	1.578	2.248	3.641	4.853	6.180
3/4	'789	1.699	2.731	3.873	5.157	6.223
13-16	.851	1.820	2'913	4.126	5.461	6.017
7/8	.910	1'942	3.092	4.368	5.764	7'281
15-16	·971	2.063	3.276	4 611	6.067	7.646
1	1.035	2.184	3'457	4.854	6 371	8.000
1 1/4	1.274	2.670	4.186	5.825	7.585	9'466
1 1/2	1.212	3'155	4.012	6.796	8.796	10'923
1 34	1760	3.641	5 642	7.768	10'013	12:375
2	2.001	4.127	6.372	8.734	11'223	13.833
21/4	2.245	4 607	7.096	9.707	12.436	15'290
21/2	2.489	5,100	7.829	10.683	13.654	16.762
234	2.729	5.283	8.554	11.650	14.869	18.204
3	2.071	6.066	9.286	12.621	16.080	19.660

Weight per foot run = 3.8834 D<sub>2</sub> - D'<sub>2</sub>, D and D' being the external and internal diameter in inches.

#### THE WEAR AND TEAR OF BUILDING MATERIALS.

At the tenth annual meeting of the Fire Underwriters' Association of the Northwest, held at Chicago in September, 1879, Mr. A. W. Spalding read a paper on the wear and tear of building materials, and tabulated the result of his investigations in the following form:

	Frame d	lwelling.		welling, e roof.)	Frame	store.	Brick store, (shingle roof.)		
MATERIAL IN BUILDING.	Average life. Years.	Per cent, of depreciation per annum,	Average life. Years,	Per cent, of depreciation per annum.	Average life. Years.	Per cent, of depreciation per annum,	Average life. Years.	. Per cent. of depreciation per annum.	
Brick	20 5 7 16 40 30 50 20 30 30 30 40 30 20 20 16	5 20 14 6 2 ½ ½ 3 2 5 ½ ½ 3 ½ ½ 3 5 5 6 4 2	75 30 7 7 10 40 50 20 30 30 30 40 30 20 20 16	11/8 31/3 14 14 6 21/2 2 5 31/3 31/3 31/3 31/3 31/3 31/3 31/3 31	16 5 5 16 30 30 40 13 25 25 20 30 30 13 20 16	- 6 20 20 6 33以2 8 4 4 5 5 3 3 8 5 6 4	66 30 6 6 6 16 17 50 13 30 30 30 30 30 30 13 20 16	1½ 3½ 16 16 6 2½ 2 8 3½ 3½ 3½ 3½ 3½ 3½ 3½ 3½ 3½ 3½ 3½ 3½ 3½	

These figures represent the averages deduced from the replies made by eighty-three competent builders unconnected with fire-insurance companies, in twenty-seven cities and towns of the eleven Western States.

#### SASH WEIGHTS

REQUIRED FOR THE FOLLOWING SIZED WINDOWS:

SIZE OF GLASS.	THICKNESS.	NO. OF LIGHTS.	WEIGHT-LBS.
10 x 14	13/8	12	6
10 x 16	"	8	6
12 x 14	"	8 8	6
10 x 15	""	12	7
10 x 16	"	12	7
10 x 18	"	8	7 *
10 x 10	"	8	7 5
10 X 20	"	8	7 0
12 x 15	"	8	7 .1₹
12 x 16	"	8 8 8 8	7 4
14 x 10 10 x 18	46	12	OOSE.—Four wei, hts required for each window.
10 X 10	"	8	8 5
12 x 18	.6	8 8 8 8	Ž, Š
12 X 20		8	g g .
12 X 22	"	9	δ <del>μ</del>
14 x 18	"	0 0	o a int
14 x 20	"	1	őğ
12 x 36	"	4	o N
10 X 20	"	12	9 H
12 x 24	. "	8 8	9 .5
15 x 22	"		9 🕉
12 x 40	"	4	9 =
12 × 44	1	4	9 ,5
14 x 36	46	4	9 🛱
IO X 22	"	12	10
10 x 24	"	12	IO 🛱
12 x 18	"	12	10 💆
12 X 20	į "	12	
I4 X 22	"	8 8	10
14 x 24			10
14 x 40	(6	4 i	IO
14 x 44	6	4	IO
12 x 22	"	12	II
15 x 48	134	4	12

## STAIRCASES-PROPORTION OF TREADS AND RISERS.

WIDTH OF TREAD.	HEIGHT OF RISER.	WIDTH OF TREAD.	HEIGHT OF RISER.
6 inches 7 " 8 " 9 "	8½ inches. 8 "1 7½ "1 7 "1 6½ "1	11 inches 12 !! 13 !! 14 !!	6 inches 5½ " 5 " 4½ " 4 "

LS. ation of the

ding read a result of his

Brick store, shingle roof.)

Per cent. of depreciation per annum. 66 1½ 3½ 16 30 6 6 16 16 6 40 50 13 30 30 21/2 8 31/3

3/3 31/3 5 31/2 31/3 8 20 30 30 5 20 16 3½ 1½ 30

s made by companies,

56

# **TABLES**

Showing Amount of a Workman's Wages from 15 Cents to 45½ Cents per Hour,

FOR ANY NUMBER OF HOURS,

FROM 1 TO 120 (A FORTNIGHT).

A.T.	AT 15 CENTS PER HOUR. AT 15½ CENTS PER HOUR.											
AT	15 CENT	S PER HO	JUR.	AT	15½ CENT   Hrs.   Am't.	S PER H	UUR.					
1115. An t.		Am t.		TIIS. Alli t.	Am t.	Tils. Au t	TIIS. AIII C.					
	302 4.57	601 9.07	901 13.57		301 4.73	601 9.38	901 14.03					
	$\begin{vmatrix} 31 & 4.65 \\ 31\frac{1}{2} & 4.72 \end{vmatrix}$	61 9.15	91 13.65 91 13.72		31 4.80 31½ 4.88	61 9.45 611 9.53	91 14.10					
2 0.30	32 4.80	62 9.30	92 13.80	2 0.31	32 4.96	62 9.61	92 14.26					
21 0.37	$32\frac{1}{2}$ 4.87	$62\frac{1}{2}$ 9.37	921 13.87	21 0.39	322 5.04	621 9.69	922 14.34					
$\begin{array}{c c} 3 & 0.45 \\ \hline 3\frac{1}{2} & 0.52 \end{array}$	$\begin{vmatrix} 33 & 4.95 \\ 33\frac{1}{2} & 5.02 \end{vmatrix}$	$63   9.45   63\frac{1}{2}   9.52$	93 13.95		33 5.11	63 9.76 634 9.84	93 14.41 932 14.49					
4 0.00	34 5.10	64 9.60	932 14.02	3 <sup>1</sup> 0.54 4 0.62	33½ 5.19 34 5.27	64 9.92	94 14.57					
42 0.67	342 5.17	$64\frac{1}{2}$ 9.67	942 14.17	42 0.70	342 5.35	642 10.00	$94\frac{1}{2}$ 14.65					
5 0.75 51 0.82	35 5.25 352 5.32	65 9.75 65½ 9.82	95 14.25 95 14.32	5 0.77 5 0.85	35 <sub>1</sub> 5.42 35 <sub>2</sub> 5.50	65 10.07	95 14.72 95½ 14.80					
6 0.90	36 5.40	66 9 90	96 14.40	$\begin{array}{c c} 5^{\frac{1}{2}} & 0.85 \\ 6 & 0.93 \end{array}$	35½ 5.50 36 5.58	66 10.23	96 14.88					
65 0.97	361 5.47	$66\frac{1}{2}$ 9.97	$96\frac{1}{2}$ 14.47	61 1.01	$36\frac{1}{2}$ 5.66	661 10.31	$96\frac{1}{2}$ 14.96					
$7   1.05  $ $7\frac{1}{2}   1.12  $	37 5.55 372 5.62	67 10.05	$97$ 14.55 $97\frac{1}{2}$ 14.62	7 1.08 71 1.16	37 5.73 37 5.81	67 10.38	97 15.03					
8 1.20	38 5.70	68 10.20	98 14.70	1.2	38   5.89	68 10.54	98 15.19					
81/2 1.27	382 5.77	$68\frac{1}{2}$ 10.27	$98\frac{1}{2}$ 14.77	81 1.32	$38\frac{1}{2}$ 5.97	68½ 10.62	$98\frac{1}{2}$ 15.27					
$9  1.35 \\ 9\frac{1}{2}  1.42$	39 5.85 39 5.92	69 10.35	99 14.85	9 1.39 91 1.47	39 6.04 39½ 6.12	69 10.69 69 <sup>1</sup> 10.77	99 15.34					
10 1.50	40 6.00	70 10.50	100 15.00	/ 4	40 6.20	70 10.85	100 15.50					
101 1.57	$40\frac{1}{2}$ 6.07	701 10.57	1002 15 07	102 1.63	402 6.28	701 10.93	100 1 15.58					
11 1.65	$41 6.15$ $41\frac{1}{2} 6.22$	71 10.65	101 15.15	11 1.70	41 6.35 41 6.43	71 11.00	101 15.66					
12 1.80	42 6.30	72 10.80	102 15.30	2	42 6.51	72 17.16	102 15.82					
121 1.87	$42\frac{1}{2}$ 6.37	$72\frac{1}{2}$ 10.87	102 2 15.77	$12\frac{1}{2}$ 1.94	$  42\frac{1}{2}  6.59  $	725 11.24	1021 15.90					
13 1.95 13 <sup>1</sup> / <sub>2</sub> 2.02	$\begin{vmatrix} 43 & 6.45 \\ 43\frac{1}{2} & 6.52 \end{vmatrix}$	73 10.95	103   15.45 103 <sup>1</sup> / <sub>2</sub> 15.52	13 2.01 13 <sup>1</sup> 2.09	43   6.66   432   6.74	73 11.31 732 11.39	103   15.97 $103\frac{1}{2}   16.05$					
14 2.10	44 6.60	74 11.10	104 15.60	14 2.17	44 6.82	74 11.47	104 16.13					
14½ 2.17 15 2.25	$44\frac{1}{2} 6.67$	742 11.17	1043 15.67	142 2.25	$  44\frac{1}{2}   6.90  $	74½ 11.55	104½ 16.21 105 16.28					
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	45 6.75 452 6.82	$75   11.25   75\frac{1}{2}   11.32$	105 15.75	$\begin{array}{c c} 15 & 2.32 \\ 15\frac{1}{2} & 2.40 \end{array}$	45 6.97 452 7.05	75 11.62	105 16.28 1052 16.36					
16 2.40	46 6.90	76 11.40	106 15.90	16 2.48	46 7.13	76 11.78	106 16.44					
$\begin{array}{c c} 16\frac{1}{2} & 2.47 \\ 17 & 2.55 \end{array}$	$  46\frac{1}{2}   6.97$ 47 7.05	76111.47	106½ 15.97	$-6\frac{1}{2}$ 2.56	462 7.21	76111.86	106½ 16.52 107 16.59					
172 2.62	47 7.05 472 7.12	77 111.55	107 16.13	17 2.03 17 <sup>1</sup> 2.71	47 7.28 47½ 7.36	772 12.01	1073 16.66					
18 2.70	48 7.20	78 11.70	108 16.20	18 2.79	48 7.44	78 12.09	108 16.74					
18½ 2.77	$\begin{vmatrix} 48\frac{1}{2} & 7.27 \\ 49 & 7.35 \end{vmatrix}$	78½ 11.77	108 16.27	~ 0 0 4	48½ 7.52	$78\frac{1}{2}$ 12.17	108\frac{1}{2} 16.82					
192 2,92	492 7.42	79111.92	109 16.42	~ 7	49 7·59 492 7·67	792 12.32	1092 16.97					
20 3.00	50 7.50	80 12.00	110 16.50	20 3.10	50 7.75	80 12.40	110 17.05					
20½ 3.07	50½ 7.57 51 7.65	81 12.15	110 16.57	20½ 3.18 21 3.25	$50\frac{1}{2}$ 7.83 $7.90$	80 12.48	1102 17.13					
212 3.22	512 7.72	811 12.22	1111 16.72		51 7.98	811 12.63	1112 17.29					
22 3.30 225 3.37	52 7.80	82 12.30	112 16.80	4	52 8.06	82 12.71	112 17.37					
<sup>22</sup> 2 3·37 <sup>23</sup> 3·45	52½ 7.87 53 7.95	82½ 12.37 83 12.45	112 16.87		52 8.14 53 8.21	82½ 12.79 83 12.86	$112\frac{1}{2}$ 17.45					
232 3.52	532 8.02	83 12.52	1132 17.02	231 3.04	53 8.29	832 12.94	1132 17.60					
24 3.60 242 3.67	54 8.10 542 8.17	84 12.60	114 17.10	24 3.72	1 54 8.37	84 13.02	114 17.68					
<sup>25</sup> 3.75	54½ 8.17 55 8.25		1145 17.17		348 3.43	84½ 13.10 85 13.17	$114\frac{1}{2}$ 17.76					
252 3.82	552 8.33	851 12.82	1152 17.32	253 3.95	55 8.60	852 13.25	115 17.91					
26 3.90 26½ 3.98	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	86 12.90	116 17.40	26 4.03	56 8.68	86 13.33 862 13.41	116 17.99					
27 4.05	56½ 8.47 57 8.55	87 13.05	116½ 17.47 117 17.55	26½ 4.11 27 4.18	56 8.76 57 8.83	87 13.48	1102 18.13					
27½ 4.12	575 8.62	872 13.13	1172 17.62	272 4.26	572 8.91	879 13.56	1172 18.21					
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	58 8.70 581 8.77	88 13.20	118 17.70 1181 17.77	28 4.34 281 4.42	58 8.99	88 13.64 88 13.72	118 18.29					
29 4.35	59 8.85	89 13.35	1102 17.77		59. 9.14	89 13.79	119 18.44					
<sup>29</sup> 2 4.42 30 4.50	592 8.92	892 13.42	1192 17.92	292 4.57	592 9.92	892 13.87	119 185 2					
.5- 14.50	11 60 1 9.00	11 90  13.50	1 120  18.00	30   4.65	60   9.30	90 113.95	120 18.60					

Vage

His.

 $\begin{array}{c} \mathbf{2} \\ \mathbf{2} \\ \mathbf{3} \\ \mathbf{1} \\ \mathbf{3} \\ \mathbf{3} \\ \mathbf{3} \\ \mathbf{4} \\ \mathbf{4} \\ \mathbf{2} \\ \mathbf{5} \\ \mathbf{5} \\ \mathbf{6} \\ \mathbf{6} \\ \mathbf{6} \\ \mathbf{6} \\ \mathbf{7} \\ \mathbf{7} \\ \mathbf{12} \\ \mathbf{13} \\ \mathbf{13} \\ \mathbf{13} \\ \mathbf{14} \\ \mathbf{12} \\ \mathbf{13} \\ \mathbf{14} \\ \mathbf{12} \\ \mathbf{13} \\ \mathbf{14} \\ \mathbf{14} \\ \mathbf{12} \\ \mathbf{13} \\ \mathbf{15} \\ \mathbf{15} \\ \mathbf{15} \\ \mathbf{16} \\ \mathbf{17} \\ \mathbf{17} \\ \mathbf{18} \\ \mathbf{18} \\ \mathbf{19} \\ \mathbf$ 

			S PER HO		AT 16½ CENTS PER HOUR.								
Hrs.	Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.					
		301 4.88	601 9.68	902 14.48		301 5.03	$60\frac{1}{2}$ 9.98	901 14.93					
		31 4.96 31½ 5.04	61 9.76	91 14.56		31 5.11	61 10.06	91 15.01 91 15.10					
2	0.32	31½ 5.04	$61\frac{1}{2}$ 9.84 62 9.92	91½ 14.64 92 14.72	2 0.33	$31\frac{1}{2}$ 5.20 $32$ 5.28	62 10.23	92 15.18					
$2_2^1$	0.40	$32\frac{1}{2}$ 5.20	621 10.00	923 14.80	$2\frac{1}{2}$ 0.41	321 5.36	62 10.31	921 15.26					
3,	0.48	33 5.28	63 10.08	93 14.88	3, 0.49	33 5.44	63 10.39	93 15.34					
32 4	0.56	33½ 5.36 34 5.44	631 10.16	931 14.96	$3\frac{1}{2}$ 0.58 0.66	332 5.53	63½ 10.48	935 15.43					
$\frac{4}{4\frac{1}{2}}$	0.72	34 5.44 34½ 5.52	64 10.24	94 15.04 $94\frac{1}{2}15.12$	$\frac{4}{4^{\frac{1}{2}}} = 0.00$	34 5.61 34½ 5.69	64 10.56	94 15.51					
5	0.80	35 5.60	65 10.40	95 15.20	5 0.82	35 5.77	65 10.72	95 15.67					
$\frac{5\frac{1}{2}}{6}$	0.88	$ 35\frac{1}{2} 5.68$	65½ 10.48	$95\frac{1}{2}$ 15.28	$5\frac{1}{2}$ 0.91	$35\frac{1}{2}$ 5.86	651 10.81	952 15.76					
$\frac{61}{2}$	0.96	36 5.76 36½ 5.84	66 10.56	96 15.36 96 15.44	6 0.99 61 1.07	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	66 10.89	96 15.84					
7	1.12	37 5.92	67 10.72	97 15.52	7 1.15	37 6.10	67 11.05	97 16.00					
$7\frac{1}{2}$	1.20	371 6.00	671 10.80	973 15.60	$7\frac{1}{2}$ 1.24	37 6.19	672 11.14	971 16.09					
8 8 <del>1</del>	1.28	38   6.08   38½   6.16	68 10.88	98 15.68	8 1.32	38 6.27	68 11.22 68½ 11.30	98 16.17					
9	I.44	39 6.24	681 10.96	98½ 15.76 99 15.84	8½ 1.40 9 1.48	38½ 6.35 39 6.43	69 11.38	98½ 16.25 99 16.33					
$9\frac{1}{2}$	1.52	$39\frac{1}{2}   6.32  $	69111.12	$99^{13.04}$ $99^{1}_{2}$ $15.92$	9 1.41 9½ 1.57	391 6.52	692 11.47	99 16.42					
10	1.60	40 6.40	70 11.20	100 16.00	10 1.65	40   6.60	70 11.55	100 16.50					
10½	1.68	40½ 6.48 41 6.56		100½ 16.08 101   16.16	10½ 1.73 11 1.81	40½ 6.68 41 6.76	70½ 11.63 71 11.71	100 16.58					
$\mathbf{II}_{2}^{1}$	1.84	41 6.64		101   16.16 101   16.24	11 1.81	41   6.76   41½   6.85		101 16.75					
12	1.92	42 6.72		102 16.32	41 0	42 6.93	72 11.88	102 16.83					
122	2.00	421 6.80		1021 16.40	121 2.06	$42\frac{1}{2}$ 7.01	11	102 16.91					
13 13½	2.08 2.16	43 6.88 43½ 6.96		103   16.48   1033   16.56	13 2.14 13 <sup>1</sup> 2.23	43   7.09     43½   7.18	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	103 16.99 103½ 17.08					
14	2.24	44 7.04		104 16.64	$13\frac{1}{2}$ 2.23 $2.3^{\circ}$	43½ 7.18 44 7.26	74 12.21	104 17.16					
142	2.32	$ 44\frac{1}{2} 7.12 $	742 11.92	1042 16.72	141 2.39	$44\frac{1}{2}$ 7.34	$74\frac{1}{2}$ 12.29	1041 17.24					
15 15½	2.40 2.48	45 7.20		105 16.80	15 2.47	45, 7.42		105 17.32					
16	2.56	45½ 7.28 46 7.36		105\frac{1}{2} 16.88	$15\frac{1}{2}$ 2.56 $16$ 2.64	2 7.51 59 7.59		105½ 17.41 106 17.49					
$16^{1}_{2}$	2.64	462 7.44		1061 17.04	$16\frac{1}{2}$ 2.72	462 7.67		1061 17.57					
17	2.72	47, 7.52		107 17.12	17 2.80	47 7.75		107 17.65					
17½ 18	2.80	$\begin{vmatrix} 47\frac{1}{2} \\ 48 \end{vmatrix} \begin{vmatrix} 7.60 \\ 7.68 \end{vmatrix}$		$107\frac{1}{2}$ 17.20	$17\frac{1}{2}$ 2.89	472 7.84		1071 17.74					
187	2.96	48½ 7.76		108   17.28 1081   17.36	18 2.97 18½ 3.05	48 7.92 481 8.00		108 17.82					
19,	3.04	49 7.84		109 17.44	19 3.13	49 8.08		109 17.98					
19½	3.12	492 7.92		1092 17.52	$19\frac{1}{2}$ 3.22	$ 49\frac{1}{2} 8.17 $		1092 18.07					
20 20 1	3.20	50   8.00   50½   8.08	1 - 41 11	110  17.60 1101  17.68	20 3.30 20½ 3.38	50 8.25		110 18.15					
21	3.36	51 8.16	11	111 17.76	20½ 3.38 21 3.46	$50\frac{1}{2}$ 8.33 $51$ 8.41	_ =   -	111 18.31					
$21\frac{1}{2}$	3.44	$51\frac{1}{2}$ 8.24	811 13.04	1111 17.84	215 3.55	512 8.50	812 13.45	1112 18.40					
$22 \frac{1}{2}$	3.52	$\begin{bmatrix} 52 & 8.32 \\ 52 & 8.40 \end{bmatrix}$		112 17.92	22 3.63	52 8.58		112 18.48					
23	3.68	52½ 8.40 53 8.48	1 0 0 0 0 11	112½ 18.00 113   18.08	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	52½ 8.66 53 8.74		112½ 18 56 113 18.64					
232	3.76	$53\frac{1}{2}$ 8.56		113 18.16	231 3.88	53 8.74 532 8.83	831 13.78	113 18.73					
24	3.84	54 8.64	84 13.44	114 18.24	<b>24</b> 3.9 <sup>6</sup>	54 8.91	84 [13.86]	114   18.81					
24½ 25	3.92	54½ 8.72 55 8.80	84½ 13.52 85 13.60	$114\frac{1}{2}$ $18.32$	245 4.04	542 8.99	842 13.94	1142 18.89					
$25\frac{1}{2}$	4.08	55\frac{1}{2} 8.83	85 13.60 85 13.68	115   18.40 115½   18.48	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	55 <sub>1</sub> 9.07   55 <sub>2</sub> 9.16	85 14.02 85 14.11	115 18.97 1151 19.06					
26	4.16	56 8.96	86 13.76	116   18.56	26 4.29	56 9.24	86 14.19	116 19.14					
26½ 27	4.24	$56\frac{1}{2}$ 9.04		1162 18.64	$26\frac{1}{2}$ 4.37	562 9.32	862 14.27	1161 19.22					
271	4.40	57 9.12 572 9.20		117 18.72 1171 18.80	27 4.45	57 9.40	87 14.35	117 19.30					
28	4.48	58 9.28		1172 10.00	$27\frac{1}{2}$ 4.54 $4.62$	572 9.49 58 9.57	87 <sup>1</sup> / <sub>2</sub> 14.44 88 14.52	118 19.47					
28½	4.56	$58\frac{1}{2}$ 9.36	881 14.16	1181 18.96	281 4.70	582 9.65	881 14.60	118 19.55					
29	4.64	59 9.44		119 19.04	29 4.78	59 9.73	89 14.68	119 19.63					
30	4.80	59½ 9.52 60 9.60	89½ 14.32 90 14.40	$119\frac{1}{2}$ 19.12	29½ 4.87 30 4.95	592 9.82 60 9.90	89½ 14.77 90 14.85	1195 19.72					
		9.00	30 (24,401)	119,20	30   4.95	60   9.90	90 [14.05]	120 119:00					

	UR.	AT 17 CENTS PER HOUR.			.	AT 17½ CENTS PER HOUR.  His. Am't.    Hrs.    Am't.    Am't.												
1.	Hrs. Am't.	I	Its.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	His.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.
8	901 14.93				301	5.18	601	10.28	905	15.38			302	5.34	60.	10.59	903	15.84
6	91 15.01		1		31	5.27	61	10.37	91	15.47			31	5.42	61	10.67	91	15.92
5	91½ 15.10 92 15.18		2	0.34	31½ 32	5·35 5·44	62	10.45	915	15.55 15.64	2	0.35	$31\frac{1}{2}$ 32	5.51 5.60	62	10.76	915	16.01
I	921 15.26	8		0.42	321	5.52		10.62	- 1	15.72	21/2	0.44	$32\frac{1}{2}$	5.69	621	10.94	925	16.19
9	93 15.34		3	0.51	33,	5.61		10.71	93	15.81	3	0.52	33.	5.77		11.02	93	16.27
6	932 15.43		3½ 4	0.59	33½ 34	5.69 5.78		10.79 10.88	93½ 94	15.89 15.98	3½ 4	0.70	33½ 34	5.86	63½	11.11		16.36 16.45
4	942 15.59	1.	42	0.76	$34\frac{1}{2}$	5.86		10.96		16.06		0.79	342	6.04	1 - 1 - 1	11.29	941	16.54
2	95 15.67		5,	0.85	35,	5.95		11.05	95	16.15	5.	0.17	35.	6.12		11.37	95	16.62
1 9	95½ 15.76 96 15.84		52	0.93	35½ 36	6.12	.02	11.13		16.23 16.32	$\frac{5^{\frac{1}{2}}}{6}$	0.90	35½ 36	6.21		11.46		16.71 16.80
7	961 15.92		$6^{1}_{2}$	1.10	36½	6.20		11.30		16.40	$6\frac{1}{2}$	1.14	361	6.39		11.64		16.89
5	97 16.00		7	1.19	37	6.29		11.39	97,	16.49		1.22	37,	6.47	67	11.72	97	16.97
4	97 16.09	86	$7\frac{1}{2}$	1.36	37½ 38	6.46		11.47		16.57 16.66	7½ 8	1.31	37½ 38	6.56		11.81		17.06 17.15
o	981 16.25		81	1.44	381	6.54	$68\frac{1}{2}$	11.64	981	16.74	$8\frac{1}{2}$	1.49	$38\frac{1}{2}$	6.74	$68\frac{1}{2}$	11.99	981	17.24
8	99 16.33 99 16.42		9 9 5	1.53	39	6.63		11.73		16.83	9,	1.57 1.66	39,	6.82	1 1	12.07 12.16		17.32 17.41
7 5	100 16.50	**	10	1.70	39½ 40	6.80	70	11.90	100	16.91 17.00	9½ 10	1.75	39½ 40	6.91 7.00	70	12.25		17.50
3	1002 16.58		102	1.78	40½	6,88		11.98	1002	17.08	$10\frac{1}{2}$	1.84	$40\frac{1}{2}$	7.09		12.34	1 4	17.59
O	101 16.66	- 10	11	1.95	41 412	7.05	71	12.07		17.17 17.25	11	1.92 2.01	41 41 <sup>1</sup> / <sub>5</sub>	7.17		12.42	101	17.67
88	102 16.83		12	2.04	42	7.14	72	12.24	1	17.34	112	2.10	412	7 35	72	12.60	102	
16	1021 16.91	4.	$12\frac{1}{2}$	2.12	$42\frac{1}{2}$	7.22	721	12.32	$102\frac{1}{2}$	17.42		2.19	$42\frac{1}{2}$	7.44	1 ' -1	12.69		17.94
14	103 16.99 1031 17.08		13 13 <sup>1</sup> / <sub>2</sub>	2.21	43	7.31 7.39	1 7 7 1	12.41		17.51	13	2.27	43	7.5 <sup>2</sup> 7.61	73 <sub>1</sub>	12.77 12.86		18.02 18.11
3	104 17.16		14	2.38	43½ 44.	7.48	74	12.58		17.59 17.68	135	2.36 2.45	43½ 44	7.70	732	12.95		18.20
29	$104\frac{1}{2}$ 17.24		142	2.46	$44\frac{1}{2}$	7.56		12.66	1042	17.76	$14\frac{1}{2}$	2.54	44 ½	7.79		13.04		18.29
37 16	105 17.32 105 17.41		15	2.55	45 45 <sup>1</sup> / <sub>2</sub>	7.65 7.73	75	12.75		17.85	15 15 <sup>1</sup> / <sub>2</sub>	2.62 2.71	45 45 <sup>1</sup> / <sub>2</sub>	7.87 7.95	75	13.12		18.37 18.46
54	106 17.49	65	16	2.72	46	7.82	76	12.92		18.02	16	2.80	452	8.05	762	13.30		18.55
52	1061 17.57		165	2.80	$46\frac{1}{2}$	7.90				18.10	$16\frac{1}{2}$	2.89	$46\frac{1}{2}$	8.14		13.39		18.64
70 79	107 17.05		17 17½	2.89	$ 47 $ $47\frac{1}{2}$	7·99 8.07	$\frac{77}{77\frac{1}{2}}$	13.09		18.19 18.27	17 17 2	2.97 3.06	47 47 <sup>1</sup> <sub>2</sub>	8.22	77	13.47 13.56	1 1	18.72 18.81
3 <b>7</b>	108 17.82		18	3.06	48	8.16	78	13.26	108	18.36	182	3.15	48	8.40	78	13.65		18.90
95	1081 17.90		181	3.14	482	8.24		13.34		18.44	181	3.24	$48\frac{1}{2}$	8.49	. ~	13.74	1 -	18.99
03 : 12	109 17.98	40	191	3.23	49  $ 49 $	8.33 8.41		13.43	100	18.61	$19 \\ 19\frac{1}{2}$	3 3 <sup>2</sup>   3.41	49 49	8.57		13.82	1	19 <b>.07</b>
20	110 18.15		20	3.40	50	8.50	80	13.60	110	18.70	20	3.50	50	8.75	80	14.00	110	19.25
28 36	110 <sup>1</sup> 18.23		202	3·48 3·57	50½ 51	8.58 8.67		13.68		18.78 18.87	20½	3.59	502	8.84	80½	14.09	1105	19.34
45	111 18.40		$21\frac{1}{2}$	3.65	512	8.75		13.85		18.95	211	3.67 3.76	51 515	9.01	\	14.26	-	19.51
53	112 18.48		22	3.74	52	8.84	- 1	13.94	1 -1	19.04	22	3.85	52	9.10	82	14.35	112	19.60
61 69	112 <sup>1</sup> / <sub>2</sub> 18 56 113 18.64		$22\frac{1}{2}$	3.82 3.91	52½	8.92 9.01	822	14.02		19.12 19.21	22½ 23	3.94 4.02	52½	9.19 9.27		14.44 14.52	1122	19.69
78	113 18.73		232	3.99	532	9.09	831	14.19		19.29	232	4.11	53 532	9.36		14.61	1132	19.86
86	114 18.81	1	24	4.08	54,	9.18	84	14.28	114,	19.38	24	4.20	54	9.45	8.4	14.70	114	19.95
94	114 <sup>1</sup> / <sub>2</sub> 18.89 115, 18.97		21½ 25	4.16	54 <sup>2</sup> 55,		842	14.36	1142	19.40	24½ 25	4.29 4.37	54 55	9.54 9.62	85	14.79 14.87	1142	20,04
11	1152 19.00	3	251	4.33	553	9.43	851	14.53	1153	19.63	251	4.46	552	9.71	852	14.96	1152	20.21
19	116 19.14		26 26	4·42 4·50	56	9.52	86	14.62	116	19.72	26	4.55	56	9.80		15.05		20.30
27 35	116½ 19.22 117 19.30	75	27	4.59	56½	9.60 9.69		14.70		19.80	26½ 27	4.64 4.72	56½ 57	9 89 9 97	87	15.14 15.22	117	20.39
44	1172 19.39		272	4.67	572	9.77	875	14.87	1173	19.97	$27\frac{1}{2}$	4.81		10.06	875	15.31	117	20.56
52	118 19.47		28 281	4.76	58 58½	9.86	88	14.96	118	20.06	28	4.90	58	10.15		15.40		
.60 .68	118½ 19.55 119 19.63	B)	29	4.93		9.94	80	15.04	110	20.14	28½ 29	4.99 5.07	502	10.24	80	15.49 15.57	1102	20.82
.77	1195 19.72		597	5.01	$59^{\frac{1}{2}}$	10.11	895	15.21	1192	20.31	$29\frac{1}{2}$	5.16	592	10.41	895	15.66	1195	20.91
.85	120 19.80	-	30	5.10	00	10.20	90	15.30	120	20.40	30	5.25	60	10.50	90	15.75	120	21.00

Hrs. F

	AT 18 CENTS PER OUR.								AT 18½ CENTS PER HOUR. t. His.  Am't.  Hrs.  Am't.  Hrs.  Am't.  Hrs.  Am't.						
Hrs	Am't.	Hrs. An	t. Hrs	Am't	Hrs.	Am't.	His.	Am't.	Hrs.	Am't.	Hrs.	Am't	Hrs.	Am't	
-		30½ 5.	49 60	10.89	901	16.29			301	5.64	60 <sup>1</sup>	11.19	901	16.74	
		31 5.	58 61	10.98	91	16.38			31	5.73	61	11.28	91	16.83	
2	0.34	31½ 5. 32 5.	5.11	11.07	915	16.47		0.37	31 2	5.83	62	11.38	$91\frac{1}{2}$ $92$	16.93	
2.		321 5.		. 1		16.65	2.	0.46	32 32 1	6.01	623	11.56	925	17.11	
3	0.54	33 5.		11.34	93	16.74		0.55	33	6.10	63	11.65	93	17.20	
3:	0.63	33½ 6.0 34 6.		11.43	93½ 94	16.83	02	0.65	332	6.20	632	11.75	935	17.30 17.39	
4		341 6.	11		942		$\frac{4}{4\frac{1}{2}}$	0.83	$\frac{34}{34\frac{1}{2}}$	6,38		11.93		17.48	
5	0.30	35 0.3		11.70	95.	17.10	5.	0.92	35	6.47		12.02	95	17.57	
5.	0.99	35½ 6.3		11.79	$95\frac{1}{9}$	17.19 17.28	$\frac{5\frac{1}{2}}{6}$	I.02 I.11	35½ 36	6.66	2.4	12.12 12.21	952	17.07	
6	1.17	36½ 6.		11.97		17.37	$6\frac{1}{2}$	1.20	361	6.75		12,30	1	17.85	
7	1.26	37 6.0		12.06	\$7	17.40	7	1.29	37	6.84		12.39	97	17.94	
7½ 8	1.35	37 6.8 38 6.8	5 67	12.15	972	17.55	7½ 8	I.39 I.48	37½ 38	0.94 7.03		12.49 12.58		18.04	
8.1	1.53	381 6.9		12.33		17.73	81	1.57	381	7.12		12.67	983	18.22	
9 9	1.02	39 7.0		12.42	99	17.82	9,	1.66	39	7.21		12.76 12.86		18.31	
92 10	1.80	39½ <b>7.</b> 1		12.51	100	18.00	92	1.76	39½ 40	7.40	- 4	12,95		18.41	
102	1.89	402 7.2	9 70	12.69	1002	18.09	$10^{1}_{2}$	1.94	$40\frac{1}{2}$	7.49	$70\frac{1}{2}$	13.04	1003	18.59	
11 J	2.07	41 7.3		12.78	101	18. 18 18. 27	117	2.03	41	7.58 7.68		13.13	101	18.68	
12	2.16	47 2 7.4	5, 11 5 -	12.90	1012	18.36	12	2.13	$41\frac{1}{2}$ $42$	7.77		13.23 13.32		18.87	
122	2.25	422 7.6	5 72	13.05	102	18.45	$12\frac{1}{2}$	2.31	421	7.86	721	13.41	1021	18.96	
13 132	2.34	43 7.7 432 7.8		13.14		18.54	13	2,40	43	7·95 8.05		13.50		19.05	
14	2.52	44 7.9		13.32		18.72	14	2.59	43 <sup>1</sup> / <sub>2</sub>	8.14		13.69		19.24	
142	2.61	445 8,0	1 745	13.41		18.81	142	2,68	$44\frac{1}{2}$	8.23	745	13.78	1041		
15 15½	2.70	45 8.1 45 8.1	11 / 5	13.50		18.90 18.99	15 15 <sup>1</sup> / <sub>2</sub>	2.77	45 45	8.32		13.87		19.42	
16	2.88	46 8.2	8 76	13.68		19.08	16	2.96	46	8.51		14.06		19.61	
165	3.06	462 8.3		13.77		19.17	161	3.05	$46\frac{1}{2}$	8.60		14.15	. ~	19.70	
17	3.15	47   8.4 47   8.5		13.86		19.26	17 17 <sup>1</sup> / <sub>2</sub>	3.14	$\frac{47}{47\frac{1}{2}}$	8.69		14.24		19.79	
18	3.24	48   8.6	1 78	14.04		19.41	18	3.33	48	8.88		4.43		19.98	
181	3.42	48 8.7				19.53	183	3.42	481	8.97		14.52		20,07	
19		49 8.5	11 //	14.22		19.62	19	3.51	49	9.00	79	14.01	109	20.10	
20	3.60	50 9.0	o 80	14.40	110	19,80	20	3.70	50	9.25	80 1	4.80	110	20.35	
20 2	3.69 3.78	50½ 9.0 51 9.1	7. II ·	14.49		19.89 19.98	201	3.79	503	9.34	- 64	4.89	1101		
213		51 9.1 51 9.2	11	14.67		20.07	212	3.98	51	9.43		5.08		20,53 20,63 [	
22	3.96	52 9.3		14 76	1 .	20.16	22	4.07	52	9.62	82 1	5.17		20,72	
23	4.05	52½ 9.4 53. 9.5		14.85		20.25	224	4.25	521	9.71	825 1	5.35	6	20.00	
231	4.23	532 9.6				20.43	234	4.35	53 <sub>1</sub>	9.90	833			21.00	
24	4.32	54, 9.7	2 8.1	15.12		20.52	2.1	4.44	54	9.99		5.54		21.09	
245 25	4.41	54½ 9.8 55. 9.9	1   ~42 0   85	15.21		20,61	245	4.53	0101	10.08			114½ 2 115 2		
251	4.59	552 9.5		15.39		20.79	251	4.72		10.27	851		1155:		
26 261	4.68	56 10.0		15.48		20.88		4.81	56	10.36	86 1	5.91	116	21,43	
27	4.86	562 10.1		15.57		20.97	261	4.99		0.54	861 1	6.09	1163	11.64	
271	4.95	572 10.3	5 875	15.75	1173		275	5,09	571	0.64	375 1		1172 2	1.74	
28 283	5.04	58 10.4	4 88	15.84		21.24	28	5.18	58 1	0.73	88 1		118	11.83	
80	5.13	582 10.5		15.93	1181	21.33	283	5. 27	581 1	0.02	883 1		1187 2	2.01	
292	5.31	592 10.7	r 891	16.11	1191	21.51	295	5.46	591		891 1	6.56	1191 2	2.11	
30	5.40	60 10.8	90	16,20	120	21,60	30	5.55	60 1	1.10	90 1	6.65	120 2	2,20	

TOTED			S PER HO		AT 19½ CENTS P				
IOUR.	Hrs. Am't.	Hrs.  Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. A	m't, Hrs.	$[\Delta \mathbf{m}]$ .	Hrs. Am't.	Hrs. Am't.
:. Hrs. Am't									
901 16.74		301 5.79	602 11.49	901 17.19		301		60111.80	901 17.65
91 16.83		31 5.89	61 11.59	91 17.29		31	6.04	61 11.89	91 17.74
913 16.93	2 0.38	31½ 5.98	62 11.78	91½ 17.38	2 0	$31\frac{1}{2}$	6.14	62 12.00	92 17.94
92 17.02	25 0.38	3-1	623 11.87	92 17.40		$\begin{array}{c c} 0.39 & 32 \\ 0.49 & 32 \\ 0.49 & 32 \end{array}$	6.34	623 12.19	92 18.04
923 17.11	3 0.57	3-2 6 0-1	63 11.97	93 17.67		58 33	6.43	63 12.28	93 13.13
93 17.20	31 0.66	1 33 1 / /1	631 12.06	931 17.76		$33\frac{1}{2}$	6.53	632 12.38	931 18.23
93 17.30	4 0.76	33½ 6.30 34 6.46	64 12.16	94 17.86		.78 34	6.63	64 12.48	94 18.33
94 17.39	45 0.85	341 6.55	$64\frac{1}{2}$ 12.25	945 17.95		.88 342	6.73	641 12.58	942 18.43
942 17.48	5 0.95	35 6.65	65 12.35	95 18.05	5 0	97 35	6.82	65 12.67	95 18.52
95 17.57	55 1.04	353 6.74	$65\frac{1}{2}$ 12.44	952 18.14	$5\frac{1}{2}$ I	.07 35	6.92	653 12.77	952 18.62
955 17.07	6 1.14	36 0.84	66 12.54	96 18.24		.17 36	7.02	66 12.87	96 18.72
96 17.70	6 1.23	361 6 93	662 12.63	965 18.33		.27 $36\frac{1}{2}$	7.12	661 12.97	961 18.82
961 17.85	7 1.33	37 7.03	67 12.73	97 18.43		.36 37	7.21	67 13.06	97 18.91
97 17.94 97 18.04	72 1.42	375 7.12	672 12.82	971 18.52		.46 372	7.31	671 13.16	97 19.01
98 18.13	8 1.52 81 1.61	38 7.22	68 12.92 681 13.01	98   18.62   985 18.71	- 1	.56 38 .66 385	7.41	68 13.26	982 19.21
983 18.22	Δ	38½ 7.31 30 7.41	69 13.11	99 18.81		.75 39	7.60	681 13.36	99 19.30
99 18.31	9 1.71 9 1.80	1 37 1 1 1 1	69 13.11	995 18.90		$.85 \mid 39_{1} \mid$	7.70	693 13.55	995 19.40
991 18.41	10 1.90	1 3 3 3 5 6	70 13.30	100 19.00		.95 40	7.80	70 3.65	100 19.50
100 18.50	101 1.99	40 7.69 401 7.69	70 13.39	100 19.09		.05 40.	7.90	701 13.75	1001 19.60
1001 18.59	11 2.09	41 7.79	71 13.49	101 19.19		. 14 41	7.99	71 13.84	101 19.69
101 18.68	111 2.18	411 7.88	712 13.58	1015 19.28	111 2	.24 413	8.09	71 13.94	$101\frac{1}{2}$ 19.79
1012 18.77	12 2.28	42 7.98	72 13.68	102 19.38	12 2	34 42	8.19	72 14.04	102 19.89
102 18.87	123 2.37	423 8.08	72. 13.77	1021 19.47		44 425	8, 29	722 14.14	1021 19.99
1021 18.96	13 2.47	43 8.10	73 13.87	103 19.57		53 43	8.38	73 14.23	103 20.08
103 19.05	131 2.50	431 8.27	732 13.96	1031 19.60	0.5	.63 432	8.48	732 14-33	1032 20.18
103½ 19.15	14 2.66	44, 8 35	74 14.06	104 19.76		73 44	8.58	74, 14.43	104 20.28
104 1 19.33	14½ 2.75 15 2.85	441 8.46	741 14.15	1042 19.65	* ***	$\begin{array}{c c} .83 & 44\frac{1}{2} \\ .92 & 45 \end{array}$	8.68	741 14.53	1042 20.38
105 19.42	15 2.85	45 8.54 451 8.65	75 14.25	105 19.95	J.	, ,	8.87	75 14.62 755 14.72	105 20.47
7 1053 19.52	16 3.04	423 0 - 1	75½ 14.34 76 14.44	106 20.14		.02   45½	8.97	75½ 14.72 76 14.82	106 20.67
10.61	165 3.13	46 8.73 463 8.84	761 14.53	106 20.23		22 461	9.07	76 14.92	100 20.77
5 106 19.70	17 3.23	47 8.92	77 14.63	107 20.33	80	31 47	9.16	77 15.01	107 20.86
1 107 19.79	173 3.32	473 9.03	77 14.72	107. 20.42		41 472	9.26	773 15.11	1072 20.96
1075 19.89	18 3.42	48 9.12	78 14.82	108 20.52		51 48	9.36	78 15.21	108 21.06
3 108 19.98	18½ 3.5I	481 9.21	78½ 14.91	1082 20.61	181 3	61 485	9.46	781 15.31	1082 21.16
2 1081 20.07	10 3.01	49 9.31	79 15.01	109 20.71		70 49	9.55	79 15.40	109 21.25
1 109 20.10	195 3.70	493 9.40	793 15.10	1092 20.80		80 492	9.65	79 15.50	1095 21.35
0 110 20.35	20 3.80	50 9.50	80 15.20	110 20.90		90 50	9.75	80 15.60	110 21 45
11 1	20½ 3.89 21 3.99	501 9.59	801 15.29	110 20.99	die	00 503	9.85	802 15.70	110 21.53
3 111 20.53	213 4.08	51 9.09	81 15.39 81 15.48	$111 \frac{1}{2} 21.09$		19 51	9.94	81 15.79 81 15.89	111 21.64
3 1113 20.63	22 4.18	513 9.78 52 9.88	82 15.58	112 21.28	40		10.1.1	82 15.99	112 21.84
7 112 20.72	221 4.27	521 9.97	821 15.67	1125 21,37			10.2.1	821 10.00	1125 21.04
5 1125 20.81	23 4.37	53 10 07	83 15.77	113 21.47	60		10.33	83 16,18	113 22.03
5 113 20.90	235 4.40	531 10.16	833 15.86	1135 21.50			10.43	835 16.28	1132 22.13
5 1135 21.00	24 4.50	54 10.26	84 15.96	114 21.66			10.53	8.1 16.38	114 22.23
4 114 21.09	215 4.05	545 10.35	844 16.05	11.13 21.75		78 543	10.53	843 16.48	1142 22.33
3 114 2 21.18	25, 4.74	55 10.45	85 16, 15	115 21.85	25 4.		10.72	85 16.57	115 22.42
2 115 21.27	= 254 4.84	553 10.54		115 21.94	255 4.	97   555	10.82	855 16.67	1152 22.52
2 115½ 21.37 1 116 21.43	26 5.03	56 10.64		116 22.04			10.92	86 16.77	116 22,62
0 1162 21.55	26½ 5.03 27 5.13	561 10.73	861 16.43	116 22.13		. 11	11.02	861 16.87	116 22.72
9 117 21.64	275 5.22	57 10.83 578 10.92		117 22.23			11.11	87 16.96 87 17.06	117 22.81
9 1173 21.74	28 5.32	58 11.02		118 22.42			11.31	88 17.16	118 23.01
8 118 21.83	281 5.41	581 11 11		118 22.51		56 583	11.41	884 17.26	1181 23.11
7 118 21.92	20 5.51	50 11.21		119 22,01	44		11.50		119 23 20
5 119 22.01	292 5,00	595 11.30		119 22.70			11.60		119 23.30
5 119 22.11	30 5.70	65 11.40		120 22.80			11.70		120 23.40
5 120 22.20	And in contrast of the last	Million and the second second second second					-	the contract of the contract o	The state of the s

	AT	20	CENT	S PE	R HO	our.	-	AT 20½ CENTS PER HOUR.							
Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Ām't.	His.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.
		305	6.10	601	12,10	ool	18. 10			301/2	6.25	601	12.40	001	18.55
- 1	Í		6.20	61	12.10	903	18,20			302	6.35	61°	12. 0	902	18.65
- 1	1	$\frac{31}{31\frac{1}{2}}$	6.30		12.30		18.30			315	6.46		12.61		18.76
2	0.40	32	6.40	62	12,40	52	18.40	2	0.41	32	6.56	62	12.71	92	18.86
25	0.50	321	6.50		12.50		18.50		0.51	321	6.66	621		92.	18.96
3	3.60	33	6.60	63	12.60	93	18.60	3	c.61		6.76	63	12.91	93	19.06
32	0.70	332	6.70		12.70		18.70		0.72	$\frac{33}{33\frac{1}{2}}$	6.87	631		931	19.17
4	0.80	34	6.80		12.80	94	18.80		0.82	34	6.97	64	13.12	94	19.27
$4\frac{1}{2}$	0.90	$34\frac{1}{2}$	6.90	1 - 1	12.90		18.90	41	0.92	342	7.07		13,22		19.37
5.	1.00	35.	7.00		13.00	95	19.00		1.02	35	7.17	65	13.32	95	19.47
51	1.10	352	7.10		13.10		19.10	$\frac{5}{2}$	1.13	$35\frac{1}{3}$	7.28		13.43		19.58
6	1.20	36	7.20		13.20	96	19.20		1.23	36	7 38	66	13.53	96	19.68
65	1.30	351	7.30		13.30	1 - 1	19.30	c 1	1.33	$36\frac{1}{2}$	7.48	661	13.03		19.78
7	1.40	37	7.40		13.40	97	19.40	7	1.43	37	7.58	67	13.73		19.88
$7\frac{1}{2}$	1.50	37 2	7.50				19,50		1.54	375	7.59	673	13.84		19.99
8	1.60	38	7.60		13.60		19.60	8~	1.64	38	7.79	68	13.94		20.09
8.	1.70	38.	7.70		13.70	1 1	19.70	81	1.74	385	7.89	681	14.04		20.19
9	1.80	39	7.85		13.80	1 - 21	19.80	9	1.84	39	7.99	69	14.14	99	20.29
$-9\frac{1}{2}$	1.90	395	7.90		13.90	1 1	19.90	$9^{1}_{2}$	1.95	392	8.10	695	14.25	$99\frac{1}{2}$	20.40
10	2.00	40	8.00				20.00	10	2.05	40	8.20	70	14.35	100	20.50
10.	2.10	402	8, 10	701	14.10		20.10	100	2.15	405	8.30	703	14.45	1002	20.00
11	2.20	41	8.20	71	14.20		20.20	11	2.25	41	8.40	71	14.55	1-0-	20.70
115	2.30	415	8.30	711	14.30	1017	20.30	112	2.36	413	8.51	715	14.66	1 2	20.81
12	2.40	42	8.40		14.40	102	20.40	12	2.46	42	8.61	72	14.70		20.91
123	2.50	422	8.50	721	14.50	102	20,50	122	2.56	421	8.71	725	14.80	1022	21.01
13	2.60	43	8,50	73.	14.60	103	20,60		2,66	43	8.81	73.	14.96	100	21.11
135	2.70	435	8.70		14.70	1035	20.70	135	2.77	$43\frac{1}{2}$	8.92	$73^{1}_{2}$	15.07	1-032	21.22
1.4	2.80	44,	8.80		14.80	1 1	20.80		2.87	4.1	9.02	74,	15.17	104	21.32
145	2.90	442	8.90		14.90		20.90	142	2.97	445	9.12		15.27	1045	21,42
15,	3.00	45,	9.00	75,	15.00		21.00		3.07	45,	9.22		15.37	105	21.52
153	3.10	455	9.10		15, 10	0	21.10	152	3.18	$45\frac{1}{2}$	9.33		15.48	1051	21.05
10	3.20	46	9.20		15.20		21.20	10	3.28	40	9-43	76	15.58	106	21.73
162	3.30	465	9.30		15.30		21.30	162	3.38	461	9.53	' -1		1061	
17	3.40	47	9.40		15.40		21.40	17	3.48	.17,	9.03	77	15.78		21.93
175	3.50	472	9.50		15.50		21.50	175	3.59	475	9.74		15.89	1071	22.14
18	3.60	48	9.00		15.00	1	21,60	18	3.69	.48	9.84	78	15.99	108	
181	3.70	482	9.70			-	21.70	187	3.70	485	9.94		10.19		22.34
101	3.80	49	9.80		15.80		21,80		3.89	49	10.04	79	16.30	109	
20	3.90	402	9.90		15.90		21,90	195	4.00	492	10.15	80	16.40		22.55
20.	4.10	50	10,00	1 - 1	16.10		22.00 22.10	205	4.10	50			10.50	110	
21	4.20	51	10.20		16.20	111	22,10	21		503	10.45	81	16.60		22.75
211	4.30	513		1	10.30		22, 30	215	4.41	513	10.50		16.71		22.86
22	4.40	52	10,40		10.40	1112	22,40	22	4.51	52	10.66	82	16.81	2	22.90
22!	4.50	52	10,50		16.50		22.50	224	4.01	523	10.76		16.91	112	
23	4.60	53	10.60		16.60	441	22.60	23	4.71	53	10.86	83	17.01		23.10
235	4.70		10.70		10.70		22.70	235	4.82	533	10.97		17.12		23.27
2.1	4.80	54.	10.80		16.80	114	22.80	24	4.92	54	11.07	84	17.22		23 37
245	4.90		10,90	8.15	16.90	1144	22,90	246	5.02		11.17		17.32	1145	
25	5.00		11.00	85	17.00	115	23.00	25	5.12	55	11.27		17.42	115	
251	5.10	553	11,10	853	17.10	1155	23.10	253	5.23	551	11.38		17.53	1155	53 08
26	5.40		11.20	86	17.20		23.20		5.33		11.48	86	17.63	116	23.78
261	5.30		11.30			1165	23.30	261	5.43	564	11.58	1	17.73	1103	23.88
27	5.40		11.40	87	17.40		23.40		5.53	57	11.68		17.83	117	23.98
271	5.50	573	11.50				23.50		5.64	573	11.79		17.94	1175	
28	5.60	58	11,60	88	17.60		23.60		5.71		11.89		18.04	118	
281	5.70		11.70	881			23.70		5.84		11.99	884	18.14	1187	
120	5.80	59.	11,80		17.80	119	23.80	29	5.94	5)	12.09	89	18.24	110	
29												0 1	173	1 1	
292	5.90	592	11.90		17.00			205	0.05	599	12.20		18.35	1195	

HOUD			ΔΤ.	21 (	ENTS	PF	B HO	IIR		1	ΔТ	211/	CENT	rg Pi	TR H	OHR	
HOUR.	-	YY							! A ! 4	T.J. w.						**	
t. Hrs.	Am't,	Hrs.	Am't.	rirs.	Am't,	rirs.	Ani t	1115.	Am t.	rirs.	Am t.	rirs.	Am t.	HITS.	Am t.	TITS.	Am t
001	18.55			301	6.40	603	12.70	903	19.00			301	6.56	603	13.01	903	19.46
<b>p</b>   902	18.65			31	6.51	61	12.81	91	19.11			31	6.66	61	13.11	91	19.56
	18.76			$31\frac{1}{2}$	6.óI	1 . 4	12.91		19.21			$31\frac{1}{2}$	6.77	1 ~	13.22	11	19.67
11 2- 1	18.86	2	0.42	32	6.72	62	13.02	92	19.32	2	0.43	32	<b>6.88</b> 6.99	62	13.33	92	19.78
11 2 21.	18.96	20	0.52	323	6.82	63	13.12		19.42	25	0.54	321	7.09	63	13.44 13.54	92½	19.89
	19.06 19.17	3 3 <sup>1</sup> / <sub>2</sub>	0.63	33	7.03		13.33		19.53 19.63	$\frac{3}{3\frac{1}{2}}$	0.75	$\frac{33}{33\frac{1}{2}}$	7.20		13.65	$93\frac{1}{93\frac{1}{2}}$	1
11 23-1	19.27	4	0.84	33½ 34	7.14	64	13.44	94	19.74	4	0.86	34	7.31		13.76	94	20.21
11 2 1	19.37	$4\frac{1}{2}$	0.94	345	7.24		13.54		19.84	$4\frac{1}{2}$	0.97	$34\frac{1}{2}$	7.42		13.87	942	20.32
	19.47	5.	1.05	35	7.35		13.65	95	19.95	5,	1.07	35	7.52		13.97	95	20.42
11 -221	19.58	52	1.15	351	7.45		13.75		20.05	52	1.18	355	7.03		14.08	953	20.53
11 2- 1	19.68	6 61	1.26	36	7.56		13.86	9	20, 16 20, 26	6 <u>1</u>	1.40	36 36	7.74 7.85		14.19 14.30	96	20.75
11 - 641	9.78	7	1.47	36½ 37	7.77	67	13.07	- 20	20.37	7	1.50	37	7.95		14.40	97	20.85
11 74 . 1	9.99		1.57	$\frac{37}{37\frac{1}{2}}$	7.87		14.17		20.47	71/3	1.61	37 1	8.06		14.51	$97\frac{1}{2}$	20.96
	0.09	$7^{\frac{1}{2}}_{2}$	1.68	38	7.98		14.28		20.58	8	1.72	38	8.17		14.62	98	21,07
11 2 31	0.19	81	1.78	382	8.08		14.38		20.68	81	1.83	$38\frac{1}{2}$	8.28		14.73		1
	0.29	9,	1.89	39	8. <b>1</b> 9   8, <b>2</b> 9		14.49		20.79 20.89	9 91	2.04	39	8.38 8.49		14.83 14.94	99	21.28
11 772 .	0.40	9½ 10	1.99 2.10	392	8.40	70	14.70		21.00	10	2.15	39½ 40	8.60		15.05	100	21.50
	0.60	103	2.20	40 403	8.50		14.80		21.10	103	2.26	403	8.71		15.16		21.61
112	0.70	11	2.31	41	8.61	71	14.91	101	21.21	11	2.36	41	8.81	71	15.26	101	21.71
	0.81	$11\frac{1}{2}$	2.41	$41\frac{1}{2}$	8.71	- 00	15.01		21.31	1112	2.47	4112	8.92	7 m	15.37	1012	21.82
11 1	0.91	12	2.52	42	8.82		15.12		21.42	12	2.58	42	9.03		15.48	102	21,93
112	1.01	125	2.62	425	8.92		15.22 15.33		21.52 21.63	122	2.69	422	9.14		15.59 15.69	-	22,04 22,I4
11. 3	1.11	13	2.73	43 43 <sup>1</sup> / <sub>2</sub>	9.03		15.43	1035		131	2.90	$\frac{43}{43\frac{1}{2}}$	9.35		15.80	U . I	22.25
1123	1.32	1.1	2.94	44	9.24		15.54		21.8.1	14	3.01	44	9.46	74	15.91	104	22.36
	1.42	143	3.04	445	9.34	$74\frac{1}{2}$	15.64	1045		142	3.12	$44\frac{1}{2}$	9.57		16.02		22.47
105 2	1,52	15,	3.15	45,	9.45		15.75	N' + 1	22.05	15	3.22	45,	9.67		10.12	105	22.57
10512	1.03	155	3.25	451	9.55		15.85   15.96	1052	22.15 22.20	152	3.33	45 <u>\$</u>	9.78		16.23 16.34		22,68 22,79
	1.73	163	3.46	46 46 <del>1</del>	9.06		16.06	1061		163	3.55		10.00		16.45		
. 11 - 21	1.03	17	3.57	47	9.87	, ,	16.17		22.47	17	3.65		01.01	1 44 1	16.55	-	23.00
	2.04	173	3.67	475	9.97		16.27	1072	22.57	$17\frac{1}{2}$	3.76	T Aug	10.21	1 1 11	16.60		23.11
1	2,14	18	3.78	- 9	80 01		16.38		22.68	18	3.87		10.32		16,77		23.22
2 -	2,24	181	3.88	4.5	10.18	. ~!	16.48	1085	22.78 22.89	732	3.98		10.43	2	16,88   16,98	1005	23.33
11 - 1 -	2.34	19 19 <sup>1</sup>	3.99	777	10.29	79	16.69	1091		19	4.19		10.53   10.64	1 / / . 1	17.09		23.43 23.54
11 221	2,55	20	4 20	50	10.50	80	16.80		23.10	20	4.30		10.75		17.20		23.65
11	2.05	20 ]	4.30		10.60	801	16.90	01	23.20	205	4-41		10.86	803	17.31	1107	23.76
	2.75	21	4.41	51	10.71	81	17.01	1 11	23.31	21	4.51	U	10.96		17.41		23.86
112	2.80	215	4.51	2.2	10.81		17.11	1 -	23.41	213	4.62	0 0	11.07	- 4-	17.52		23.97
~ ~ ~	2.90 3.00	22	4.62	52 523	10.92		17.22		23.52 23.62	22	4.73	0 .	11.29	- 4	17.63		24.10
	3, 10	23	4.83	53	11.13	83	17.43		23.73	23	1.91		11.39		7.84	- 1	24.29
11 0.1	. 27	233	4.93		11.23	0 1	17.53		23 83	232	5.05		11.50		17-95	1135	24.40
	37	2.1	5.04	54	11.34	84	17.64		23.94	2.4	5.16		11.61		18.00		24 51
	3-47	245	5.14	4 60	11.44	842	17.7.4	1145		245	5.27		11.72		18.17		21,62
115 23	3.57	25 251	5.25	55	11.55		17.85		24.15 24.25	25 253	5.48		11.82    11.93	85	18.38		24.53
1155 5		26	5.46		11.76		18.06		24.36	26	5.59		12.04	86			24.04
110 2	88.8	261	5.56	561	11.86	861	18.16		24.46	261	5.70		12,15	863	15.60	1107	25.05
117 23		27	5.67	57	11.97	87	18.27	117	24.57	27	5.80	57	12.25		18.70	117	25.15
1175 24	.09	273	5.77		12.07	871	18.37	1174		271	5.91		12.30		18.81		25,20
118 24		28 28}	5.98	58	12.18		18.48		24.78 24.88	281	6.02		12.47		1 92 19.03	1181	25.37 25.48
1182 24		203	6,00		12.30		18.69		24.70	200	6.23		12.68		19.13		25.58
110 24		20/2	6.19	593	12.49		18.79		25.00	293	6.31		12.79	893.	19.21	1103	25.60
120 24		30	6.30	60	12.60		18.90		25.20		6. 15	. 60	12,60				25.80
-				-						and the same of the same of							

Hrs. A

	AT	22	CENT	S PE	R HO	UR.		1	AT 2	22½	CENT	S P	ER H	OUR	•
Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.
		30 \}	6.71	603	13.31	ook	19.91			301/2	6.86	603	13.61	001	20.36
		31	6.82	61	13.42	91	20.02			31	6.97	61	13.72	91	20.47
		312	6.93	615	0 00		20.13			31 1/2	7.09		13.84	$91\frac{1}{2}$	<b>20.</b> 59
2.	0.44	32	7.04	62 62	13.64	92	20.24	2	0.45	32	7.20	62	13.95	92	20.70
3.	0.66	323	7.26	63	13.75	92½ 93	20. 35 20. 46	$\frac{2\frac{1}{2}}{3}$	0.56 c.67	32½ 33	7.42	63	14.17	92 <u>1</u>	20.81 20.92
32	0.77	$33\frac{1}{2}$	7.37		13.97	932	20.57	$\frac{3}{3^{1}_{2}}$	0.79	331	7.54		14.29		21.04
4	0.88	34	7.48	64	14.08	94	20.68	4.	0.90	34	7.65	6.4	14.40	94	21.15
$\frac{4\frac{1}{2}}{5}$	0.99	345	7.59		14.19		20.79	42	1.01	341	7.76		14.51		21.26
$\frac{51}{52}$	1.21	35 35 2	7.70	65 65 65 65 65	14.30	95	20.90 21.01	5 52	1.12 1.24	35 <sub>1</sub> 35 <sub>2</sub>	7.87	65 65 l	14.62		21.37 21.49
6	1.32	36	7.92		14.52	1	21.12	6	1.35	36	8.10	66	14.85		21.60
$6\frac{1}{2}$	1.43	361	8.03	11	14.63		21.23	65	1.46	$36\frac{1}{2}$	8.21	. ~	14.96		21.71
$\frac{7}{7\frac{1}{2}}$	1.54	37 37 ½	8, 14 8, 25		14.74 14.85	97	21.34	7 71 72	1.57	37	8.32 8.44	671	15.07		21.82 21.94
8	1.76	38	8.36		14.96		21.45	82	1.80	37½ 38	8.55	68	15.30		22.05
85	1.87	385	8.47	681	15.07		21.67	$8\frac{1}{2}$	1.91	381	8.66		15.41		22.16
91 92	2.09	39	8.58		15.18		21.78	9,	2.02	39	8.77		15.52		22.27
10	2.20	39½ 40	8.69 8.80		15.29		21,80 22.00	$9^{1}_{2}$	2.14	39½ 40	9.00	69½ 70	15.64 15.75		<b>22.</b> 39 <b>22.</b> 50
102	2.31	$40\frac{1}{2}$	8.91		15.51		22.11	105	2.36	401	9.11		15.86	1003	
11	2.42	41	9.02	71	15.62		22.22	11	2.47	41	9.22		15.97		22.72
112	2.53	41½ 42	9.13		15.73		22.33	1112	2.59	$41\frac{1}{2}$	9.34		16.09	1012	
123	2.75	$42\frac{1}{2}$	9.24		15.84   15.95	102	22,44 22,55	$12 \\ 12 \\ 2$	2.70	42 42	9.45		16.31		22.95 23.06
13	2.86	43	9.46		16.06		22.66	13	2.92	43	9.67	73	16.42	· ~ i	23.17
132	2.97	432	9.57		16.17	1032		135	3.04	$43\frac{1}{2}$	9.79	.02	16.54	103½	_
$\frac{14}{14\frac{1}{2}}$	3.08	$\frac{44}{442}$	9.68 9.79		16.28 16.39	104	22.88	14 14 <sup>1</sup> / <sub>2</sub>	3.15   3.26	44	9 90	$\frac{74}{74\frac{1}{2}}$	16.65 16.76	104	23.40
15	3.30	45	9.79		16.50	105	22.99 23.10	15	3.37	1 10	10.12		16.87		53.62
151	3.41	452	10.01	753	16.61	1055	23.21	153	3.49	10	10.24	$75\frac{1}{2}$	16.99	1051	
161	3.52 3.63	46 46§	10,12		16.72	106	23.32	16	3.60	46	10.35		17.10		23.85
17	3.74	47	10.23		16.83 16.94	1061	23.43 23.54	165	3.71		10.46   10.57	* 44	17.21 17.32	1061	23.90 24.07
172	3.85	472	10.45		17.05	1072		175	3.94	**	10.69		17.44	1075	
18	3.96	48	10.56			108	23.76	18	4.05	48	10.80	78	17.55		24.30
185	4.07	483	10.67		17.27	1081		189	4.16	* **	10.91	64	17.66	1081	
193	4.29		10.89			109	<b>23</b> ,98 21,00	191	4.27		11.14		17.77 17.89	109	24.52 24.61
20	4.40	50	11.00	80	17.60	IIO	24.20	20	4.50		11.25	80	18.00		24.75
202	4.51	-	11,11			1102	24.31	201	4.61	~ ~ 1	11.30		18.11	1103	,
213	4.73	$51\frac{1}{51\frac{1}{2}}$	11.22		17.82   17.93	111 1	24.42	21 213	4.72		11.47    11.59		18.22	1111	24.97
22	4.84	52.	11.4.1		18.04		24.64	22	4.95		11.70		18.45		25.20
22.	4.95	523			18.15	1122	24.75	$22\frac{1}{2}$	5.06	521	11.81		18.50	1121	25.31
23 23 <sup>1</sup> / <sub>2</sub>	5.00	53	11,66		18.26		24.86	23	5.17	00,	11.92		18.67		25 .12
24	5.28	54.	11.88		18. 37 18. 48		24.97 25.08	232	5.29		12.04	to do	18.90	1135	25.51 25.65
241	5-39		11.99		18.59		25.19	245	5.51		12,26		19,01	1145	25.76
25	5.50	55,	12.10		18.70	115	25.30	25	5.62	55	12.37	85	19.12	115	
253	5.72	55ª 56	12.21		18.81	1152	25.41 25.52	252	5.74	552	12.49		19.24 19.35	1152	<b>2</b> 5.99 <b>2</b> 6.10
261	5.83	561	12.43		19.03	116	25.63	261	5.85		12.60		19.35	1163	
27	5.94	57	12.54	87	19.14	117	25.74	27	6.07	57	12,82	87	19.57	117	26.32
275	6.05		12.65		19.25	1175	25.85	272	6.19	571	12.94		19.69	1175	
283	6.10	58 583	12.76		19.36		25.96 26.07	28	6.41		13.05		19.80	118	26, 55 26, 66
29	6.38	59.	12.98		19.58		26.18	200	6.52	U a	13.27		20,02	119	
292	6.49	593	13.09	801	19.69	1191	26.29	201	6.64		13.39	895	20.14	1195	26.19
30	6.60	00	13.20	90	19.80	120	26.40	30	6.75		13.50	90	20.25	120	27.00

 $\begin{array}{c} \mathbf{2} \\ \mathbf{2}^{\frac{1}{2}} \\ \mathbf{3} \\ \mathbf{3}^{\frac{1}{2}} \\ \mathbf{4}^{\frac{1}{2}} \\ \mathbf{5} \\ \mathbf{5} \\ \mathbf{6}^{\frac{1}{2}} \\ \mathbf{7} \\ \mathbf{7} \\ \mathbf{8} \\ \mathbf{10} \\ \mathbf{10} \\ \mathbf{11} \\ \mathbf{11} \end{array}$ 

11½
12
13
13½
14
14½
15½
16½
17
17½
18
18½
19
10½

20

		24 CENT			AT 24½ CENTS PER HOUR Hrs.  Am't.  Hrs.  Am't.  Hrs.  Am't.  Hrs							
Hrs.	Àπ.	Hrs. Am'ı.	Hrs. Am't.	Hrs. Am't.	Hrs. Am t.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't				
		30 / 7.32	601 14.52	901 21.72		301/2 7.47	601 14.82	901 22.17				
		31 7.44	61 14.64	91 21.84		31 7.59	61 14.94	91 22.29				
2	0.48	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	62 14.88	91 21.96		31½ 7.72 32 7.84	62 15.19					
21	0.60	$32\frac{1}{2}$ 7.80	62 15.00	$92\frac{1}{2}$ 22.20		$32\frac{1}{2}$ 7.96	$62\frac{1}{2}$ 15.31	921 22.66				
3,	0.72	33 7.92	63 15.12	93 22.32	3, 0.73	33 8.08	63 15.43	93 22.78				
$\frac{3^{1}_{2}}{4}$	0.84	33½ 8.04 34 8.16	63 15.24	932 22.44		33½ 8.21 34 8.33	63½ 15.56 64 15.68	93½ 22.91				
$\frac{4}{4\frac{1}{2}}$	1.08	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	64 15.30	94 22.56 94 <sup>1</sup> 22.68	4 0.98 4 <sup>1</sup> / <sub>2</sub> 1.10	34 8.33 34½ 8.45	641 15.80	94 23.03 94 23.15				
5	1.20	35 8.40	65 15.60	95 22.80	5 1.22	35 8.57	65 15.92	95 23.27				
5½ 6	1.32	35\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	65 15.72	951 22.92	5 1.35	35½ 8.70	65 16.05	951 23.40				
6.5	1.44	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	66 15.84 66 <sup>1</sup> 15.96	96 23.04 96 <del>1</del> 23.16	6 1.47 61 1.59	36 8.82 361 8.94	66 16.17	96 23.52 96½ 23.64				
7	1.68	37 8.88	67 16.08	97 23.28	7 1.71	37 9.06	67 16.41	97 23.76				
71	1.80	37 2 9.00	672 16.20	972 23.40	75 1.84	372 9.19	671 16.54	972 23.89				
8 8 8	2.04	38 9.12 38½ 9.24	68 16.32	98 23.52		38 9.31 38½ 9.43	$\begin{array}{c c} 68 & 16.66 \\ 68 & 16.78 \end{array}$	98 24.01				
9	2.16	39 9.36	69 16.56	98½ 23.64 99 23.76	9 2.20	38½ 9.43 39 9.55	69 16.co	905 24.13				
$9\frac{1}{2}$	2.28	$39\frac{1}{2}$ 9.48	695 16.68	991 23.88	92 2.33	391 9.68	692 17.03	992 24.38				
10	2.40	40 9.60	70 16.80	100 24.00		40 9.80	70 17.15	100 24.50				
102	2.52	40½ 9.72 41 9.84	70½ 16.92 71 17.04	100½ 24.12 101 24.24	10½ 2.57 11 2.69	40½ 9.92 41 10.04	$70\frac{1}{2}$ 17.27 71 17.39	1001 24.64				
$11\frac{1}{2}$	2.76	415 9.96	71 17.04	101 24.24	$11\frac{1}{2}$ 2.82	41 10.04	71 17.52	101 3 24.87				
12	2.88	42 10.08	72 17.28	102 24.48	12 2.94	42 10.29	72 17.64	102 24.99				
121	3.00	425 10.20	721 17.40	102 24.60	122 3.06	421 10.41	72 17.76	1021 25.11				
135	3.12	43 10.32	73 17.52 73 <sup>1</sup> 17.64	103 24.72	13 3.18 13 <sup>1</sup> 3.31	43 10.53 43 <sup>1</sup> 10.66	73 17.88 73 <sup>1</sup> 18.01	103 25.23				
14	3.36	44 10.56	74 17.76	104 24.96	14 3.43	44 10.78	74 18.13	104 25.48				
142	3.48	442 10.68	742 17.88	1042 25.08	141 3.55	$4.4\frac{1}{2}$ 10.90	742 18.25	10.4 \ 25.60				
151	3.60	45 10.80 45 <sup>1</sup> 10.92	75 18.00 75½ 18.12	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	15 3.67 151 3.80	$45   11.02   45\frac{1}{2}   11.15  $	75 18.37 75 18.50	105 25.72				
16	3.84	46 11.04	76 18.24	105 25.44	16 3.92	46 11.27	76 18.62	106 25.97				
161	3.96	46½ 11.16	762 18.36	1062 25.56	162 4.04	461 11.39	761 18.74	1061 26.09				
$\frac{17}{17\frac{1}{2}}$	4.08	47 11.28 47 11.40	77 18.48	107 25.68	17 4.16	47 11.51	77 18.86	107 26,21				
18	4.32	475 11.40	77½ 18.60 78 18.72	107 2 25.80	17½ 4.29 18 4.41	47½ 11.64 48 11.76	77½ 18.99 78 19.11	107 26.34				
$18\frac{1}{2}$	4.44	48111.64	781 18.84	108 25.04	185 4.53	481 11.88	78 19.23	1081 26.58				
19	4.56	49 11.76	79 18.96	109 26.16	19 4.65	19 12.00	79 19.35	109 26.70				
20	4.68	49 <u>3</u> 11.88 50 12.00	79½ 19.08 80 19.20	109 26.40	192 4.78	$49\frac{1}{2}$ 12.13	79½ 19.48 80 19.60	1092 26.83				
20.1	4.92	502 12.12	801 19.32	110 26.40 110 26.52	20 4.90	50 12.25	801 19.72	110 20.95				
21	5.04	51 12.24	81 19.44	111 25,64	21 5.14	51 12.49	81 19.84	111 27.19				
211	5.16	512 12.36	811 19.56	1112 26.76	21 2 5.27	513 :2.62	813 19.97	111 2 27.32				
223	5.40	52 12.48	82 19.68 82 <sup>1</sup> 19.80	112 26.88	22 5.39 22 <sup>1</sup> / <sub>2</sub> 5.51	52 12.74 52 12.86	82 20.09	112 27.44				
23	5.52	53 12.72	83 19.92	113 27.12	23 5.63	53 12.98	83 20.33	113 27.68				
232	5.64	532 12.84	832 20.04	113 27.24	232 5.76	531 13.11	831 20.46	1135 27.81				
24	5.76 5.88	54 12.96 542 13.08	84 20.16 84 20.28	114 27,36	24 5.88	54 13.23	84 20.58	114 27.03				
25	6.00	55 13.20	85 20 40	1112 27.48	24½ 6.00 25 6.12	54½ 13.35 55 13.47	84½ 20.70 85 20.82	1143 28.05				
253	6.12	552 13.32	851 20.52	115 27.60 1153 27.72	251 6.25	553 13 60	85 20.95	115 28.30				
26	6.24	56 13.44	86 20,64	115 27.84	26   6.37	56 13 72	86 21.07	116 28.42				
26½	6.36	561 13.56		115 27.96	265 6.49	562 13.84	861 21.10	1162 28.54				
273	6.60	574 13.80	87 20.88 87 21.00	117 28.08	27 6.61 272 6.74	57 13.96	87 21.31	117 28.00 1175 28.79				
28	6.72	53, 13.92	88 21.12	118 28.32	28 6.86	58 14.21	88 21.56	118 28.91				
285	6.84	583 14.04	885 21.24	118 28.44	281 6.98	581 14.33	881 21.08	1181 29.03				
29	7.08	59 14.16	80 21.36	119 28.56 119 28.68	29 7.10	59 14.45	89 21.80	119 29.15				
30	7.20	60 14.40		120 28.80		60 14.70	90 22.05					
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HOUR.	AT	25 CENT	S PER HO	UR.	AT	25½ CEN	IS PER H	OUR
t.   Hrs.   Am't		Hrs. Am't.						
$\frac{1}{90\frac{1}{2}}$ 22.17	1115				I	l		
4 91 22.29		$30\frac{1}{2}$ 7.62 $7.75$	60½ 15.12 61 15.25	90½ 22.52 91 22.75	K 1	$30\frac{1}{2}$ 7.78 $7.90$	60 15.43	902 23.08
7 91 22,42		$31\frac{1}{2}$ 7.87	$61\frac{1}{2}$ 15.37	91 1 22.87		31 8.03	$61\frac{1}{2}$ 15.68	91 23.20
9   92   22.54 1   92   22.66	2 0.50	32 8.00	62 15.50	92 23.00	2 0.51	32 8.16	62 15.81	92 23.46
3 93 22.78 6 93 22.78	21 0.62	322 8.12	$62\frac{1}{2}$ 15.62	$92\frac{1}{2}$ 23.12		321 8.29	621 15.94	$92\frac{1}{2}$ 23.59
	3 0.75 31 0.87	33 8.25 33 <sup>1</sup> 8.37	63 15.75	$93 \ 23.25 \ 93\frac{1}{2} \ 23.37$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	33 8.41 33½ 8 54	63 16.0	93 23.71 932 23.84
94 23.03	3½ 0.87 4 1.00	33½ 8.37 34 8.50	64 16.00	94 23.50	03	34 8.67	64 16.32	932 23.04
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$4\frac{1}{2}$ 1.12	342 8.62	642 16.12	942 23.62	42 1.15	34½ 8.80	642 16.45	94 2 24.10
$95\frac{1}{2}$ 23.40	5 1.25	35 8.75 351 8.87	65 16.25 65 <sup>1</sup> 16.37	95 23.75 95 <sup>1</sup> 23.87	$\begin{array}{c c} 5 & 1.27 \\ 5\frac{1}{2} & 1.40 \end{array}$	35 8.92 35½ 9.05	65 16.57	95 24.22
7   96   23,52	5½ 1.37 6 1.50	351 8.87 36 9.00	66 16.50	96 24.00	6 1.53	352 9.03	66 16.83	952 24.35 24.48
961 23.64	61 1.62	361 9.12	661 16.62	962 24.12	$6\frac{1}{2}$ 1.66	365 9.31	661 16.96	962 24.61
971 23.89	7 1.75	37 9.25	67 16.75 67 16.87	97 24.25	7 1.78 71 1.91	37 9.43	67 17.08	97 2:.73
98 24.01	7½ 1.87 8 2.00	37½ 9·37 38 9·50	68 17.00	97½ 24.37 98 24.50	0 1	$\begin{vmatrix} 37\frac{1}{2} \\ 38 \end{vmatrix} \begin{vmatrix} 9.56 \\ 9.69 \end{vmatrix}$	68 17.34	97½ 24.86
98½ 24.13 99 24.25	81 2.12	381 9.62	681 17.12	981 24.62		381 9.82	681 17.47	985 25.12
991 24.25	9 2.25	39 9.75	69 17.25	99, 24.75	9 2.29	39 9.94	69 17.59	99 25 24
100 24.50	9½ 2.37 10 2.50	39½ 9.87 40 10 00	69½ 17.37 70 17.50	99½ 24.87 100 25.00	$9^{1}_{2}$ 2.42	39½ 10.07 40 10.20	69 <u>5</u> 17.72 70 17.85	100 25.50
1002 24.69	10 2.62	401 10.12	701 17.62	1002 25.12	101 2.68	401 10.33	701 17.98	1002 25.63
101 24.74	11 2.75	41 10.25	71 17.75	101 25.25	11 2.80	41 10.45	71 18.10	101 25.75
102 24.99	11 2 3.00	41½ 10.37 42 10.50	71 17.87	101 25.37 102 25.50	$11\frac{1}{2}$ 2.93 $12$ 3.06	41½ 10.58 42 10.71	71½ 18.23	101½ 25.88 102 26.01
1021 25.11	12 3.00	42 10.50 421 10.62	72 18.12	102 25.62	121 3.19	425 10.84	721 18.49	102 26.14
103 25.23 1031 25.36	13 3.25	43 10.75	73 18.25	103 25.75	13 3.31	43 10.96	73 18.61	103 26.26
104 25.48	132 3.37	432 10.87	731 18.37	103½ 25.87	132 3.44	43½ 11.09	732 18.74	1031 26.39
10.4 5 25.60	14 3.50	44 11.00 44 11.12	74 18.50 74 <sup>1</sup> 18.62	104 26.00 104 26.12	14 3.57 14 <sup>1</sup> / <sub>3</sub> 3.70	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	74 18.87 745 19.00	104 26.52
105 25.72	15 3.75	45 11.25	75 [18.75]	105 26.25	15 3.82	45 11.47	75 19.12	105 26.77
106 25.97	155 3.87	452 11.37	$75\frac{1}{2}$ 18.87	105\frac{1}{2} 26.37	151 3.95	45½ 11.60	752 19.25	1052 26.90
106 2 26,09	16 4.00 16 4.12	46 11.50 461 11.62	76 19.00 761 19.12	106 26.50	16 4.08 16 <sup>1</sup> 4.21	46 11.73   465 11.86	76 19.38 761 19.51	106 27.03
107 26,21	17 4.25	47 11.75	77 19.25	107 26.75	17 4.33	47 11.98	. 41 - 11	107 27.28
108 26.46	172 4.37	472 11.87	772 19.37	1072 26.87	172 4.46	471 12,11	775 19.76	1072 27.41
1081 26.58	18 4.50 18 <sup>1</sup> 4.62	48 12.00 481 12.12	78 19.50 781 19.62	108 27.00	18 4.59 181 4.72	48 12,24 48 12,37	78 19.89 78 20.02	108 27.54
109 26.70 109 26.83	19 4.75	49 12.25	79 19.75	109 27.25	19 4.84	49 12.49	79 20.15	109 27.79
110 26.05	101 4.87	492 12.37	792 19.87	$100\frac{1}{2}$ 27.37	192 4.97	492 12.62	791 20.27	1092 27.92
1102 27.07	20 5.00 20 5.12	50 12.50 501 12.62	80 20 00 80 3 20, 12	110 27.50 110 27.62	20 5.10	50 12.75	80 20.40 80 20.53	110 28.05
111 27.19 111 27.32	21 5.25	51 12.75	81 20.25	111 27.75	21 5.35	51 13.00	81 20.65	111 28.30
111 27.32	21 5.37	511 12.87	811 20.37	1112 27.87	211/2 5.48	511 13.13	811 20.78	1112 28.43
1121 27.50	22 5.50 22 5.62	52 13.00		112 28.00	22 5.61 225 5.74	52 13.26	82 20.91	112 28.56
113 27.08	23 5.75	52½ 13.12 53 13.25	82½ 20.62 83 20.75	113 28.25	225 5.74 23 5.85	53 13.51	83 21.16	113 28.81
113½ 27.81 114   27.93	231 5.87	535 13.37	831 20.87	1131 28.37	233 5.99	532 13.64	831 21.29	1131 25.04
141 28.05	24 6.00	54, 13.50	84 21.00	114 28.50	2.1 6.12	54 13.77	84 21,42	114 29.07
15 28.17	24½ 6.12 25 6.25	54½ 13.62 55 13.75	85 21.25	$114\frac{1}{2}28.62$ $115 28.75$	24½ 6.25 25 6.37	54½ 13.90 55 14.02	8.1 <sup>1</sup> / <sub>2</sub> 21.55 85 21.67	115 20.33
15½ 28.30 16 28.42	$25\frac{1}{2}$ 6.37	551 13.87		1152 28.87	251 6.50	552 14.15	851 21.80	1151 29.45
161 28.54	26 6.50	56 14.00	86 2r.50	116 29,00	26 6.63	56 14.28	86 21.03	116 29.58
17 28.00	26½ 6.62 27 6.75	562 14.12		$116\frac{1}{2}$ 29. 12 117 29. 25	26½ 6.76 27 6.88	56 14.41	86½ 22.06 87 22.18	
17½ 28.79 18 28.01	271 6.87	57 [4.25] 57 <sup>1</sup> [4.37]		117 29.25	27 7.01	57 14.55	873 22.31	1175 29.96
184 29.03	25 7.00	58 14.50	88 22,00	118 29.50	28 7.14	58 14.79	88 22.44	118 30.09
19   29, 15	28 7.12	581 14.62				585 14.92		1182 30.22
191/29.28	29   7.25   292   7.37	59 14.75 59 <sup>1</sup> 14.87		119 29.75 119 29.87	29 7.39 7.52	59 15.04	89 22,69 89½ 22,82	119 30,47
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Α'.	26 CENT	S PER HO	UR.	AT	26½ CEN'	rs per h	OUR.
Hrs. Am'	.  Hrs. Am't,	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.
	Hrs. Am't.  30½ 7.93 31 8.06 31½ 8.19 32 8.32 32½ 8.45 33 8.58 33½ 8.71 344 8.84 34½ 9.910 35½ 9.23 36 9.36 36½ 9.49 37, 9.75 38 9.88			Hrs. Am't.			
9 2.34 9½ 2.47 10 2.60 10½ 2.73 2.86 11½ 2.99 3.12 12½ 3.25 13 3.38	39 IO.14 391 IO.27 40 IO.53 41 IO.66 411 IO.79 42 IO.92 421 II.05 43 II.18	69	99 25.74 992 25.87 100 26.00 1001 26.13 101 26.39 102 26.52 102 26.78	9 2.38 9½ 2.52 10 2.65 10½ 2.78 11 2.91 11½ 3.05 12 3.18 12½ 3.31 13 3.44	39 10.33 39½ 10.47 40 10.60 40½ 10.73 41 10.80 41½ 11.00 42 11.13 42½ 11.26 43 11.39	69 18.28 69½ 18.42 70 18.55 70½ 18.68 71 18.81 71½ 18.95 72 19.08 72½ 19.21 73 19.34	$\begin{array}{c} 99 \\ 99 \\ 26.23 \\ 99 \\ 26.37 \\ 100 \\ 26.50 \\ 101 \\ 26.63 \\ 101 \\ 26.90 \\ 102 \\ 27.03 \\ 102 \\ 27.03 \\ 102 \\ 27.29 \\ \end{array}$
13½ 3.51 14 3.64 14½ 3.77 15 3.90 15½ 4.03 16 4.16 16½ 4.29 17 4.42 17½ 4.55 18 4.68	431 11.31 44 11.44 445 11.57 45 11.70 451 11.83 46 11.96 461 12.09 47 12.22 471 12.35 48 12.48	73½ 19.01 74½ 19.24 7½ 19.37 75 19.50 75½ 19.63 76½ 19.76 76½ 19.89 77 20.02 77½ 20.15 78 20.28	103½ 26.91 104 27.04 104½ 27.17 105 27.30 105½ 27.43 106½ 27.56 106½ 27.69 107½ 27.95 108 28.08	13½ 3.58 14 3.71 14½ 3.84 15 3.97 15½ 4.11 16 4.24 16½ 4.37 17½ 4.64 18 4.77	43½ 11.53 44 11.66 44½ 11.79 45 11.92 45½ 12.06 46 12.19 46½ 12.32 47 12.45 47½ 12.59 48 12.72	73½ 19.48 74 19.61 74½ 19.74 75 19.87 75½ 20.01 76½ 20.14 76½ 20.27 77 20.40 77⅓ 20.54 78 20.67	103½27.33 104½27.46 104½27.59 105 27.72 105½27.86 106½28.12 107½28.39 108 28.52
18½ 4.81 19 4.94 19½ 5.07 20 5.20 20½ 5.33 21 5.46 21½ 5.59 22 5.72 22½ 5.85	48½ 12.61 49 12.74 49½ 12.87 50 13.00 50⅓ 13.13 51 13.26 51½ 13.39 52 13.52 52⅓ 13.65	78½ 20.41 79½ 20.54 79½ 20.67 80 20.80 80½ 20.93 81 21.06 81½ 21.19 82 21.32 82½ 21.45	108½ 28.21 109 28.34 109½ 28.47 110 28.60 110½ 28.73 111 28.86 111½ 28.99 112 29.12 112½ 29.25	18½ 4.90 19 5.03 19½ 5.17 20 5.30 20½ 5.43 21 5.56 21½ 5.70 22 5.83 22½ 5.96	481   12.85   49   12.98   13.12   50   13.25   13.38   13.51   511   13.65   52   13.78   52   13.91	78\ 20.80 79\ 20.93 79\ 21.07 80\ 21.20 80\ 21.33 81\ 21.46 81\ 21.60 82\ 21.73 82\ 21.86	108½ 28.75 109   28.88 109½ 29.02 110   29.15 110½ 29.28 111   29.41 111½ 29.55 112   29.68 112½ 29.81
231 5.98 231 6.11 24 6.24 241 6.37 251 6.63 261 6.76 261 6.89 27 7.02 271 7.15 28 7.28 281 7.41	53 13.78 53½ 13.91 54½ 14.04 54½ 14.17 55 14.30 55½ 14.43 56 14.56 56½ 14.69 57½ 14.82 57½ 14.95 58 15.08 58½ 15.21	84½ 21.97 85 22.10 85½ 22.23 86 22.36 86½ 22.49 87 22.62 87½ 22.75 88 22.88	113 29.38 113½ 29.51 114½ 29.77 115 29.90 115½ 30.03 116 30.16 116½ 30.29 117; 30.42 117; 30.55 118; 30.68 118½ 30.81	23 6.09 23 6.23 24 6.36 24 6.36 25 6.61 25 6.61 25 6.75 26 6.89 26 7.02 27 7.15 27 7.29 28 7.42 28 7.42	53	84 22.26 84½ 22.39 85 22.52 85½ 22.66 86 22.79 86½ 22.92 87 23.05	113   29.94 113\frac{1}{2} 30.08 114   30.21 114\frac{1}{2} 30'34 115   30.47 115\frac{1}{2} 30.61 116   30.74 116\frac{1}{2} 30.87 117   31.00 117\frac{1}{2} 31.14 118   31.29 118\frac{1}{2} 31.40
29 7.54 295 7.67 30 7.80	59 15.34 59 15.47	89 23.14 89½ 23.27	119 30.94 119 31 07	29 7.68 29 7.82 30 7.95	59 15.63 592 15.77	89 23.58 89½ 23.72	119 31.53 119 31.67 120 31.80

UR.		ΑТ	27 CENT	'S PE	R HO	UR.			AT 2	27½	CENT	SP	ER H	OUR	
Hrs. Am't.	Hrs.	Am't	Hrs. Am't.	Hrs.	Am't.	Hrs. A	m't.	His.							
Hrs.   Am't,	2 2 2 3 3 3 2 4 1 4 2 2 5 5 2 6 6 5 7 7 8 8 8 5 9 5 10 10 1 1 1 1 2 1 2 1 3 1 3 1 2 1 1 1 1 1 1 1	0.54 0.67 0.81 0.98 1.35 1.48 1.62 1.75 1.89 2.02 2.16 2.29 2.43 2.97 3.10 3.32 4.97 3.51 3.64 3.78 3.91 4.05 4.45 4.59 5.186 4.99 5.136 5.67 5.80 5.67 5.80 5.67 5.80 6.07 6.07 6.07 6.07 6.07 6.07 6.07 6.0	30   8.23 31   8.37 31   8.50 8.64 8.77 33   9.04 34   9.18 34   9.31 35   9.45 36   9.75 36   9.75 36   9.75 37   9.85 38   10.26 38   10.39 39   10.53 39   10.53 39   10.53 39   10.53 39   10.53 30   10.93 41   11.07 42   11.34 42   11.47 43   11.61 43   11.61 43   11.61 43   11.61 43   11.20 42   11.34 44   12.01 45   12.15 46   12.28 46   12.28 46   12.28 46   12.28 47   12.62 48   12.96 48   12.96 48   13.99 49   13.36 50   13.50 50   13.50 50   13.50 50   13.63 51   13.77 51   14.04 52   14.04 52   14.04 53   14.04 53   14.04 54   14.04 55   14.04 55   14.04 56   14.04 57   14.04 57   14.04 58   14.04 59   14.04 50   14.04 50   14.04 51   14.04 52   14.04 53   14.04 54   14.04 55   14.04 56   14.04 57   14.04	Hrs. 60½ 61½ 62½ 63½ 65½ 66½ 65½ 66½ 67½ 70½ 72½ 75½ 76½ 76½ 76½ 77½ 75½ 76½ 76½ 77½ 75½ 76½ 76½ 77½ 75½ 76½ 76½ 77½ 78½ 80½ 81½ 82⅓ 83¸ 81½ 82⅓ 83¸ 65% 81½ 82⅓ 83¸ 65% 81½ 82⅓ 83¸ 65% 81½ 81½ 82⅓ 83¸ 65% 81½ 81½ 82⅓ 83¸ 65% 81½ 82⅓ 83¸ 65% 81½ 81½ 82⅙ 81½ 82⅙ 81½ 82⅙ 81½ 82⅙ 81½ 82⅙ 81½ 82⅙ 81½ 82⅙ 81½ 81½ 82⅙ 81½ 81½ 82⅙ 81½ 81½ 82⅙ 81½ 81½ 81½ 81½ 81½ 81½ 81½ 81½ 81½ 81½	Am't.  16, 33 16.47 16.60 16.74 16.87 17.01 17.14 17.28 17.48 17.48 17.49 18.63 18.49 18.63 18.49 18.63 19.17 19.30 19.17 19.30 19.17 19.30 19.41 19.57 19.71 19.34 19.57 19.34 19.57 19.30 19.41 19.57 19.30 19.41 19.57 19.21 19.22 19.2	Hrs.   All		$\begin{array}{c} 2\\ 2\frac{1}{2}\\ 3\\ 3\\ 3\frac{1}{2}\\ 4\\ 4\frac{1}{2}\\ 4\\ 5\\ 5\\ 5\\ 5\\ 5\\ 5\\ 5\\ 5\\ 10\\ 10\\ 2\\ 12\\ 12\\ 12\\ 12\\ 12\\ 13\\ 13\\ 2\\ 11\\ 15\\ 15\\ 15\\ 12\\ 12\\ 13\\ 13\\ 13\\ 2\\ 17\\ 17\\ 12\\ 18\\ 18\\ 19\\ 19\\ 2\\ 2\\ 20\\ 2\\ 21\\ 2\\ 22\\ 22\\ 22\\ 23\\ 3\\ \end{array}$	0.555 0.69 0.82 0.96 1.124 1.37 1.51 1.65 1.79 2.206 2.34 2.47 2.47 2.75 2.89 3.30 3.34 3.57 3.44 3.57 3.71 3.89 4.40 4.54 4.67 4.67 4.67 5.50 5.50 5.50 5.50 5.50 6.19 6.32	Hrs. 30½ 31 32½ 331 33½ 33½ 33½ 33½ 33½ 33½ 33½ 33½ 33	8.39 8.52 8.66 8.80 8.94 9.07 9.21 9.35 9.49 9.62 9.76 9.90 10.45 10.52 10.86 11.00 11.14 11.55 11.69 11.82 11.90 12.24 12.51 12.65 12.79 12.92 13.06 13.20 13.34 13.47 13.67 13.67 13.75 13.89 14.16 14.16 14.16 14.16 14.16 14.16 14.16 14.16 14.16 14.16 14.16 14.16 14.16 14.16	Hrs.  601 61 61 62 62 62 62 63 63 64 65 66 66 66 67 70 70 70 71 71 71 71 71 71 71 71 71 71 71 71 71	Am't.  16.64 16.77 16.91 17.05 17.19 17.32 17.46 17.74 17.87 18.01 18.15 18.29 18.42 18.56 18.70 18.84 18.97 19.52 19.66 19.90 19.94 20.07 20.21 20.35 20.40 21.17 21.31 21.45 21.59 21.76 22.00 22.14 22.27 22.41 22.55	Hrs.	Am't
110 29.15 110 29.28 111 29.41 111 29.55 112 29.68 112 29.68	20 20 21 21 21 22 22	5.40 5.53 5.67 5.80 5.94 6.21 6.34 6.61 6.78 7.29 7.42 7.56 7.69 7.83 7.96	50 13.50 502 13.63 51 13.77 512 13.90 52 14.04 52 14.17 53 14.31 532 14.45 542 14.51 552 14.98 56 15.12 562 15.52 57 15.39 572 15.52 58 15.66 582 15.79 592 16.06	80 80 80 80 80 80 80 80 80 80 80 80 80 8	21.60 21.73 21.87 22.00 22.14 22.27 22.41 22.54 22.68 22.81 22.95 23.08 23.22 23.35 23.49 23.62 23.76 23.76 23.76 23.86 24.03 24.03	$\begin{array}{c cccc} 110 & 29 \\ 110 & 29 \\ 111 & 29 \\ 111 & 30 \\ 112 & 30 \\ 112 & 30 \\ \end{array}$	.70 .83 .97 .10 .24 .37 .51 .64 .78 .91 .05 .18 .32 .45 .59 .72 .86 .99 .13	20 20 21 21 21 22 22 23 23 24 24 25 26 26 26 26 26 27 28 28 29 29 29 29 29 29 29 29 29 29	5.50 5.64 5.77 5.91 6.05 6.05 6.46 6.60 6.74 7.01 7.15 7.29 7.42 7.56 7.70 7.84 7.97	50 12 51 12 52 13 53 12 53 12 55 6 12 56 1	13.75 13.89 14.02 14.16 14.30 14.44	80 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	22.00 22.14 22.27 22.41 22.55 22.69	110 111 111 111 111 111 111 111 111 111	30. 25 30. 39 30. 52 30. 66 30. 86 31. 07 31. 21 31. 35 31. 49 31. 76 31. 76 32. 04 32. 17 32. 31 32. 45 32. 72 32. 86

Hrs. A

 $\begin{array}{c} \mathbf{2} \\ \mathbf{2} \\ \mathbf{3} \\ \mathbf{3} \\ \mathbf{3} \\ \mathbf{2} \\ \mathbf{3} \\ \mathbf{3} \\ \mathbf{2} \\ \mathbf{3} \\ \mathbf{4} \\ \mathbf{4} \\ \mathbf{2} \\ \mathbf{5} \\ \mathbf{5} \\ \mathbf{5} \\ \mathbf{6} \\ \mathbf{6} \\ \mathbf{7} \\ \mathbf{7} \\ \mathbf{7} \\ \mathbf{8} \\ \mathbf{8} \\ \mathbf{12} \\ \mathbf{9} \\ \mathbf{9} \\ \mathbf{13} \\ \mathbf{10} \\ \mathbf{10} \\ \mathbf{12} \\ \mathbf{12} \\ \mathbf{12} \\ \mathbf{13} \\ \mathbf{13} \\ \mathbf{14} \\ \mathbf{12} \\ \mathbf{15} \\ \mathbf{15} \\ \mathbf{16} \\ \mathbf{16} \\ \mathbf{17} \\ \mathbf{12} \\ \mathbf{18} \\ \mathbf{19} \\$ 

20 20½ 21  $21\frac{1}{2}$ 

22<sup>1</sup>
22<sup>1</sup>
23<sup>1</sup>
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OUR.			S PER HO			29½ CEN'		
Hrs. Am ?	Irs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.
Hrs. Ant 5  901 25.79 91 25.93 912 26.98 92 26.22 921 26.36 93 26.50 932 26.50 932 26.65 94 26.79 941 26.93 951 27.50 96 27.36 961 27.50 97 27.64 972 27.79 98 27.93 981 28.07 99 28.21 100 28.56 100 28.56 100 28.56 101 28.78 101 28.78 101 28.78 101 28.78 101 28.78 101 29.97 1021 29.97 103 29.35 104 20.64 104 20.64 104 20.64 105 30.35 107 30.49 107 30.49 107 30.49 107 30.49 108 30.78 108 30.78 109 31.06	Irs. Anı't.  2 0.58 21 0.72 3 0.87 31 1.01 4 1.30 4 1.30 5 1.45 5 1.45 5 1.45 6 1.30 7 2 2.37 8 2 2.32 8 1 2.46 9 2.61 9 3 2.90 10 2.90 10 2.90 10 3.04 11 3.39 112 3.38 12 3.62 13 3.77 131 3.94 14 4.06 141 4.78 17 4.93 172 5.07 181 5.52 181 5.52 181 5.52 181 5.52 181 5.52 181 5.55 191 5.65 20 6.09 211 6.09	IIrs.   Am't.   30½   8.84   8.99   31½   9.13   9.28   32½   9.42   33   9.57   33½   10.00   35½   10.29   36   10.15   35½   10.29   36   10.15   37½   10.87   38½   11.16   39½   11.16   39½   11.16   39½   11.16   39½   11.16   39½   11.16   41½   12.03   42½   12.18   42½   12.18   42½   12.18   42½   12.19   45½   13.05   45½   13.48   47½   13.63   47½   13.77   48½   14.64   14.21   49½   14.21   49½   14.35   50½   14.50   50½   14.64   51½   14.93   14.93   14.93   14.93   14.93   14.93   14.94   14.51   14.79   51½   14.93   14.93   14.93   14.93   14.94   14.95   14.95   14.93	Hrs.   Am't.     60½   17.54     61½   17.63     62½   17.98     62½   18.12     63	Hrs. Am't.  90½ 26.24 91 26.39 91½ 26.68 92½ 26.68 92½ 26.97 93½ 27.11 94 27.26 96 27.84 96½ 27.98 97½ 28.27 98 28.13 97½ 28.27 98 28.42 98½ 28.56 99 28.71 99½ 29.00 100½ 29.14 101 29.29 101½ 29.58 102½ 29.58 102½ 29.72 103 29.68 102½ 30.59 106 30.74 106½ 30.30 105½ 30.45 105½ 30.45 105½ 30.59 106 30.74 106½ 30.88 107½ 31.03 107½ 31.75 108 31.32 108½ 31.46 109½ 31.75 110 31.90	1 Irs. Am't.  2 0.59 2 1: 0.74 3 1.18 3 1.13 4 1.13 4 1.13 5 1.47 1.62 1.77 1.92 2.06 6 1: 1.47 1.92 2.06 1.17 1.92 2.06 1.17 1.92 2.05 2.31 2.05 2.31 2.05 2.31 2.05 2.31 2.05 2.31 2.05 2.31 2.05 2.31 2.05 2.31 2.05 2.31 2.05 2.31 2.05 2.31 2.05 2.31 2.05 2.31 2.05 2.31 2.05 2.31 2.05 2.31 2.05 2.05 2.05 2.05 2.05 2.05 2.05 2.05	Hrs.   Am't.	Hrs. Ani't.    60½   17.85     61½   18.199     61½   18.144     62   18.29     63½   18.58     63½   18.73     64½   19.03     65½   65½     65½   19.47     66½   20.21     69   20.35     69½   20.35     69½   20.35     69½   20.35     70½   20.80     71½   21.24     71½   21.29     72½   21.24     72½   21.24     72½   21.24     72½   22.57     77½   22.71     77½   22.30     78½   23.30     79½   23.45     80½   23.75     81½   24.04     8	Hrs.   Am't.
108½ 30.02 109   31.06 109½ 31.21 110   31.35 110½ 31.40 111   31.63	18½ 5.36 19 5.51 19½ 5.65 20 5.80 21 6.23 22 6.38 22½ 6.52 23 6.65 24½ 7.10 25 7.25 25½ 7.25 26 7.53 27 7.68 27 7.68 27 7.68 27 7.68 27 7.68 27 7.68 27 7.68 27 7.68 27 7.68 27 7.68 28½ 26 29 41 20½ 8.55	48½ 14.06 49 14.21 49½ 14.35 50 14.64 51 14.79 51½ 14.93 52 15.08 52½ 15.22 15.37 53½ 15.51 54 15.51 55½ 15.66 54½ 15.80 55½ 16.66 55½ 16.63 56½ 16.53 56½ 16.53 57½ 16.67 58½ 16.53 16.53 16.53 17.11 59½ 17.25	78½ 22.76 79 22.91 79½ 23.05 80½ 23.20 80½ 23.34 81 23.49 81½ 23.63 82 23.78 82½ 23.92 83½ 24.07 83½ 24.21 84½ 24.50 85½ 24.79 86½ 25.08 87 25.23 87½ 25.37 88½ 25.56 89 25.88	108½ 31.46 109½ 31.61 109½ 31.75 110 31.90 110½ 32.04 111 32.19 111½ 32.33 112 32.48 112½ 32.62 113 32.77 113½ 32.91 114 33.06 114½ 33.20 115 33.35 115½ 33.49 116½ 33.78 117 33.93 117½ 34.07 118½ 34.26 119⅓ 34.51 110⅓ 34.65	18½ 5.46 19 19½ 5.75 20 0.05 21 6.19 2½ 6.34 22 1 6.49 22½ 6.64 23½ 7.08 24¼ 7.23 7.52 25½ 7.57 26½ 7.67 26½ 7.67 28 8.11 28 8.26 28½ 8.41 8.85 20⅓ 8.70	48½ 14.31   49½ 14.45   40½ 14.45   50½ 14.45   50½ 14.90   51½ 15.19   52½ 15.34   52½ 15.63   53½ 15.63   53½ 15.63   53½ 16.07   55½ 16.22   55½ 16.67   56½ 16.67   57½ 16.67   57½ 16.52   56½ 17.11   58½ 17.26   59½ 17.55	78.1 23.16 79 23.30 79.1 23.45 80.1 23.66 80.1 23.75 81 23.66 81.2 24.04 82 24.19 82.4 34 83.1 24.63 84.2 24.78 84.2 24.78 84.2 24.78 85.1 25.07 85.2 25.06 87.1 25.81 88 25.01 88 25.01 88 25.01 88 25.01 89 26.25 89 26.40	108½ 32.01 109 32.15 109½ 32.30 110 32.45 110½ 32.60 111 32.74

AT	30 CENT	S PER HO	OUR.	AT 8	30½ CENT	S PER H	OUR.
Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am t.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't
2 0.60 2½ 0.60 2½ 0.90 3½ 1.05 4 1.20 4½ 1.35 5½ 1.65 6 1.80 1.95 7 2.10 7½ 2.25 8 2.40 8½ 2.55 9 2.26 3.00 10½ 3.15 11 3.30 11½ 3.45 12 3.60 12⅓ 3.45 11¼ 3.30 11½ 3.45 12 3.60 12⅓ 3.75 13ٰ 3.90 13⅓ 4.05 14ٰ 4.20 14⅓ 4.35 15⅓ 4.65 16ٰ 4.80 16⅓ 5.55 19 5.55 19 5.57 19⅓ 5.55 19 5.70 19⅓ 5.55 19 6.30	Hrs.   Am't.	Hrs. Am't.    60½   18.15   61½   18.35   62½   18.45   62½   18.90   63½   19.05   64½   19.35   65½   19.50   65½   19.65   66½   20.10   67½   20.25   68½   20.40   68½   20.55   69½   20.70   20.85   70   21.15   71   21.30   71½   21.45   72   21.60   72½   21.60   72½   22.50   74½   22.35   75½   22.65   76½   22.95   77½   23.55   79½   23.75   79½   23.75   79½   23.75   79½   23.75   79½   23.75   79½   23.75   79½   23.75   79½   23.75   24.00   80½   24.15   81   24.30	Hrs. Am't.  90½ 27.15 91 27.45 92 27.60 92½ 27.75 93 27.90 93½ 28.65 94 28.25 95 28.65 95½ 28.65 96½ 28.80 96⅓ 28.95 97½ 29.25 98 29.40 98½ 29.55 99 29.70 100⅓ 30.45 101⅓ 30.45 102⅓ 30.45 102⅓ 30.45 102⅓ 30.45 102⅓ 30.55 104⅙ 31.20 104⅙ 31.35 105⅓ 31.55	2 0.61 21 0.76 31 1.07 41 1.37 51 1.68 6 1.83 6 1.83 6 1.83 6 1.83 6 1.93 2.13 71 2.29 2.74 92 2.74 92 2.74 92 3.05 102 3.35 112 3.66 112 3.51 12 3.66 112 3.90 11 3.35 112 3.66 113 3.96 114 4.27 142 4.42 15 1.52 14 4.27 142 4.42 15 5.38 171 5.34 18 5.49 181 5.64 19 5.79 191 5.95 20 6.10 201 6.25 21 6.40	Hrs. Am't.  30½ 31 9.30 31 9.45 31½ 9.61 32 9.76 32½ 9.91 33 10.06 33⅓ 10.22 35 10.67 35½ 10.67 35½ 10.69 38½ 11.13 37 11.28 37½ 11.44 39 11.29 38½ 11.74 39 12.50 41½ 13.57 45½	Hrs. Am't.    60½   18.45     61½   18.76     62½   18.91     62½   19.06     63½   19.21     63½   19.37     64½   19.67     65½   19.82     65½   19.98     66½   20.28     20.43     67½   20.59     68½   20.43     67½   20.59     68½   21.04     69½   21.04     21.20     70½   21.50     71½   21.50     71½   21.81     72½   22.11     72½   22.25     74½   22.72     75½   22.72     75½   23.38     76½   23.38     76½   23.48     77½   23.94     79½   24.25     80½   24.55	Hrs.   Am't
20   6.00 20   6.15 21   6.30 21   6.45 22   6.60 22   6.75	50 15.00 502 15.15	80 24.00 80 24.15	110 33.00 110 <u>1</u> 33.15	20 $6.10$ $20\frac{1}{2}$ $6.25$	50 15.25 50½ 15.40	80 24.40 80 24.55	110 33.55 110 <sup>1</sup> 33.70
23   6.90 232   7.05 24   7.20 242   7.35 25   7.50 252   7.65 262   7.95	53   15.90 532   16.05 54   16.20 542   16.35 55   16.50 552   16.65 56   16.80 562   16.95	83   24,90 832   25,05 84   25,20 842   25,35 85   25,50 852   25,65 86   25,80 862   25,95	$\begin{array}{c} \textbf{113} & 33.90 \\ \textbf{113} & 34.05 \\ \textbf{114} & 34.20 \\ \textbf{114} & 34.35 \\ \textbf{115} & 34.50 \\ \textbf{115} & 34.65 \\ \textbf{116} & 34.80 \\ \textbf{116} & 34.95 \\ \end{array}$	$\begin{array}{c cccc} 23 & 7.01 \\ 23\frac{1}{2} & 7.17 \\ 24 & 7.32 \\ 24\frac{1}{2} & 7.47 \\ 25 & 7.62 \\ 25\frac{1}{2} & 7.78 \\ 26 & 7.93 \\ 26\frac{1}{2} & 8.08 \\ \end{array}$	53 16.16 53½ 16.32 54 16.47 54½ 16.62 55 16.77 55½ 16.93 56 17.08 56½ 17.23	83 25.31 83 25.47 84 25.62 84 25.77	113 34.46 113½ 34.62 114 34.77 114½ 34.92 115 35.07 115½ 35.23 116 35.38 116½ 35.53
27   8.10 27   8.25 28   8.40 28   8.55 29   8.70 29   29   29   29   29   29   29   29	57 17.10 572 17.25 58 17.40 582 17.55 59 17.70 592 17.85	87   26,10 87   26,25 88   26,40 88   26,55 89   26,70 89   26,85	117   35.10 117   35.25 118   35.40 118   35.55 119   35.70 119   35.85 120   36.00	27 8.23 27 8.39 28 8.54 28 8.69 29 8.84 29 9.00 30 9.15	57 17.38 57 17.54 58 17.69 58 17.84 59 17.99 59 18.15 60 18.30	87 26.53 87½ 26.69 88 26.84 88½ 26.99 89 27.14 89½ 27.30	117 35.68 117 35.84 118 35.99 118 36.14 119 36.45 120 36.60

			AT 94 CENTS DED HOUR														
ŀ	IOUR.		AT	31 (	CENTS	S PER HOUR.					AT	31½	CENT	S P	ER H	OUR	
n't	Hrs. Am't	Hrs	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Anı't.	Hrs.	Am't.
1 4506 196 1 7 2 7 2 8 3 8 4 3 9 4 9 4 9 5 0 5 0 6 1 6 1 7 2 7 2 8 3 3 5 7 4 9 4 9 5 0 5 0 6 1 6 1 7 2 7 2 8 3		Hrs.  2: 3: 3: 4: 4: 5: 5: 6: 7: 8: 9: 9: 9: 10: 11: 12: 13: 13: 14: 15: 15: 16: 16: 17: 17: 18: 18: 19: 20: 20: 21: 22: 23: 24: 24: 25: 25: 25: 25: 25: 25: 25: 25: 25: 25	0.62 0.77 0.93 1.08 1.39 1.55 1.70 1.86 2.01 2.17 2.32 2.48 2.63 2.79 2.94 3.25 3.41 3.56 3.72 3.41 3.56 3.72 3.41 3.56 5.11 5.27 4.80 4.96 5.11 5.55 5.73 5.89 6.05 6.65 6.65 6.65 6.65 7.72 7.79 7.79 7.79 7.79 7.79 7.79 7.79	Hrs.	Am't.  9.45 9.61 9.76 9.92 10.07 10.23 10.54 10.69 10.85 11.00 11.16 11.31 11.47 11.62 11.78 11.93 12.09 12.21 12.86 13.02 13.17 13.33 13.48 13.64 13.79 13.95 14.10 14.26 14.41 14.57 14.72 14.88 15.03	Hrs.	Am't.  18,75 18,91 19,06 19,22 19,37 19,53 19,68 19,84 19,99 20,15 20,30 20,46 20,61 20,77 20,92 21,08 21,23 21,39 21,39 21,39 21,39 22,18 22,27 22,63 22,27 22,63 22,37 22,36 23,56 23,71 23,87 24,49 24,64 24,48 24,48 24,48 24,48 24,48 25,57 25,73 25,88 26,04 26,50	Hrs.	28. 05 28. 21 28. 36 28. 52 28. 67 28. 83 29. 14 29. 29. 45 29. 45 29. 91 30. 07 30. 22 30. 53 30. 53 30. 53 31. 15 31. 31 31. 31 31. 31 31. 93 32. 28 32. 28 32. 39	$\begin{array}{c} 2 & 1 & 2 & 2 & 1 & 2 & 3 & 1 & 3 & 4 & 4 & 1 & 4 & 5 & 5 & 5 & 6 & 6 & 2 & 7 & 7 & 8 & 8 & 5 & 5 & 6 & 6 & 2 & 7 & 7 & 6 & 6 & 2 & 7 & 7 & 6 & 6 & 2 & 7 & 7 & 6 & 6 & 2 & 7 & 7 & 6 & 6 & 2 & 7 & 7 & 6 & 6 & 2 & 7 & 7 & 6 & 6 & 2 & 7 & 7 & 6 & 6 & 7 & 7 & 7 & 6 & 6 & 7 & $	0.63 0.79 0.94 1.10 1.26 1.42 1.73 1.89 2.05 2.20 2.36 2.32 2.36 2.33 3.46 3.62 3.78 3.94 4.09 4.25 4.41 4.57 4.72 4.83 5.04 5.520 5.53 5.51 5.64 6.67 6.67 6.77 6.93 7.09 7.72 7.78 7	Hrs. 30½ 31½ 33½ 33½ 33½ 33½ 33½ 33½ 33½ 33½ 33	Am't.	Hrs.   60½   61½   62½   63½   64½   64½   65½   666½   666½   67½   68   665½   77½   72½   77½   77½   77½   77½   77½   80½   81½   82½   82½   82½   82½   82½   82½   85½   8	Am't.  19.06 19.21 19.37 19.53 19.69 19.84 20.00 20.16 20.32 20.47 20.63 20.79 20.95 21.10 21.26 21.26 21.26 21.26 22.21 22.36 22.22 22.68 22.23 22.68 22.47 23.62 23.31 23.47 23.62 23.78 24.41 24.57 24.48 25.04 25.50 25.51 25.63	Hrs.	28.51 28.56 28.98.82 29.45 29.45 29.77 230.08 300.24 300.551 300.77 300.78 300.

	AT	32 (	CENT	S PE	RHO	UR.			AT	<b>32</b> ½	CEN'	rs p	ER H	OUR	•
H-3.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hirs.	Am't.	Hrs.	Am't,	Hrs.	Am't,	Hrs	Am't.	Hrs.	Am'i,
		301	9.76	601	19.36	901	28.96			301	9.91	60.1	19.66	901	29.11
		31	9.92	61	19.52	91	29,12			31	10.07	61	19.82	91	29.57
_	0.64	31 1/2			19.68	11 ' -1	29.28		0.65		10.24	615	19.99		29.74
2 2 3	0.64	$\frac{32}{32\frac{1}{2}}$	10.24	62	19.84 20.00	11 ~ _1	29.44 29.60	$\frac{2}{2\frac{1}{2}}$	0.81	32 32 \frac{1}{2}	10.40	1	20.15	92	<b>2</b> 9.90 <b>3</b> 0.06
3.	0.96	33	10.50	63	20.16		29.76	3	0.97	33	10.72		20.47	93	30.22
$\frac{3}{3}$	1.12	333	10.72		20.32		29.92	32	1.14	332	10.89		20.64		30.39
4	1.28	34	10.88	64	20.48		30.08	-4	1.30	34	11.05	64	20.80	94	30.55
42	1.44	$34\frac{1}{2}$	11.04	T21	20.64		30.24	45	1.46	342	11.21		20.96		30.71
$\frac{5}{5^{\frac{1}{2}}}$	1.60	35 35g	11.36	65	20,80 20,96		30,40 30,56	5 53	1.79	35 35 2	11.37	65	21,12	95	30,87
$\frac{52}{6}$	1.92	36	11.52	66	21.12		30.72	6	1.95	36	11.70	66	21.45	952	31.04
63	2.08	361	11.68		21,28		30.88	-63	2.11	361	11.86	1 .	21,01		31.35
7	2.24	37	11.84		21.44	97	31 04	7.	2.27	37	12.02	67	21.77	97	31.52
75	2.40		12.00		21.60	972	31.20	72	2.44	372	12,19		21.94		31.69
6	2.56	38	12.16		21.76	98	31.36	8	2.60	38	12.35	68	22.10	98	31.85
87	2.72	0 2	12.32		21.92	983	31.52 31.68	85	2.76	302	12.51	69	22.26		32.01 32.17
91	3.04	39 39 <sup>1</sup> / <sub>2</sub>	12.64	1 2.1	22,24	99	31.84	95	3.09	393	12.84		22.59		32.34
10	3.20	40	12.80		22.40		32.00	10	3.25	40	13.CO	70	22.75	100	32.50
$10^{1}_{2}$	3.36		12.96	703	22.56	1003	32, 16	$10\frac{1}{2}$	3.41	101	13.16		22.91	$100\frac{1}{2}$	32.00
11	3.52	41	13,12		22.72		32.32	II.	3.57	41,	13.32	71.	23.07	101	32.82
112	3.68		13.28	1 2	22.88		32.48	115	3.74		13.49		23.24	1017	
12	3.84		13.44		23.04	102	32.64 32.80	12	3.90	42	13.81	72	23.40	102	
125	4.00 4.16		13.76		23.20   23.30	1023	32.96	13	4.00	423	13.97	73	23.56 23.72	1025	33-31 33-47
135	4.32		13.92		23.52		33.12	135	4.39		14.14		23.89		33.64
14	4.48		14.08		23.68		33.28	14	4.55	44	14.30	7-1	24.05	104	33.80
145	4.64		14.21	741	23.84	1042	33-44	145	4.71	445	14.46	7.13	24.21	1042	33 90
15	4.80	45.	14.40	75,	24.00	105	33,60	15,	4.87	45,	14.62	75	24.37	105	
152	4.96		14.56		24.16	1051		153	5.04	452				1053	
16 16}	5.12	46 463	14.72		24.32 24.48	1063	33.92	16	5.20	46 <u>1</u>	14.95	76 761	24.70	106	31.45
17	5.44	47	15.04		21.64		34.24	17	5.52	47	15.27	77	25.02		34.77
174	5.60		15.20		24.80		34.40	173	5.69	475	15.44		25.19		34.94
18	5.76	48	15.36	78	24.96	108	34.56	18	5.85		15,00	78	25 35	108	35-10
183	5.92	485	15.52	785	25.12	1081	31.72	183	6.01		15.76		25.51	1081	35.26
19	6,08	49,	15.68		25.28	109	31.88	19	6.17	49	15.92		25.07	109	35- 12
195	6.40		15.84		25.44 25.60	1003	35.04 35.20	195	6.34	1/2	16,09		25.84	1093	
201	6.56	50 503	16.16		25.70	1103	25.26	201	6.66		16.41		26.16	110	35.75
21	6.72	51	16,32		25.02		35.52	21	6.82	51	16.57	64	26.32		36.07
211	6.88	513	16.48	1 21	26.68	11112	35.68	211	6.99	515	16.74		26.49	IIII	36,24
22	7.04	52	16.64		26,24	112	35.84	22	7.15		16.90		26,65		30 10
222	7.20	27 2	16,80		26, 10	1122	30.00	225	7.31	-	17.06		26.81	1123	
23	7.30	53	16.96		26,56 26,72		36.16; 36.32	23	7.47		17.22		26.97 27.14		36,72
21	7.68	53½ 54	17.28		26,88	1 av 1	30.32 36.48	2.1	7.80		17.55		27.30	1135	37. 15
215	7.8.1		17.44		27.04	1146	36.64	2.11	7.96		17.71	8.13	27.46	1142	37 21
25	8,00	55	17.60	1 60	27.20	115	36,80	25	8.12	55	17.87	85	27.62	115	37 37
255	8.16	555	17.76		27.36	1152	36.95	251	8.29	552	18.04	851	27.79	1153	37-54
261	8.32	56	17.92		27.52	116	37.12	26	8.45		18.20		27.95	116	37.70
261		50	18,08		27.68 27.84		37.28	264	8.61		18.36		28.11	1161	
27	8.64 8.80	57	18.40		28.00	117		27 272	8.77		18.52   18.69		28.27 28.44	117	38.02
25	8.05		18.56		28.16	118		28	9.10		18.85		28,60		38. 35
284	- 1	583	18.72		28.32	1181	37.92	281	9.26		19.01		28.76	1181	
29	9.28	59	13.88		28.48		38.08	20	9.42	59	19.17	89	28.92	110	38 67
295	9.44		19.04		28.64	1107	38 24	293	9.59		19.34		29.09	119	
30 !	9.60	UC .	19.20	90	28.80	120	30 40 1	30	9.75 !	00	19.50	90	29.25	120	30 00

H	OUR.		R							OUR.		1	AT :	33½	CEN'	rs p	ER H	OUR	
	Hrs.	Am'a		Hrs.	Am't.	Hrs.	Am'ı.	Hrs.	Am't	Hrs.	Am't.	His.	Am t.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am'
0	901	29.41					10.06		19.96	901	29.86			301	10.22	60 l	20.27	903	30.32
21	91	29.57				31	10.23		20,13		30.03			31	10.38		20.43	91	30.48
	912	29.74 29.90		2	0.66	32	10.39		20.29 20.46	915	30, 19	2	0.67		10.55	62	20.00	915	30.65
2		30.06	ш.	25	0.82		10.72		20.02		30.52	23	0.84	32	10.89	11	20.94	92	
,	93	30.22		3,1	0.99	33	10.89		20.79	93	30.69	3.	1.00	33.	11.05		21.10	93	31.15
		30. (9		4	1.15	332	11.05		20,95 21.12		30.85	32	1.17	335	11.22	_ ~ ~	21.27		31,32
	94	30.55	1	45	1.48		11.38		21.12	94	31.18	4 4 2	1.34	34	11.39	64	21.44	94	31.66
:	95	30,87		5	1.65	35,	11.55	65.	21.45	95	31.35	5	1 67	35	11.72	65	21.77	95	31.82
		31.04		5.1	1.81		11.71		21.61	952	31.51	52	1 8.4	0.2	11.89		21.94	1 15 1	31.99
)	96	31.20	3	61	2.14		12,04	66.	21.78 21.91	961	31,68 31,84	6. 6.	2.01	363	12,00	661	22,11	90	32,16 32,33
,	1 -	31.52		7,	2.31		12 21		22, 11	97	32,01	7	2.34	37	12.39	. 04	22.44	97	32.49
-	971	31.69		75	2.47		12.37		22.27	975	32.17	74	2.51	37 5	12.50		22.61		32,66
)	98	31,85		8.	2.80		12.54 12.70	681	22,44 22,60	98	32, 34 32, 50	81	2.68	38	12.73		22.78	98	32.83 33.co
1 01	903	32.01 32.17	10	9	2.97	39	12.87		22.77		32.67	9	3.01	39	13.05		23.11	99	33.16
)		32.34	и.	95	3.13		13.03	695	22.93	991	32,83	$9\frac{1}{2}$	3.18		13,23	691	23.28	993	33-33
í	100	32.50	10.	101	3.30	1 1	13.20		23, 10		33.00	10	3.35	40	13.40	70	23.45	100	33.50
,		32.82	10	11	3.63		13.53		23, 26 23, 43	100	33.10 33.33	105	3.52	405	13.57		23.62 23.78	Tot	33.67 33.83
-		32.49		112	3.79	412	13.69	1 1	23.50	11 +1	33.49	110	3.85		13.90		23.95	1015	
)	102	133.15	16	12	3.95		13 86	72	23.70	102	33.	12	4.02		14.74		24.12	102	34-17
)		33.31	E0.		4.29		14.10		23.92 24.09	102 \		123	4.19		14 24		24.45	102 1	34-34 34-50
)		33-47	в.	13.	4-45		14.35		21.25	1031	34.15	135	4.52		14.57		24.62		34.07
5	104	33.80		14	1 02		14.52	1 1 1	24.42	10.1	34.32	1.4	4.69	44	14.74	1 1	21.70	104	34,84
		33 96	100		1.78		14.68		24.58 24.75	1045		145	4.86		14.91		24.90		35.01
		34.12	н.	155	5.11	- 11	15.01		34.75	105	34.65 34.81	15	5.02		15. 7 15.24		25.12 25.29	105	35.17 35.34
0		31-15	111	16	5.28		15.18	76	25.08	106	34.98	16	5.36	45	15.41	76	25.46	106	35.51
9		34.51	и.	17	5.44		15.34		25. 2.1	106	35.14	165	5.53	465	15.58		25.03	1	35,68
2	107		и.	171	5.77		15.51	77 3	25.41 25.57	107	35.31	17	5,69		15.74		25.79 25.95	107	35.84 36.01
	108	35.10	и.	18	5.04	48	15.84	78	25.74	108	35.64	18	6.03		16.08		26,13	108	36.18
	_	35.26	и.	-	6.10		16.00		25.90	1085	35.80	181	0.20	* ***	16.25		26, 30		36 35
	100	35-42 35-59	и.		5.43	1	16.33		26. 07   26. 23	109	35.97	107	6.36		16.41   16.58		26.46 26.63	100	36.51 36.68
)		35.75	ы.	20	6.60	50	16.50	80	26,40	110		20	6.70		10.75		26.8e	110	36.85
5	_	35.91	88		6.76		16.66	803	25.56	1103		201	6.87	503	16.92		26.97		37.02
01		36.24	OIL	213	7.09		16.83		26.73 26.89	1119	36.03	21	7.03	1 1	17.08	81 8	27.13 27.30	111	37.18 37.35
-		36 10	m	22	7.26	52	17.16	82	7.05		36,96	22	7.37	91 40	17.42	0 -	27.47	112	37.52
		36, 56	ы.	221	7.12		17.32		27.22	112	37.12	225	7-5-1		17.59		27.64	1123	37.69
		30,72	10	235	7 59 7 7 7 5		17.49 17.65		27.39		37.29	23	7.70		17.75	~~	27.80	113	37.65
5	114	36.89	ш.	21	7.92		17.82	0	27.55 27.72	1135	37.15	232	7.87 8.04		17.92 18.09		27.97 28.14	1135	38.02
3		37 31	10.		8,08		17.98	8.10	27.88	1143	37.78	245	8.21		18.26		28.31		38,36
		37 37	ы.	254	8.25 8.41	55	18.15 18.31	85	28.05	115	37-95	25	8.37	55	18.42		28.47		
-	11153	37.54	10	20	8, 58		18.48	86	28. 21 28. 38	1155	38, 28	252	8.54	550	18, 59 18, 76		28.6.µ 28.81		38.86
ì	116	37.86	10		8.74	565	18.54	865	28, 54	1165	8.44	263	8.88		18.93		28.98		39.03
7	117	38.02	W	27 273	9.07	57	18,81	87 3	28.71	117	38.61	27	9.04	57	19.09		29.14	117	SU-10
1		38.35	ы	28	9.21		19.14	88	28.87	1175		271	9.21		19.26		29.31		39.53
)		38 51	M		9 40	583	19.30	885 2	29.20	1187	39, 10	281	9.55		19.60				39.53
2	119	38 67	9	201	9-57	59,	10.47	3	29.37	119	G. 27	20	9.71	59	19.76	89	29.81	119	39 86
)		38 84		30	9.90		In tig	8932	9.53	1194	39-13 19-60	293	9.88		19.93		29.98		
5	11120	13.100	10	- Dest		-	2	90	1.70	120	1001	30	10.05	(() 12	20, 10	90	50. 1511	120	40.20

	AT	34 (	CENT	S PER HO	OUR.		1	AT 3	34½	CEN	TS P	ER H	OUR.	
Hrs.	Am't.	Hrs.	Am'ı.	Hrs. Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't,	Hrs.	Am'ı.	Hrs. And	Hrs \
		301	10.37	60 20.57	905	30.77			30.	10.52	603	20.87	90 31.22	
	1	31	10, 54	61 20.74	91	30.94			31	10.69	61	21.0.1	91 31.39	
		312	10.71	61½ 20.91	915	31.11				10.87	615	21.22	912 31.57	2 4
2	0.68	32,	10,88	62 21.08	92,	31.28		0.69	32	11.04	62	21.39	92 31.74	23
2	0.85	321	11.05	62 21.42	- 10	31.45	25	0,86		11.21	625	21.56	92 31.91	3.
35	1.19	33 33 <sup>1</sup> / <sub>2</sub>	11.39	63 21.42	93	31.79	3 <sub>1</sub> 3½	1.21	33	11.56		21.91	93 32.08 932 32.26	31
4	1.36	34	11.56	64 21.76	932	31.96		1.38		11.73	64	22.08	94 32.43	4
43	1.53	345	11.73	641 21.93	941	32.13	43	1.55		11.90		22.25	942 32 00	4
5.	1.70	35	11.90	65 22.10	95.	32.30	5	1.72	00.1	12.07		22.42	95. 32.77	5 5 1
5 52	1.87	352	12.07	652 22.27	955	32.47	52	1.90	0.	12,25		:,2,60	955 32.95	201
6	2.04	36	12.24	661 22.44	96	32.64	6 6	2.07		12.42		22.7 <b>7</b> 22.94	96 33.12	6.5
6!. 7	2.21	365	12.58	66½ 22.61	903	32.98	7	2.24		12.76		23.11	961 33.29	7
71	2.55	37 37 <sup>1</sup> / <sub>2</sub>	12.75	671 22.95	971	33.15	71	2.59	371	12,94		23.29	97 33.64	72 -
7 <sup>1</sup> / <sub>8</sub>	2.72	38	12.02	68 23,12	98	33.32	8	2.76		13.11		23.46	98 33.81	8 . 8
81	2.89	381	13.09	681 23.29	981	33.49	81	2.93	384	13.28		23.63	981 33 98	9
9,	3.06	39	13 20	69 23.46	99	33.00	9	3.10		13.45	69	23.80	99, 34-15	05 5
92	3.23	0 / 2	13.43	695 23.63		33.83	$9^{\frac{1}{2}}$	3,28	395	13.63		23.98	99 31 31	10
10	3.40	40	13.00	70 23.80	100	34.00 34.17	10.	3.45		13.80		24.15 24.32	100 34.50	102
11	3.57	405	13.77	71 24.14		34.34	11	3.79		14.14		24.49	TOT 34.84	11
113	3.91		14.11	713 24.31	10.3	34.51	113	3.97		14.32		21.67	101 2 35.02	47
12	4.08		14.28	72 24.48	102	34.68	12	4.14	.12	14.49		24.84	102 35.10	12
122	4.25	425	14.45	721 24.05	102	34.85	125	4.31		14.00		25.01	1021 35.30	13 4
13	4.42	43.	14.62	73 24.82		35.02	13,	4.48		14.83		25.18	103, 35.53	132 4
135	4.59	14/6	14.79	735 24.99		35.19	132	4.66		15.01		25 50	103 35.71	14 1
14	4.70		14.90	74   35, 16   745   25, 33		35•3 <sup>6</sup> 35•53	143	5.00		15.35	74	25.53 25.70	104 35.88	11 5
15	5.10		15.13	745 25.33 75 25.50		35.7	15	5.17		15.52	75	25.87	105 36.22	15, 5
155	5.27	100	15.47	751 25.67		35.87	155	5.35	455	15.70		26.05	105 36.40	15, 1
16	5-44	46	15,64	76 25.84	100	30.04	16	5.52	46	15.87	0	20.22	106 30 57	10,
163	5.61		15.81	761 26.01		36.21	$16_{2}^{1}$	5.60		16.04		26,39	1062 36.74	17
17	5.78	47,	15.98	77 26,18		36.38	17	5.80	77	16.2.		26.56	107 30 91	171 6
175	5.95		16.15	77 26.35		30.55 36.72	175	6.21		16. 39   16. 56		26,74	107 3 37.09	18 (
181	6.20		16.49	78 26.69		36.89	183	0,38		16.73		27.08	109 37.43	181,
10	6.46		16,66	79 20.86		37.00	10	6.55		16.90		27.25	109 37.00	11
105	6.63		16,83	795 27.03	1003	37.23	191	6.73		17.08	79!	27.43	109 37.78	$\frac{1}{20}$
20	6.80		17.00	80 27.20		37.40	50	6.90		17-25	1	27.60	110 37.95	20
20 1	6.07	17 6	17.17	801 27.37	1102		201	7.07		17.42		27.77	110 38.12	21
21	7.14	67	17.34	81 27.54		37·74   37·91	21 21 1	7. 24	~ .	17.59		27.04	111 38.29	21
22	7.18		17.68	82 27 88	112	38.08	22	7.59		17.94		28.29	112 38 04	22
224	7.65		17.85	823 28.05	1124		223	7.70	6.7	18.11		28.46	1121 38.81	29)
23	7. 2	53	18.02	83 28.22	113	38.42	23	7.1/3		18.28	83 3	28.63	113 38.98	231
231	- 1		18.19	831 28.39		38.59	231	8.11	P. 2 4 105 1	18.46	833		1135 30 16	21
2.1			18,36	84 28.56		38.76	34	8.25		18.63		28,98	114 30-33	211
(3.1	1		18.53	8.13 28.73		38.93	243	8.45 8.62		18.80	841		1145 30.50	2
251	8. 7	55	18.70	85 28.90	1153		251	8,80		18.97		29.32 29.50	115 30.07	25)
26	6.8.1		10 04	86 29.24		39-44	26	8.07		19.32		29.07	116 40 03	20
263	9.01		19.21	861 20.41	1165		261	9.14		19.40		207 81	1165 10 10	27
27	9.18		19.38	87 29.58		39.78	27	0.31		10.66	87 .	30.01	117 40.36	275
273	9.35		19.55	871 29.75	1175		271	9.49		19.84		30.19	1174 40 54	18
281	9.52		10.72	88 29.92		40, 12 40, 2.)	283	9.66		10.05	881	30,36	118 40.71 1184 40.88	2 1
209	9.69		19.89 20.06	80 30.26		10.49	(6)	0.83		20.18		30.53	110 41.05	20 1
	10 03	44. A. A. I.	20.23	891 30, 43		10.03		10.18		20.53	893	30,88	1195 41.23	771
	10.20		20.40				10	10.35		20.70		31.05	120 41.40	30 1

Hrs

AT	36 CENT	S PER HO	UR.	•	AT	36½ CEN	TS PER H	OUR.
Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs.	Δm't.	Hrs. Am't.	Hrs. Am't.	Hrs. An'i,
2 0.72 21 0.90 1.08 31 1.26 4 1.44 41 1.62 51 1.80 51 2.16 61 2.34 7 2.52 71 2.52 71 2.70 8 3.06 9 3.78 11 3.60 10 3.78 11 4.44 12 4.32 12 4.50 1.68	Hrs. Am't.    30½ (0.98     31	601 21.78 61 22.14 62 22.32 621 22.50 63 22.68 631 22.86 64 23.04 651 23.58 66 23.76 661 23.94 671 24.30 68 24.49 681 24.66 69 24.84 691 25.02 70 25.20 70 25.20 70 125.38 71 25.56 711 25.77 72 125.92 721 26.10 73 125.28	Hrs. Am't.  90-1 32-58 91 32-76 91-1 32-76 91-2 33-30 92 33-48 93 33-8 93 33-8 93 33-8 93 33-8 93 33-8 93 33-8 93 33-8 94 33-8 95 34-20 95 34-20 95 34-20 95 34-20 95 34-20 95 34-20 95 34-6 96 35-64 97 35-64 98 35-64 99 35-64 99 35-64 99 35-64 90 35-64 90 36-72	2 12 3 12 4 12 5 5 6 62 7 7 8 12 9 9 0 10 11 12 12 12 13	0.73 0.91 1.09 1.28 1.46 1.64 1.82 2.01 2.37 2.55 2.74 2.92 3.10 3.28 3.47 3.65 3.83 4 01 4 20 4.38 4.56 4.71	Hrs. Am't.  30½ 11.13 31½ 11.31 31½ 11.50 32 11.08 32½ 11.86 33½ 12.04 33½ 12.23 34½ 12.23 35½ 12.29 35 12.77 35½ 12.96 36 13.14 36½ 13.32 13.50 37½ 13.50 38½ 14.05 30 14.23 30½ 14.42 40 14.60 40½ 14.78 41½ 15.15 42½ 15.51 42½ 15.51	Hrs. Ant't.  60½ 22.08 61 22.26 61½ 22.45 02 22.63 02½ 22.81 63 22.99 03₺ 23.18 04 23.01 65 23.72 65₺ 23.72 65₺ 23.01 24.09 66₺ 24.27 24.45 67₺ 24.04 68 24.82 68₺ 25.00 69 25.18 69₺ 25.73 70 70 25.55 70 25.73 71 25.01 71₺ 26.10 72 26.28 72₺ 26.46	Hrs.   An   1,
12 4.32 123 4.50	42   15. 12   12. 12   15. 30   15. 48   15. 48   15. 48   16. 02   45. 16. 38   46. 16. 56   46. 16. 74   47   16. 92   47. 17. 10   48   17. 49   18. 30   50. 18.	72   25, 92   72½   20, 10   73   20, 28   73½   20, 82   75½   27, 36   76½   27, 54   27, 72   27, 90   28, 62   28, 41   70½   28, 62   28, 80   28, 41   20, 52   16   81½   20, 16	102 36.72 102 36.90	12   12   13   13   13   14   15   15   15   15   16   16   17   17   18   18   19   10   10   10   10   10   10   10	4.38 4.50	$42   15.33$ $42\frac{1}{2}   15.51$	72 26.28 721 26.46	102   37 23 102   37 41
25 9.00 253 9.18 26 9.30 254 0.54 27 0.72 273 9.00 28 10.08 284 to.26 20 10.44 20 10.62 30 10.80	55   10,80 55   10,38 56   20,16 56   20,24 57   20,52 57   20,70 56   20,88 58   20,88 59   21,24 50   21,24 50   21,42 60   21,60	85 30.00 853 30.78 80 30.78 80 31.14 87 31.32 87 31.50 88 31.68 88 31.86 80 22.04 80 32.22	115 41.40 115 41.58 116 41.76 110 41.76 110 41.04 117 42.30 118 42.48 118 42.96 119 42.84 110 42.84	25 25 20 20 20 27 27 27 28 1 28 1	9,12 9,31 0,40 9,67 9,85 0,04 0,28 0,78 77	55   20.07 55   20.26 56   20.44 50, 20.62 57   20.80 57   20.80 58   21.17 58   21.35 50   21.53 50   21.53 50   21.53	85 31,02 85 31,21 86 31,39 563 31,57 87 1 5 5 87 1 5 4 83 32,12 361 32,30 80 32,48 5 1.67	115 (1) 7 117 (1) 7 117 (1) 117 (1) 118 (1) 118 (1) 118 (1) 110 (1) (1) (1) (1) (1) (1) (1) (1) (1) (1)

HOUR.	AT	37 CENT	S PE	R H	OUR.	-		AT 3	37½	CENT	S P	ER H	OUR.	•
t. [Hrs.] Am't,	Hrs. Am't.	Hrs. Am't.	Hrs.	Am't-	Hrs. A	m't.	Hrs.	Ani t.	Hrs.	Am t.	Hrs.	Am't.	Hrs.	Am't.
1. Hrs. Andt.  8			Hrs.   601   62   63   63   64   65   65   66   67   68   68   69   67   68   69   67   68   69   67   68   69   67   68   69   69   69   69   69   69   69	Am't.  22.38 22.57 22.75 22.79 23.31 23.49 23.36 24.05 24.23 24.49 25.53 24.42 25.53 25.53 25.71 25.60 26.68 26.27 26.45 26.68 27.75 27.73 28.12 27.75 28.30 28.49 28.80 29.28 29.28 29.28 30.31 29.60 29.23 30.71 30.89 31.08 31.26	Hrs. A  90½ 33 91 33 91½ 33 92 34 92 34 93 34 94 34 94 34 94 35 95 35 96 35 96 35 96 35 96 35 96 35 97 36 98 36 98 36 99 36 90 36 90	3.48 3.47 3.48 3.47 4.22 4.41 4.59 4.78	$\begin{array}{c} 2 \\ 2 \\ 1 \\ 2 \\ 3 \\ 3 \\ 3 \\ 4 \\ 4 \\ 2 \\ 5 \\ 5 \\ 6 \\ 6 \\ 1 \\ 2 \\ 5 \\ 6 \\ 6 \\ 1 \\ 2 \\ 1 \\ 1 \\ 1 \\ 2 \\ 1 \\ $		301 313 3213 3313 3313 3313 3313 3313 33	Am t. 11.44 11.62 11.81 12.00	Hrs.   60	Am't.  22.69 22.87 23.06 23.25 23.44 23.62 23.81 24.00 24.137 24.56 24.75 24.94 25.12 25.50 25.69 25.87 26.06 26.25 26.44 26.62 27.19 27.37 27.56 27.75 27.94 28.12 28.50 28.87 29.06 29.28 30.00 30.10 30.50 30.75	Hrs.	Am't

10 | 11 | 11 | 12 | 12 |

23 23\ 24

25\ 20 20\ 20\

AT 38 CENT	S PER HO	OUR.	A	Г 38½ CEN	TS PER H	O R.
Hrs. Am't. Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am	t. Hrs. Am't.	Hrs. Am't.	His. Am't.
Hrs.   Am't.   Hrs.   Am't.	Hrs.   Am't.	90½ 34.39 91 34.58 91½ 34.96 91½ 34.96 92½ 35.15 93 35.34 93½ 35.53 94 35.91 95 36.29 96 36.48 96½ 36.67 97 36.86 97½ 37.05 98 37.24 98½ 37.43 99 37.62 99½ 37.81 100 38.00 100½ 38.19 101 38.38 101½ 38.57 102 38.76 102 38.76 103 39.14 103½ 39.52 104½ 30.7 105 40.28 106½ 40.47 107½ 40.85 108½ 41.04	11 1 2 2 3 3 4 4 1 1 1 1 2 2 1 4 4 4 4 4 4 4 4 4 4 4	*t.   Hrs.   Am't.   30½   11.74 31   31½   12.19 31½   12.32 32½   12.51 33½   12.30 32½   12.51 33½   12.90 33½   12.90 33½   13.28 33½   13.28 33½   13.47 35½   13.67 36½   14.05 37½   14.05 37½   14.05 37½   15.01 40½   15.59 41½   15.98 41½   15.98 41½   15.98 41½   15.98 41½   15.98 41½   16.36 42½   16.75 43½   16.75 44½   17.93 45½   17.32 45½   17.32 45½   17.32 46½   17.90 47½   18.29 48   18.48	Hrs.   Am't.   23.29   61   23.49   23.48   62   23.68   62   24.45   63.   24.45   64.   24.64   64.   24.64   64.   25.02   65.   25.02   66.   25.79   67.   25.79   68.   26.56   26.56   27.14   27.33   27.53   27.53   27.53   27.53   27.53   27.53   27.53   27.53   27.53   28.40   74.   28.68   75.   29.45   77.   29.26   676.   29.45   77.   29.84   78.   30.03	H1., Am't,
16½ 6.27 46½ 17.07 17 6.46 47 17.86 17½ 6.65 47½ 18.05	76½ 20.07 77 29.26 77½ 29.45 78 29.04 78½ 29.83 70 30.02 79½ 30.21 80 30.40	106½ 40.47 107 40.66 107½ 40.85	16½ 6.3 17 6.5 17 6.7 18 6.9 18½ 7.1 18½ 7.1 19½ 7.5 20 7.7 20½ 7.8 21½ 8.2 22½ 8.4 22½ 8.6 23↓ 9.2 24½ 9.4 25∫ 9.8	5   46½   17.90   47½   18.09   47½   18.29   3.48   18.48   48½   18.67   49   18.86   19.25   50½   19.44   51   19.63   51½   19.63   51½   19.63   51½   20.60   53½   20.60   54   54   20.70   55½   21.17   55½   21.17   55½   21.56	76½ 29.45 77 29.64 77½ 29.84	1061 41.00 107 41.19 1071 41.30
27 10.26 57 21.06 27 10.45 57 21.85 28 10.64 58 22.04 28 10.83 58 22.04 28 10.83 59 22.42 29 11.02 50 22.42 30 11.40 60 22.80	87 33.06 871 33.25 88 33.44 881 33.03 89 33.82 891 34.01	117 44.46 1172 44.65 118 44.84 1182 45.03 110 45.22 1104 45.41 120 45.60	27 10.3 27½ 10.5 28 10.7 28½ 10.9 29 11.1 29½ 11.3	9 57 21.94 9 57 22.14 8 58 22.33 7 58 22.52 6 50 22.71 6 59 22.91	87 33.49 878 33.09 86 33.88 882 34.07 89 34.26 892 34.46	117 45.04 1172 45.24 118 45 43 1182 1 62 119 4 81 1195 10 01 120 14'.20

				Ci	ANAD	IAN	CONT	RACT	OR'S	S HAN	D-BC	OOK				89
HO R.		AT	39 C	ENT	S PE	RH	OUR.		ı	AT 8	39½	CENT	rs P	ER H	OUR	
· Hi Am't,	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hs.	Am t.	Hrs.	Am't.	Hrs.	Am't.		TOTAL SECTION AND ADDRESS.
901 34.84			303	11.89	60 l	23.59	901	35.29			301	12.05	601	23.90	004	35.75
91 35.03			1 0 .1	12.09	61	23.79	91	35.49			31	12.24	61	24.09	91	35.73
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	2	0.78		12.28		23.98 24.18	915	35.68			6. 6.	12.44		24.29		36.14
92 35.61	21,	0.97		12.67		24.10	92	35.88 36.07	2 21/3	0.79	32	12.64	62	24.49	111	36.34
93, 35.80	3	1.17		12.87		24.57		36.27	3	1.18	33	13.03	63	24.88		36,54 36.73
932 36.00	32	1.36	335			24.76	931/2	36.46	35	1.38		13.23		25.08		36.93
94 36.19 94 36.38	$\frac{4}{4\frac{1}{2}}$	1.56	342	13.26	64	24.96 25.15		36.66	4	1.58	34,	13.43	64	25.28		37.13
95   36.57	5	1.95		13.65		25.35		36.85 37.05	4½ 5	1.73	012	13.63	65	25.48 25.67		37-33
952 36.77	55	2.14	35 1			25.54		37.24	51/2	2.17	35 <sub>1</sub>	14.02		25.87		37·52 37·72
96 36.96	65	2.34		4.04	66	25.74	96	37.44	6	2.37		14.22	66	26.07	96	37.92
$96\frac{1}{2}   37.15$ 97   37.34	7	2.53		4.43		25.93 26.13	965	37.63	$6\frac{1}{2}$	2.57	0 2	14.42		26.27	1	38.12
972 37-54	75	2.92		4.62		26.32		37.83 38.02	7 7 1 7 2	2.76		14.61 14.81		26.46 26.66		38,31 38,51
98 37.73	8	3.12	38 I	4.82	68	26,52	98	38.22	8	3.16	38	15.01		26.86	9/2	38.71
982 37.92	87	3.31		5.01		25.71		38.41	81	3.36	38½	15.21	. ~!	27.06	981	38.91
99 38.11	9.1	3.5t 3.70	39 1	5.21		26.91	99 1	38.61 38.80	9.5	3.55		15.40		27.25		39.10
100 38.50	10	3.90		5.60		27.30		30,00	10	3·75 3·95		15.60 15.80		27.45 27.05		39, 30 39, 50
1002 38.69	TO	4.09	401 1			27.49		39. 19	100	4.15		16.00		27.85		39.70
101 38.88	11	4.29		5 99		27.69		39.39	11	4.34		16.19		28.04	101	39.99
101 <sup>1</sup> / <sub>2</sub> 39.08 102 39.27	12	4.48	415 I 42 I	6.38		27.88 28.08	101		115	4.54		16.39		28,24	1011.	
1022 30.46	123	4.87	425 I			38.27	102	39.78 30.07	12	4.74		16.59   16.79		28, 4.4 28,6.4	102 1	10.29
103 39.05	13	5.07		6.77	73 2	28.47		10.17	13	5.13		16.98		28.83	and I	10,68
1032 39.85	132	5.26	100	6,96	732		1032		132	5.33	$43\frac{1}{2}$	17.18	$73\frac{1}{2}$	29.03	1033.	
104 40.01	14	5.46		7.16   7.35	1 1 1	28.86	10.1	10.50	14	5.53	44	17.38		29.23		80.1
105 40.42	15	5.85		7.55		29.25		10.95	143	5.73 5.92		17.58		20.62	10.12	
1052 40.02	152	6,04	455 1	7.74		29.44	1051		T 5 1	6.12		17.97		29.82	1053	
106 40.81	16.1	0.24		7.94		29.64		11.34	16	6.32	45	18.17	76	30.02		1.87
1062 41.00	17	6.03	465 I	8.33		29.83	1067	- 3	165	6.52		18.37		30.22	10614	
1075 41.30	173	0.82		8.52		30.22	107	11.73	175	6.91		18.56   18.76	77	30.41 30.61	107 4	12.26 12.16
108 41.58	15,	7.02	48 1	8.72		30.12		12, 12	18	7.11		18.90		30.81		2.66
1081 41.77	Inj.	7.21		8,91	783		1083		187	7.31	483	19.10		31.01	1081	2.86
109 41.90	1 1	7.41	100 1	9.11		18.08	100 -	12,51	10	7 50		19.35		31.20		13.05
110 42.35	201	7 80		9,50		31.20		12,90	192	7.70	die	19.55		31.60	100 4	13.25 13.45
1102 42,54	2	7.99	0	2.60		31.39	Hola.		201	8.10	67	19.95		31.80	1103 4	
111 42.73	21	3 38	47 4	9,89		31,59		13.29	21	8.2)		20, 14	81	32.99		13.84
11112 42,03 112 (43,12	22	8.58		0.08		31.98	1112	. 0	215	8.60		20.34	81	32,19	11124	
1122 43.31	22	8 77	6.5	0.47	. 1 1	32.17	1123	1.7	22!	8.30		20. 54		32.39 32.59	112.	14. 24 14. 44
113, 43.50	23	8.07	53, 2		83	32.37		1.07	23	9.08	53	20.93	83	32.78		4.63
1131 43.70	235	9.30	5352			3.1.50	1135		235	). 28	534	21.13			1135	
114 43,80 1141 44,08	2.1.	9.55	51 2			32.76		11.46	2.1	9.48	5-1	21.33		00	114	
115 44 27	25	0.75	55. 2		85 1	32.95 33.15	115	14.65	25	0.87	545	21.53	85		1145 4	
115441.47	255	9.54	552,2	1.64		33-34	1153			10.07		21.92	853	33.77	1151	
116 44.66	20	10.14	56 2	1.8.1	86	33.54	116	15.24	20	10.27	50	22,12	86	33-97	116 4	15,82
1162 44-85 117 45.04	27	10,53	57 2			33.73	116}.			10.47		22,32		34.17		
1175 45.24	273	10.72	571 2			33.93	117	15.82		10.65	57	22.51		31.35		
118 45 43	2.	10.62	58, 2	2.02	88	14.32	118			11.06		22.01		34.76		
1182   62	2/1	11.11	582 2		881 -	34 51	1181.	0,21	285	11.26	584	23.11	885	34.96	1187	6 8I
119 4 81	201	11.31	59 2 592 2	3.01		34.71		10,41		11.45	59	23.30		35.15	119 4	
120 10.20	10	11 70	60 2	3.40				16,60 16,80		11.65	592	23.70		35·35 35·55	120 4	
			arriance is solvening. The party	S. Same	- 9 - 13	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	, = 5	100	50	-110311		2:/01	90	ווכהינט		KIEL

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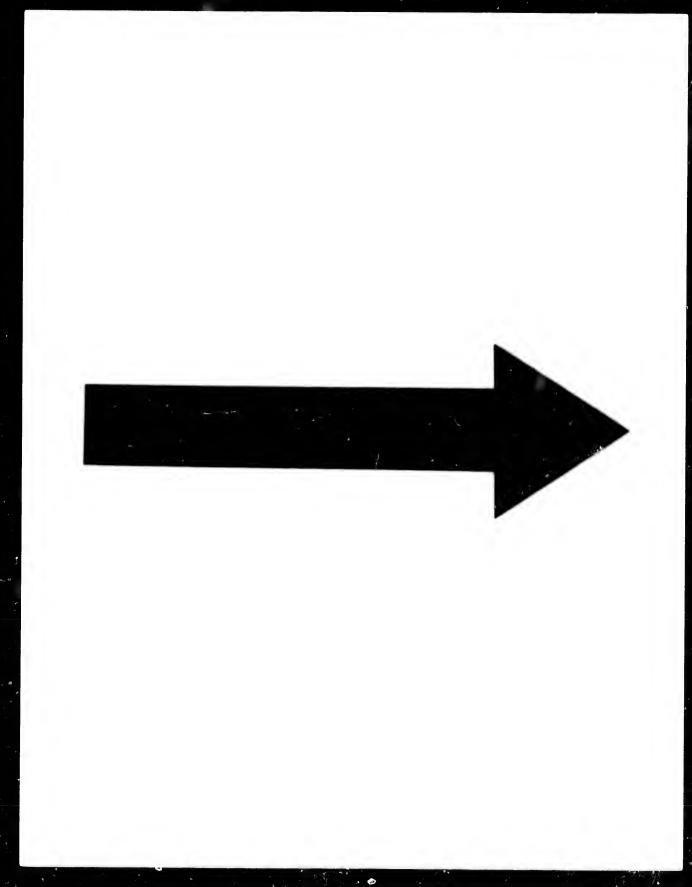
29<sup>2</sup> 29<sup>3</sup> 

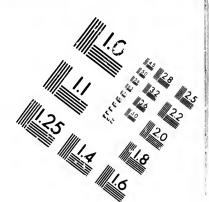
	AT	40 CENT	S PER HO	OUR.	1	AT 4	40½ CE	NTS PER H	OUR.
Hrs.	Am't.	Hrs. Am't.	Hrs.  Am't.	Hrs. Am't	. Hrs.	Am't.	Hrs. Am	t. Hrs. Am't.	Hrs. Am't.
-		301/2 12.20	601 24.20	902 36.20			30112.	601 24.50	901 36.65
		31 12.40	61 24.40	91 36.40			31 12.	5 61 24.70	91  36.85
		31 12.60	$61\frac{1}{2}$ 24.60	91 36.60		. 0.	31 12.7		912 37.06
2	0.80	32 12.80	62 24.80	92 36.80		1.01	32 12.9 32 13.1	6 62 25.11	92 37.26
25	1.00	321 13.00	621 25.00	92 37.20	$\frac{2\frac{1}{2}}{3}$	1.01	33 13.3		92½ 37.46 93 37.66
$\frac{3}{3\frac{1}{2}}$	1.40	33 13.20	631 25.40	93 37.40	$\frac{3}{3\frac{1}{2}}$	1.42	33 13.5		93 37.66 932 37.87
4	1.00	34 [13.60]	64 25.60	94 37.60	4	1.62	34 13.7		94 38.07
$4\frac{1}{2}$	1.80	341 13.80	642 25.80	948 37.80	41	1.82	342 13.9	7   $64\frac{1}{2}$ 26.12	942 38.27
5	2.00	35 14.00	65 26.00	95 38.00	5,	2.02	35, 14.1		95, 38.47
$\frac{51}{6}$	2.20	351 14.20	65½ 26, 20 66 26, 40	95\\\28.20	$\frac{52}{6}$	2.23	35 14.3	$65\frac{1}{2}$ 26.53	951 38.68
63	2.40	36 14.40 36½ 14.60	66 26.40	95 38.40 96½ 38.60		2.43	36 14.5 36½ 14.7		96 38.88 96½ 39.08
	2.80	37 14.80	67 26.80	97 38.80	7	2.83	37 14.9		902 39.00
$\frac{7}{7\frac{1}{2}}$	3.00	371 15.00	671 27.00	973 39.00	75	3.04	372 15.1		97 39 20
8	3.20	38 15.20	68 27.20	98 39.20	8	3.24	38 15.3	9 68 27.54	98   39.69
81	3.40	381 15.40	681 27.40	981 39.40	81	3.44	$38\frac{1}{2}$ 15.5		98計39.89
9	3.60	39 15.60	69 27.60	99 89.60		3.64	39 15.7		99 40.09
95	3.80	392 15.80	69 27.80 70 23.60	991 39.80	92	3.85 4.05	39½ 16.0 40 16.2	11 741 0	995 40.30
10	4.20	40 16.00	70 23.60	100 40.00	103	4.25	40 16.2	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	100 40.50 100 40.70
11	4.40	41 16.40	71 28.40	101 40.40	II	4.45	41 16.6	71 28.75	101 40.90
113	1.60	413 16.00	715 23.60	1013 40.60	111	4.66	411 16.8	71 28.96	1012 41.11
12	4.80	42 16.80	72 28.80	102 40.80	12	4.86	42 17.0	72 29.16	102 41.31
120	5.00	$42\frac{1}{2}$ 17.00	$72\frac{1}{2}$ 29.00	1022 41.00	121	5.06	$42\frac{1}{2}$ 17.2	_ 1) ' -   ' /	1022 41.51
13	5,20	43 17.20	73 29.20	103 41.20	13	5.26	43 17.4	11 10 1 20	103 41.71
132	5.60	43½ 17.40 44 17.60	732 29.40 7.1 29.60	103 11.40	0.2	5.47 5.67	43 <sup>1</sup> / <sub>2</sub> 17.6	132 - 11	1032 41.92
14	5.80	44 17.80 441 17.80	74 29.00 742 2 80	104 41.80	14	5.87	44 17.8 44 <sup>1</sup> / <sub>2</sub> 18.0		104 42.12 104 42.32
15	6.00	4s [18,00]	75 30.00	105 12.00	15	6.07	45 18.2	75 30.37	105 42.52
155	6.20	454 18,20	754 30.20	1052 42.20	151	6,28	453 18.4	75 30.58	1053 42.73
16	6,40	46 18.40	76 30.40	100 12.40	16	6.48	46 18.5	76 30.78	106 42.93
161	0.60	161 18.60	769 30.00	100 42.60	161	6.68	461 18.8		1062 43.13
17	6,80	47 18.80	77 30.80	107 42.80	17	6,88	47 19.0		107 43.33
175	7.00	472 19.00 18 19.20	772 31.00	1071 43.00	173	7.09	47½ 19.2 48 19.4	1 / 2 10 00	107: 43.54 108 43.74
185	7.40	48 19.20 48 19.40	78 31.20	1081 43.40	185	7.49	485 19.6	.    /  0 0 /	108 43.74 108 <u>1</u> 43.04
10	7.00	49 19.00	79 31.60	109 43.60	10	7.69	49 19.8		100 11.14
101	7.80	493 19.80	79 31.80	1002 43 80	195	7.90	491 20.0	79! 32.20	109/41/5
20	8.00	50 20,00	80 32.00	110 14.00	20	8.10	50 20.2	80 32.40	110 11 11
201	8 20	50 20.20	801 32.20	110 44.20	~	8.30	50 20.4		1105 1
21	8.40	51 20,40	81 32.40	111 41.40	21	8.50	51 20.0		111 (10
21 h	8.80		81 ½ 32.60 82 32.80	111 44.00	212	8.91	51 20.50		111 \$ 15.36
22.	9.00	52 20.80 52 21.00	825 33.0C	112 45.00	221	9.11	52 21.2		112   5.30
23	9.20	53 21,20	83 33.20	113 45.20	23	9.31	53 21.4	83 133.61	113 45.70
231	9.40	532 21.40	832 33.40	1132 45.40	232	9.52	53 21.6	831 33.82	1134 15.97
2.1	9.60	51 21.00	84   33.60	114 45.60	24	9.72	54 21.8		114 40-17
9.11	19.80	545 21.80	841 33.80	1142 45.80		9,92	541 22.0		1142 46.37
25	0.20	55 22.00	85 34.20 85 34.20	115 40.00		10.12	55 22.4		115 40.57
	10, 10	55 22.40	86 34.40	116 46.40	25½ 26	10.33	56 22.6		116 45 98
- 1	0.60	561 22.00	863 34.60	1163 46.60	261	10.73	563 22.8		1165 17.18
	0.80	57 22.80	87 34.80	117 46.80		10.93	5 23.0	87 35.23	117 47.38
273	1.00	571 23.00	871 35.00	1172 47.00		11.14	572 23.2		1175 47-59
	1.20	58 23.20	88 35.20	118 47.20		11.34	58 23.4	88 35.64	118 47.79
281		581 23.40	881 35.40	1182 47.40		11.54	58 23.6	1 - 2 00	1185 47.99
20 1	1.60	59 23.00	89 35.60	119 47.60	29	11.74	59 23.8		119 48-19
	2.00	592 23.80	90 36.20	110 47.80 120 48.00	30	11.05	59 24.10	11 010	119 48.40
20 11	2.0011	00 124.0011	20,130,501	1 40,00	30	1213	00 124.3	0   90   36.45	1120 10100

HOUR.	AT	41 CENT	S PER HO	UR.	AT	41½ CEN7	S PER H	OUR.
t. Hrs. Am't,	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	His. Am t.	Hrs. Am't.	Hrs. Am t.	Hrs. Am't.
0 901 36.65		303 12.50	601 24.80					
0 91 36.85		31 12.71	61 25.01	90½ 37.10 91 37.31		30 1 12.66	60 25.11	902 37.56
912 37.06		31 12.91	61 2 25.21	91 37.51		31 12.86	611 25.52	91 37.70
I 92 37.26	2 0.82	32 13.12	62 25.42	92 37.72	2 0.83	32 13.28	62 25.73	92   38, 18
1 92 37.46	2½ I.O2	$32\frac{1}{2}$ 13.32	62 25.62	923 37.92	21 1.04	32 13.49	621 25.94	921 38.39
93 37.66 2 932 37.87	3 1.23	33 13.53	63 25.83	93, 38.13	3, 1.24	33 13.69	63 26.14	93   38.59
2 94 38.07	3½ I.43 4 I.64	335 13.73	63 26.03	932 38.33	32 1.45	335 13.90	63½ 26.35	93 38.80
2 942 38.27	4 I.64 41 I.84	34 13.94 34½ 14.14	64 26.24	94 38.54 94½ 38.74	4 I.66 4½ I.87	34 14.11	64 26.50	04 30,01
2 95 38.47	5 2.05	35 14.35	65 25.65	95 38.95	5 3.07	34½ 14.32 35 14.52	65 26.97	942 39.22
$3   95\frac{1}{2} 38.68$	52 2.25	352 14.55	651 26.85	952 39.15	52 2.28	35 14.52 35 14.73	65 27.18	95 39.63
3   96   38.88	6 2.46	36 [14.76]	66 27.06	96   39.36	6 2.49	36 14.94	66 27.39	96 39.84
3   96½ 39.08 3   97  39 28	$6\frac{1}{2}$ 2.66	36 1 14.96	662 27.26	961 39.56	$6\frac{1}{2}$ 2.70	361 15.15	661 27.60	961 40.05
1 97 39 20	7 2.87 71 3.07	37 15.17 37 15.37	67 27.47 67 27.67	97 39.77	7 2.90	37 15.35	67 27.80	97 40.25
98 39.69	8 3.28	38 15.58	68 27.88	97 <sup>1</sup> / <sub>2</sub> 39.97 98 40.18	72 3.11	372 15.56	67½ 28.01 68 28.22	075 40.40
1 981 39.89	8 3.48	38 2 15.78	681 28.08	983 40.38	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	38 15.77 381 15.98	681 28.43	985 40.88
1 99 40.09	9 3.69	39 [15.99]	69 28.29	99 40.59	9. 3.73	39 16.18	69 28.63	99 41.08
5 99½ 40.30	92 3.89	392 16.19	692 28.49	991 40.79	92 3.94	$39^{\frac{1}{2}}$ 16.39	691 28.84	992 41,29
5 100 40.50 5 100 40.70	10 + 10	40 16.40	70 28.70	100 41,00	10 4.15	40 16.60	70 29.05	100 41.50
5 101 40.90	10 4.30	40 16.60	702 28.90	1003 41.20	101 4.36	402 16.81	702 29.26	1002 41.71
5 101 41.11	$11 \ 4.51$ $11\frac{1}{2} \ 4.71$	41 16.81 412 17.01	71 29,11 712 29.31	101 41.41 101 41.61	11 4.56 11 4.77	41 17.01	71 29.46 71 29.67	101 41.91 101 42.12
5 102 41.31	12 4.92	42 17.22	72 29.52	102 41.82	115 4.77	42 17.43	72 29.88	102 42.33
0 1022 41.51	122 5.12	422 17.42	72 29.72	102 42.02	125 5.19	42 17.64	721 30.09	102 3 42.54
0 103 41.71	13, 5.33	43 17.63	73 29.93	103 42.23	13 5.39	43 17.84	73 30.29	103 42.74
7 1032 41.92	[32] 5.53	432 17.83	731 30.13	1032 42.43	13 5.60	432 18.05	732 30.50	1031 42.95
7 104 42.12 7 104 42.32	14 5.74	44 18.04	74 30.34	104 42.64	14   5.81	44 18.20	74 30.71	104 43.16
7 105 42.52	15 6.15	442 18.24	742 30-54	104 <sup>1</sup> / <sub>2</sub> 42.84 105 43.05	145 6.02	44 18.47	745 30.92	104 43.37
8 1053 42.73	152 6.35	45 18.65	75 30.75	105 43.05	15 6.22	45 18.88	75 31.33	1053 43.78
3 106 42,93	10 6.56	46 18.86	76 31.16	106 43.40	16 6.54	46 19.09	76 31.51	106 43.99
3 1002 43.13	104 6.76	465 19.06	761 31.36	1061 43.00	165 6.85	461 19.30	765 31.75	1062 44.20
3 107 43.33 3 107! 43.54	17 6.97	47 19.27	77 31.57	107 43.87	17 7.05	47 19.50	77 31.95	107 44.40
9 1071 43.54	17½ 7-17 18 7-38	48 19.47	775 31.77	107 44.07	172 7.26	472 19.71	771 32.16	1072 44.61
1085 43.44	181 7.58	48 19.68 485 19.88	78 31.98 78 32.18	108 11.48	18 7.47 18½ 7.68	48 19.92	78 32.37 783 32.58	108 44.82
0 109 11.14	10 7.79	49 20,00	70 32.39	1081 44.48	10 7.88	40, 20, 33	70 32.78	100 45.23
5 109 4105	1 3 7.00	100 20. 20	795 32 59	1003 14.80	193 8.00	493 20, 54	795 32.99	1095 45.44
0 110	1 8 30	50 20.50	80 32.80	110 45.10	20 8.30	50 20.75	80 33.20	110 45.65
D 111 11-00 \	21 8 61	502 20.70	805 33 00	110 45.30	201 8.51	50 20.00	802 33 41	110. 45.80
1 1114 15.10	21, 8.6r 2 8.8r	51 20,91	81 33.21	111 45.51	21 8.71	51 21.16	81 33.01	111 10.00
112 15.36	9 02	52 21.32	81 33.41 82 33.62	1115 15.71	21 8.92	51 21.37	82 34.03	112 40, 48
1122 15.50	21, 9.22	52 21.52	822 11.83	112 15.92	22 9.13	524 21.70	821, 34, 24	112 40.69
113 45.76	23, 0.43	53 21.73	83 34.03	113 46.33	23 9.54	53 21.99	83 34.44	113 46.89
2 1135 45-97	= 13, 0.03	53 21.93	832 34.23	1131 46.53	232 9.75	532 22,20	835 34.65	113, 47.10
2   114   40.17 2   114 <sup>1</sup> / <sub>2</sub>   40.37	213 10.01	54, 22.14	84, 34-44	114, 46,74	24 9.46	5.1 22.41	81 34.86	114 47.31
2 1145 40.37	2,10.01	542 22 31	8.12 34.61	1145 46.94	242 10.17	545 22.62	43	114 47.52
3 1155 10.78	254 10.45	55 22.55	85 34 85 854 35.05	115 47-15	25 10.37	55 22.82	85 35 48	115 47.72
	70 10.66	56 22.96	86 35.26	115½ 47.35 116 47.56		56 23,24	80 35.00	116 48.14
3   116   47, 48 3   116   47, 18 3   117   47, 38	203 10.86	563 23.16	862 35.40	1165 47.76	265 11.00	564 23.45	861 35.90	1161 48.35
3 117 47.38	27 11.07	57, 23-37	87 35.67	117 47.97	27 11.20	57 23.65	87 30,10	117 48.55
4 1175 47.59	75 11.27 11.48	572 23.57	872 35.87	$117\frac{1}{2}$ 48.17		571 23 86	871 36.31	1175 48,76
1 118 47.79 1 118 47.99	287 11.48		88 36.08	118 48,38	28 11.62	58 24.07	88 36.52	118 48.97
4 119 48.19	29 11.89	582 23.98 59, 24.19	88½ 36.28 89 36.49	118 48.58		58 24. 28	88½ 36.73 89 36.93	119 .19.38
1 0	202 12.00	592 24.39	801 36.60	119 48.79 119 48.99	29 12.03	59 24.69	895 37.14	1195 49.59
5 1194 18.40	30 12,30	60 24.60	90 36.90	120 49.20			90 37.35	120 49.80
		All the state of t		17:	13		The second secon	and the second second

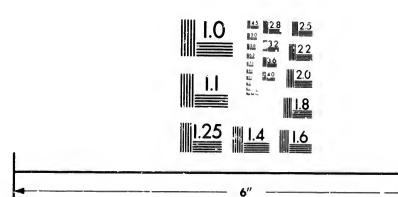
	AT	42 CENT	S PER HO	OUR.	1	AT 4	42½ CEN'	TS PER H	OUR.
Hrs.	Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs.	Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.
		301 12.81	601 25.41	902 38.01			302 12.96	601 25.71	901 38.46
		31 13.02	61 25.62	91 38.22			31 13.17	61 25.92	91 38.67
	- 0.	311 13.23	$61\frac{1}{2}$ 25.83	91 2 38.43		0.85	31 1 1 3 3 9	$61\frac{1}{2}$ 26.14	912 38.89
2 2 }	0.84	32 13.44 32 <sup>1</sup> 13.65	62 26.04 621 26.25	92 38.64 921 38.85	$\frac{2}{2\frac{1}{2}}$	1.06	32 13.60	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	92 39.10 92 39.31
3.	1.26	33 13.86	63 26.46	93   39.06	3	1.27	33 14.02	63 26.77	93 39.52
35	1.47	335 14.07	$63\frac{1}{2}$ 26.67	932 39.27		1.49	335 14.24	$63\frac{1}{2}$ 26.99	932 39.74
$\frac{4}{4\frac{1}{2}}$	1.68 1.89	34 14.28 34 <sup>1</sup> 14.49	64 26.88	94 39.48 94½ 39.69		1.70	34 14.45 34½ 14.66	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	94 39.95
5	2,10	35 14.70	65 27.30	95 39.90	5	2.12	35 14.87	65 27.62	95 40.37
51/2	2.31	352 14.91	$65\frac{1}{2}$ 27.51	952 40,11	$5\frac{1}{2}$	2.34	355 15.09	652 27.84	952 40, 50
$\frac{6}{6!}$	2.52	36 15.12 36½ 15.33	66 27.72	$96   40.3^2$ $96\frac{1}{2}   40.53$	61	2.55	$36   15.30   36\frac{1}{2}   15.51  $	66 28.05	96 40.80 96 41.01
7	2.94	37 15.54	67 28.14	97 40.74	7	2.97	37 15.72	67 28.47	90 41.22
7½ 8	3.15	375 15.75	672 28.35	975 40.95	7 <sup>1</sup> / <sub>2</sub> 8	3.19	372 15.94	671 28.69	972 41.44
	3.36	38 15.96 38½ 16.17	68 28.56 68 28.77	98 41.16 98 41.37	8 8 <u>1</u>	3.40	38 16.15 381 16.36	68 28.90	98 41.65
8½ 9	3·57 3·7 <sup>8</sup>	30 16.38	69 28.98	99 41.58	9	3.61 3.82	39 16.57	682 29.11	98½ 41.86 99 42.07
$9\frac{1}{2}$	3.99	392 16.59	692 29.19	995 41.79	91	4.04	392 16.79	692 29.54	99 42.29
10	4.20	40 16.80	70 29.40	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		4.25	40 17.00	70 29.75	100 42.50
102	4.4I 4.62	40½ 17.01 41 17.22	70½ 29.61 71 29.82	101 42.42	$10\frac{1}{2}$	4.46	40½ 17.21 41 17.42	70½ 29.96 71 30.17	1002 42.71
$11\frac{1}{2}$	4.83	411 17.43	711 30.03	1013 42.63	$11\frac{1}{2}$	4.89	412 17.64	71 30.39	101 43.14
12	5.04	42 17.64	72 30.24	102 42.84	12	5.10	42 17.85	72 30.60	102 43.35
121	5.25 5.46	42½ 17.85 43 18.06	72½ 30.45 73 30.66	$102\frac{1}{2}$ 43.05 103 43.26	122	5.31	42½ 18.06 43 18.27	$72\frac{1}{2}$ 30.81 $73$ 31.02	102 43.56
135	5.67	431 18.27	732 30.87	1035 43.47	131	5.74	431 18.49	$73\frac{1}{2}31.24$	103 43.09
14	5.88	44 [18.48]	74 3r.08	104 43.68	14	5.95	44 18.70	74 31.45	104 44,20
145	6.30	44½ 18.69 45 18.90	$74\frac{1}{2}$ 31.29 75 31.50	104 <sup>1</sup> / <sub>2</sub> 43.89 105 44.10	143	6.16	44 <sup>1</sup> / <sub>2</sub> 18.91 45 19.12	745 31.66 75 31.87	104 44.41
153	6.51	45 19.11	$75\frac{1}{2} 31.71 $	1052 44.31	151	6.59	451 19.34	75 32.09	1053 44.84
16	6.72	46 19.32	76 31.92	106 44.52	16	6.80	46 19.55	76   32, 30	TO6 45.05
16 <u>1</u>	6.93 7.14	46½ 19.53 47 19.74	$76\frac{1}{2}$ 32.13 77 32.34	106½ 44.73 107 44.94	165	7.01	46½ 19.76   47   19.97	$76\frac{1}{2}$ 32.51 77 32.72	106½ 45.26 107 45.47
173	7.35	475 19.95	77 32 32 55	1073 45.15	175	7.44	$47\frac{1}{2}$ 20.19	77 32.54	1072 45.69
18	7.56	48 20.10	78 32.70	108 45.36	18	7.65	48 20.40	78   33.15	108 45.90
185	7.77 7.98	$48\frac{1}{2}$ 20.37 $49$ 20.58	78½ 32.97 79 33.18	108 45.57	182	7.86 8.07	48½ 20 01 49 20.82	78½ 33.36 79 33.57	108 <u>1</u> 46.11
193	8.19	492 20.79	$79\frac{1}{2}33.39$	1092 45.99		8.29	492 21.04	79 33.79	109 46.54
20	8.40	50 21.00	80 33.60	110 46.20	20	8.50	50 21.25	80 34.00	110 46.75
20½	8.61	50 <sup>1</sup> 21.21 51 21.42	80½ 33.81 81 34.02	110 46.41	20½ 2I	8.71	50 21.46 51 21.67	80½ 34.21 81 34.42	1 102 46.96 1 1 1 47.17
$21\frac{1}{2}$	9.03	51 21.63	81 34.23	1111 46.83	212	9.14	512 21.80	81 34.64	1111 47.39
22	9.24	52 21.84	82 34.44	112 47.04	22	9.35	52 22.10	82 34.85	112 47.60
$22\frac{1}{2}$	9.45 9.66	52½ 22.05 53 22.26	$\begin{array}{c c} 82\frac{1}{2} & 34.65 \\ 83 & 34.86 \end{array}$	$112\frac{1}{2}$ 47.25 113 47.46	225	9.50	$52\frac{1}{2}$ 22.31 $53$ 22.52	82 <sup>1</sup> / <sub>2</sub> 35.06 83 35.27	112½ 47.81 113 48.00
231	9.87	531 22.47	831 35.07	1132 47.67	231	9.99	$53\frac{1}{2}$ 22.74	835 35.49	1135 48.24
	10.08	54 22.68	84 35.28	114 47.88	24	10.20	54, 22.95	84 35.70	114 48,45
	10.29	54½ 22.89 55 23.10	84 <sup>1</sup> / <sub>2</sub> 35.49 85 35.70	114 <sup>1</sup> / <sub>2</sub> 48.09 115 48.30		10.41	54 <sup>1</sup> <sub>2</sub> 23.16 55 23.37	841 35.91	1145 48.00
	10.71	55 <sup>1</sup> / <sub>2</sub> 3,31	85½ 35.91	1152 48.51		10.84	55 23·37 552 23·59	85   36, 12   85½   36, 34	115 48.87 1152 49.09
	10.92	56 23.52	86 36.12	116 48.72	26	11.03	56 23.80	86   36 55	116 49.30
	11.13	56½ 23.73	86½ 36.33 87 36.54	116 48.93		11.47	565 24.01	86½ 36.76 87 36.97	1162 49.51
272	11.55	572 24.15	872 36.75	1172 49.35	275	11,69	57 24. 14	871 37.19	117 40.72 1171 49.94
28	11.76	58 24.36	88   36.96	118 49.56		11.90	58 24.65	88 37.40	118 50.15
202	11.97	58½ 24.57 59 24.78	88½ 37.17 89 37.38	118½ 49.77 119 49.98		12.11	58½ 24.86 59 25.07	88½ 37.61 89 37.82	118½ 50.36
293	12.39	593 24.99	893 37.59	119 50.19	291	12.54	591 25.29	892 38.04	1192 50.79
	12,60	60 25.20	90   37.80	120 50.40	30	12.75	60 25 50	90   38.25	120 51.00

HOUR.	AT	43 CENT	C DED U	)IID	- A/T	40 . CEN	TC DED U	IOUD
t. Hrs. An't	Hrs !\m't	Hrs. Am't.	S FER II	JUK.			TS PER H	
		1113. 11111.	TIIS. AIII L.	rirs. Am t.	Hi Am t.	Hrs. Am t.	Hrs. Am't.	Firs. Am't.
90 2 38.45		3013.11	603 26,01	901 38.91		301 13.27	601 26.32	901 39.37
91 38.67 91 38.89		31 13.33	61 26.23	91 39.13		31 13.48	61 26.53	91 39.58
92 39.10	- 0.06	312 13.54	61 2 26.44	915 39.34		31 13.70	61 26.75	911 39.80
921 39.31	2 0.86	32 13.76	62 25.66	92 39.56		2 13.92	62 26.97	92 40.02
93 39.52	3 1.29	32½ 13.97 33 14.19	62 26.87	92 39.77	$2\frac{1}{2}$ 1.09	$32\frac{1}{2}$ I4. I4	62 27.19	925 40.24
932 39-74	31 1.50	332 14.40	63 27.30	93 39.99 935 40.20	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	33 14.35	63 27.40	93 40.45
94, 39.95	4 1.72	34 14.62	64 27.52	94 40.42	4 I.74	332 14.57	64 27.84	94 40.89
945 40.16	42 1.93	342 14.83	645 27.73	942 40.63	41 1.96	341 15.01	641 28.06	941 41.11
952 40, 50	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	35 15.05	65 27.95	95, 40.85	5, 2.17	35 15.22	65 28.27	95 41.32
96 40.80	6 2.58	35½ 15.26 36   15.48	65 28.16	955 41.06	$\frac{5^{1}_{2}}{6}$ 2.39	35 15.44	65 28.49	955 41.54
962 41.01	$6\frac{1}{2}$ 2.79	36½ 15.69	661 28.59	96 41.28 96½ 41.49	6 2.61	36 15.66 361 15.88	66 28.71	96 41.76
97 41.22	7 3.01	37 15.91	67 28.81	97 41.71	7 3.04	37 16.09	67 29.14	97 42.19
97½ 41.44 98 41.65	75 3.22	37 2 16.12	67 2 29.02	972 41.92	72 3.26	375 16.31	671 29.36	975 42.41
981 41.86	8 3.44 8 3.65	38 16.34	68 29.24	98 42.14	8 3.48	38 16.53	68 29.58	98 42.63
99 42.07	9 3.87	38½ 16.55 39 16.77	68 29.45	981 42.35	$8\frac{1}{2}$ 3.70	381 16.75	681 29.80	981 42.85
992 42.29	$9\frac{1}{2}$ 4.08	39 16.98	69 29.88	99 42.57	9 3.91 $9\frac{1}{2}$ 4.13	39 16.96 39½ 17.18	69 30.01 69 30.23	99 43.06 99 <sup>1</sup> 43.28
100 42.50	10 4.30	40 17.20	70 30.10	100 13.00	10 4.35	40 17.40	70 30.45	100 43.50
1002 42.71	10 4.51	$40\frac{1}{2}$ 17.41	701 30.31	100 1 43.21	102 4.57	405 17.62	703 30.67	$100\frac{1}{2}$ 43.72
1012 43.14	11 4.73 111 4.94	41 <sub>1</sub> 17.63	71 30.53	101 43.43	11 4.78	41 17.83	71 30.88	101 43.93
102 43.35	12 5.16	41 17.84 42 18.06	71½ 30.74 72 30.96	101 <sup>1</sup> / <sub>2</sub> 43.64 102 43.86	$11\frac{1}{2}$ 5.00 12 5.22	411/2 18.05	71 2 31.10	101 2 44.15
1022 43.56	121 5.37	421 18.27	72 31.17	102 44.07	$12   5.22  $ $12\frac{1}{2}   5.44  $	42 18.27 42 18.49	72 31.32 72 31.54	102   44.37 102   44.59
103 43.77	13 5.59	43 18.49	73 31.39	103 44.29	13 5.65	43 18.70	73 31.75	103 44.80
104 44.20	13½ 5.80 14 6.02	432 18.70	732 31,60	1031/2 44.50	135 5.87	432 18.92	732 31.97	1032 45.02
1042 44.41	143 6.23	44 18.92 442 19.13	74 31.82 74½ 32.03	104   44.72   104   144.93	14 6.09	44 19.14	$74   32.19   74\frac{1}{2}   32.41  $	104 45.24
105, 44.02	15 6.45	45 19.35	75 32.25	105 45.15	$14\frac{1}{2}$ 6.31 $6.52$	44½ 19.36 45 19.57	75 32.62	104 45.46
105½ 44.84 106 45.05	15½ 6.66	452 19.56	75 2 32.46	105 45.36	151 6.74	45 19.79	75 32.84	1053135.89
106 45.00	16 6.88	46 19.78	76 32.68	106 45.58	16 6.96	46 20.01	76   33.06	106 46.11
107 45.47	105 7.09 17 7.31	465 19.99	761 32.89	1062 45.79	161 7.18	46½ 20.23	761 33.28	106½ 46.33
1072 45.69	172 7.52	47 20.42	77 33.11 $77 33.32$	107 46.01 107 46.22	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	47 20.44 47 20.66	77   33·49   77½   33·71	107   46.54 107   46.76
108 45.90	18 7.74	48 20.64	78 33.54	108 46.44	18 7.83	48 20.88	78 33.93	108 46.98
100 46.32	18½ 7.95	48 20.85	78 2 33.75	$108^{1}_{2}$ 46.65	182 8.05	481 21.10		1082 47.20
1095 46.54	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	49 21.07 492 21.28	79   33.97	109 46.87	19 8.26	49 21.31	79 34.36	109 47.41
110 46.75	20 8.60	50 21.50	79½ 34.18 80 34.40	100 <sup>1</sup> 47.08	19½ 8,48   20   8,70	$49\frac{1}{2}$ 21.53 50 21.75	79½ 34.58 80 34.80	109½ 47.63 110 47.85
1102 46.96	202 8.81	502 21.71	801 34.61	1102 47.51	201 8.92	50 21.97		110 48.07
111 47.17 111 47.39	21 9.03	51 21.93	81  34 83	111 47.73	21 9.13	51 22.18	81 35.23	111 48.28
112 47.60	21 5 9.24	51 22.14 52 22.36	812 35.04	1111 +7.91	$21\frac{1}{2}$ 9.35	51 22.40		1111 48.50
1122 47.81	223 9.67	52 22.57	82 35.26 822 35.47	112 48.16 112 48.37	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	52 22.62 52 22.84	_ ,   00 _ ,   11	112 48.72 112 48.94
113 48.00	23 0.89	53 22.79	83 35.09	113 48.59	23 10.00	53 23.05		113 49.15
1135 48.24	235 10.10	532 23.00	832 35.90	1132 48.80	231 10.22	531 23.27		1131 49.37
1142 48,00	24 10.32 24 10.53	54 23.22	84 36,12	114 49.02	24 10.44	54, 23.49		114 49.59
115 48.87	25 10.75	542 23.43 55 23.65		1145 49.23	242 10.66	542 23.71		1142 49.81
1152 49.09	255 10.96	55 23.86	85 36.76	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	25 10.87 25½ 11.09	55 23 92 553 24.14	85   36.97   85½   37.19	115 50.02 115 50.24
116 49.30 116 <sup>1</sup> 49.51	26 11.18	56 24.08	86 36.98	115 49.88	26 11.31	56 24.36	86 [37.41]]	110 50.46
117 40.72	26½ 11.39 27 11.61	562 24.29.	$86\frac{1}{2}$ 37.19	1161 50.09	261 11.53	563 24.58	861 37.63	116½ 50.68
1172 49.94	275 11.82	57 24.51 572 24.72		117 50.31	27 11.74	57 24.79	87 37.84	
118 50.15	28 12.04	58 24.94		117½ 50.52 118 50.74	27½ 11.96 28 12.18	57½ 25.01 58 25.23	87½ 38.06 88 38.28	118 51.22
1182 50.36	282 12,25	582 25.15	881 38.05	1182 50.95	281 12.40	581 25.45	881 38.50	$118\frac{1}{2}$ 51.55
119 50.57 119 <sup>1</sup> 50.79	29 12.47 29 12.68	59, 25.37	89   38.27	119 51.17	29 12.61	59 25 66	89   38.71	119 51.76
120 51.00	30 12.90	592 25.58 60 25.80	89 38.48	1192 51.38	291 12.83	591 25.88	892 38.93	119 51.98
	2 190 1	125.00	90 138,701	120  51.60	30 [13.05]	60 26,10	90  39.15	120 152,20





## IMAGE EVALUATION TEST TARGET (MT-3)



Photographic Sciences Corporation

23 WEST MAIN STREET WEBSTER, N.Y. 14530 (716) 872-4503

SIM VIM EZZIMI



Hrs

AT	44 CENT	S PER HO	UR.	AT 44½ CENTS PER HOUR.					
Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am't.	Hrs. Am'	t. Hrs. Am't.	Hrs. Am't.	Hrs. Am't		
	30113.42	601 26.62	901 39.82		30113.57	601 26,92	903 40.27		
	31 13.64	61 26.84	91 40.04		31 13.79	61 27.14	91 40.49		
2 0.88	31½ 13.86 32 14.08	62 27.28	91 40.26	2 0.80	31 14.02	61 27.37	912 40.72		
25 L.10	32 14.30	622 27.50	922 40.70	$2\frac{1}{2}$ I.1	321 14.46	621 27. 1	921 41.16		
3 1.32	33 14.52	63 27.72	93 40.92	3 1.3		63 28.03	93 41.38		
3½ 1.54 4 1.76	33½ 14.74 34 14.96	64 28.16	94 41.36	3½ 1.50 4 1.78		63½ 28.26	93½ 41.61 94 41.83		
4 1.70 4½ 1.98	34 14.90 34½ 15.18	641 28.38	941 41.58	$4\frac{1}{2}$ 2.00	11 0 1 1 1	641 28.70	942 42.05		
5 2.20	35 15.40	65 28.60	95 41.80	5 2.22		65 28.92	95, 42.27		
51 2.42	35½ 15.62 36 15.84	65 29.04	95 42.02	5½ 2.43 6 2.67	1 333 6	65 29.15	952 42.50		
$6\frac{1}{3}$ 2.86	361 16.06	661 29.26	063 42.46	61 2.89	36 16.24	661 29.59	961 42.94		
7 3.08	37 16.28	67 29.48	97 42.68	7 3.11	37 16.46	67 29.81	97 43.16		
7½ 3.30 8 3.52	37½ 16.50 38 16.72	63 29.92	97½ 42.90 98 43.12	7½ 3.34 8 3.56	38 16.91	67 <sup>1</sup> / <sub>2</sub> 30.04 68 30.26	972 43.39 98 43.61		
81 3.74	381 16.94	681 30. 14	983 43.34	81 3.78	385 17.13	681 30.48	98 43.83		
9 3.96	30 77.10	69 30.36	99 43.56	9 4.00	1 1 0	69 30.70	99 44.05		
9½ 4.18 10 4.40	39½ 17.38 40 17.60	70 30.80	991 43.78	9½ 4.23 10 4.45	40 17.80	69½ 30.93 70 31.15	99½ 44.28 100 44.50		
10 4.62	401 17.82	701 31.02	1002 44.22	101 4.67	40 18.02	702 31.37	1005 44.72		
11 4.84	41 18.C4	71 31.24	101 44.44	11 4.80		71 31.59	101 44.94		
11½ 5.06 12 5.28	41 18.26	71 31.46	101 44.66	115 5.12	1 0 6 0	71½ 31.82 72 32.04	1015 45.17		
121 5.50	125 18.70	721 31.90	102 1 45. 10	121 5.56	421 18.91	$72\frac{1}{2} 32.26 $	1022 45.61		
13 5.72	43 18.92	73 32.12	103 45.32	13 5.78		73, 32.48	103 45.83		
14 5.94 6.16	43½ 19.14 44 19.36	73½ 32.34 74 32.56	103 45.76	13½ 6.01	44 19.58	73½ 32.71	103 <u>\$</u> 46.06		
141 6.38	441 19.58	742 32.78	1045 45.98	142 6.45	443 19.80	742 33.15	1045 46.50		
15 6,60	45 10.80	75  33.00	105 46.20	15 6.67		75 33.37	105 46.72		
15½ 6.82 16 7.04	451 20,02 46 20,24	75½ 33.22 76 33.44	1051 46.42	15½ 6.90 16 7.12	1 2 2	75½ 33.60 76 33.82	105 46.95 106 47.17		
165 7.26	461 20.46	761 33.66	1061 46.86	161 7.31	461 20.69	763 34.04	1061 47.39		
17 7.48	47 20.63	77 33.88	107 47.08	17 7.56		77 34.26	107 47.61		
17½ <b>7.</b> 70 18 7.92	.17½ <b>2</b> 3.90 .18 21.12	77 34.10	107 47.30	175 7.79 18 8.01	48 21.36	775 34.49 78 34.71	108 48.00		
185 8.14	481 21.34	785 34.54	1081 47.74	181 8.23	481 21.58	781 34.93	108 48.28		
19 8.36	49 21.56 49 21.78	79 34.76	109 47.96	19 8.45		79 35.15	109 48.50		
19\\ 8.58 20 8.30	50 22,00	80 35.20	110 48.40	19½ 8.68 20 8.90		793 35.38 80 35.60	110 48.95		
201 9.02	503 22,22	801 35.42	1101 48.62	201 9.12	501 22.47	801 35.82	1101 49.17		
21 9.24	51 22.44	81 35.64 813 35.86	111 48.84	21 9.34 21 9.57		81 36.04	111 49.39		
21 9.45	52 22.88	82 36.08	112 49.28	21½ 9.57 22 9.79	52 23.14	82 36.49	112 49.84		
221 9.90	525 23.10	821 36.30	1125 49.50	220 10.01	521 23.36	823 36.71	112 50.06		
23 10.12	53 23.32 53.54	83 36.52	113 49.72	23 10.23	1 0	83 36.93 83 <sup>1</sup> 37.16	113 50.23		
23 10.34	54 23.76	84 36.96	114 50.16	23 10.46	54 24.03	83 <sup>1</sup> / <sub>2</sub> 37.16 84 37.38	113 50.51		
241 10.78	543 23.98	843 37.18	1143 50.38	2.15 10.90	541 24-25	843 37.60	114 50.95		
25 11.00 251 11.22	55 24.20 55 24.42		115 50.60	25 11.12 25 11.35	1 004	85 37.82 85 38.05	115 51.17		
26 11.44	56 24.64	86 37.84	116 51.04	26 11.57		86 38.27	116 51.62		
26166	561 24.86	861 38.06	1162 51.26	26 11.79	563 25.14	861 38.49	1161 51.84		
27 11.88	57 25.08	87 38.28 87 38.50	117 51.48	27 12.01 27 12.24		87 38.71 871 38.94	117 52.06		
28 12.32	58 25.52	88 38.72	118 51.92	28 12.46	58 25.81	88 39.16	118 52.51		
281 12.54	583 25.74	881 38.94	1184 52.14	28 12.68	581 26.03	881 39.38	1185 52.73		
29 12.76	59 25.96	89 39.16	119 52.36 119 <del>1</del> 52.58	29 12.90		89 39.60 891 39.83	119 52.95 1194 53.18		
30 13.20			120 52.80			90 40.05	120 53.40		
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DITON			A 773	4 = 4													
R HOU			AT	45 (	Am't.	S PE	R HC	UR.			AT 4	151/2	CENT	rs P	ER H	OUR	•
m'ı.  Hı	s. Am't	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs.	Am't.	Hrs	Am't
	1 40.27				13.72		27.22	90}	40.72			301	13.88		27.53	903	41.18
14 91	40.49			31	13.95	61	27.45	91	40.95			31	14.10	61	27.75	91	41.40
37 91	2 40.72	2	0.90		14.17		27.67	1	41.17		0.01		14.33		27.98		41.63
	40.94 1 41.16	2.	1.12	32	14.40		27.90 28.12		41.40	2 21/2	0.91	32	14.56		28. 21 28. 44	92	41.86 42.09
	41.38	3	1.35	33	14.85		28.35		41.85	3	1.36	325	15.01				42.31
26   93	1 41.61	$\frac{31}{2}$	1.57		15.07	633	28.57	932	42.07	$\frac{3_{1}}{3_{2}}$	1.59	331	15.24	631	28.89		42.54
48 94	41.83	4,	1.80	34	15.30		28.80		42.30	4	1.82	34	15.47	64	29.12		42.77
	42.05	42	2.02		15.52		29.02		42.52	$4\frac{1}{2}$	2.05	341	15.70		29.35		43.00
	42.27	5 <sub>1</sub> 5 <sup>1</sup> / <sub>2</sub>	2.25	35	15.75		29.25 29.47		42.75	5,	2.27	35	15.92		29.57 29.80		43.22
- 11 -04	42.50 42.72	6	2.70	352	15 97		29.70		42.97 43.20	$\frac{5^{\frac{1}{2}}}{6}$	2.50 2.73	35½ 36	16.38		30.03		43.45 43.68
	42.94	61	2.92		16.42		29.92		43.42	63	2.96	361	16.61	661	30.26		43.91
1 97	43.16	7,	3.15	37	16.65		30.15	97	43.65	7	3.18	37	16.83	67	30.48	97	44.13
6 972	43.39	7 ½ 8	3.37	372	16.87	671	30.37	$97\frac{1}{2}$	43.87	75	3.41	$37\frac{1}{2}$	17.06		30.71	972	44.36
. 11 90	43.61	87	3.60	38	17.10		30.60	98	44.10	8	2.64		17.29	68	30.94		44.59
903	43.83	9	4.05	39	17.32		30.82 31.05		44.32	8½ 9	3.87		17.52		31.17		44.82 45.04
99.	44.05	91	4.27		17.77		31,27		41.55 44.77	$9\frac{1}{2}$	4.32		17.97		31.62		45.27
5 100	44.50	10	4.50	40	18.00		31.50	100	45.00	10	4.55	40	18.20		31.85	100	45.50
7 1003	44.72	105	4.72		18.22	701	31.72	1002	45.22	102	4.78	$40\frac{1}{2}$	18.43		32.08	1002	
101		11	4.05	41	18.45		31.95	101	15.45	II	5.00		18.65		32.30		45.95
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102	45.61	125	5.62	1	19.12		32.40	102	45.90 I	12	5.69	42	19.11	72	32.99	1025	
103	15.83	13	5.85	43	19.35		32.85	103		13	5.91		19.56		33.21	103	
1033	16.06	135	6.07		19.57		33.07	1032		131	6.14		19.79		33.44	1035	47.09
104		14	6.30	44	19.80				46.80	14	6.37		20,02	74	33.67	104	
10414		145	6.52	1	20.02		33.52	1043		145	6.60		20.25	745	33.90	104.	
105 4		153	6.97	45	20.25		33·75 33·97	105	47.25	153	7.05		20.47	75	34.12 34.35	105	47·77 48.00
106 4	7.17	16	7.20	46	20.70		34.20		47.70	16	7.28		20.93	76	34.58		48.23
10614	7-39	165	7.42	465	20.92	761	34.42	1063		164	7.5I		21.16	763	34.81	1061	48,46
107 4	7.61	17	7.65	47	21.15	77	34.65	107	48.15	17	7.73	47	21.38		35.03		48,68
107 4		175	7.87 8, 10	475	21.37		34.87	107		174	7.96		21.61		35.26	108	48.91 49.14
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109 48	3.73	195	8.77	492	22.27	792	35.77	1091	49.27	191	8.87	495	22.52	795	36.17	1093	
	3.95	20	9.00	50	22,50	80	36.00	011	49.50	20	9.10		23.75		36.40		50.05
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113 50			13.35		23.85	83	37.35		50.85	9	10.46		24.11		37.76		51.41
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1154 51.	40	254	11.47	555	24.97	853	38.47	1153	51.97	253	11.60	553	25.25		38.90	1154	52.55
116 51.	62	261	11.70	56	25.20	86	38.70	116	52.20	26	11.83	56	25.48	86	39.13	116	52.78
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1184 52.	73	285	12.82	584	26.32	884	39.82	1187	53.32	283	12.97	581	26,62		40.27	1187	53.92
FIG  52.	95	29	13.05	59	26.55	89	40.05	119	53.55	20	13.19		26.84		40.49	119	54.14
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## PRACTICAL HINTS FOR BUILDERS.

BUILDING SITES—In selecting the site for a dwelling, the preference should be given to one in which the subsoil is naturally dry and the ground elevated, so as to afford facility for getting rid of the seweage and surface water. The best soil is supposed to be gravel or sand. Chalk and other open strata are also good, but clay, particularly if of a retrentive nature, appears less likely to form a healthy site. The rain water is often retained for a long time on the surface of clayey soil, and with some kinds of clay it is absorbed, making the ground cold and damp, causing fogs, which hang over it longer than usual. The sites to be particularly avoided are those in the neighborhood of swamps, or other ground recently reclaimed from rivers, estuaries, or harbours, in which deposits have been formed from mud containing organic matter such as that produced when sewage is allowed to flow into the stream. Slight eminences on the borders of swamps are also frequently unhealthy, according to their position with respect to the prevailing winds. Among hills the unheathly spots are enclosed valleys, any spot where the air must stagnate, ravines, or places at their head or entrance. In well-drained towns the nature of the subsoil is not of so much importance as in the country, owing to the buildings, roads, and pavements preventing the rain water from finding its way below the surface, and from the provision usually made for taking it away rapidly into the streams and water courses. To render a site healthy, the level of the subsoil water should be 8 or 10 feet below the surface, and where this does not occur naturally, drains should be formed to keep it below this depth. The raising of subsoil in malarious districts has been known to cause an outbreak of ague, and the lowering of it by draining has, on the contrary, caused an improvement in the health of the inhabitants. All soils except when saturated with water, contain a large quantity of air, and the more porous the soil the more readily does the air pass through In the case of towns and habitations generally this fact has an important bearing on health, as this air may be drawn into the houses through the ground under the basement, and dangerous consequences ensure if the soil in the neighborhood is saturated with organic matter, which frequently happens when animal excreta has been deposited on the surface or has escaped from sewers and other receptacles. Ground air is invariably damp and where it is permitted to stagnate as in the basements of houses, the growth of fungi is encouraged and the woodwork of the house is destroyed by dry rot; an unwholesome smell pervades the whole house, and the health of the occupants suffers. In all cases, even those in which the natural subsoil has not been disturbed, and whatever may be its nature, the ground under the basement of a dwelling should be rendered impervious to air and moisture. To effect this, nothing appears to answer so well as a layer of good concrete about 6 inches thick—probably the best is that made with well dried gravel and coal tar. Portland cement, both in the concrete and as a rendering over the surface, also answers, but no lime should be used which is not capable of resisting the effects of moisture. Stone slabs or flagging 2 or 3 inches thick, if well bedded and jointed with good mortar or cement, and asplialte not less than 1/2 inch thick, if laid on concrete, may also be used, but they are expensive. Before the adoption of any of these coverings, it is of

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course presumed that the level of the subsoil water is sufficiently below the floor of the basement. Where the subsoil is naturally moist the damp should be pervented from rising through the walls by the interposition of a proper damp course which may be of roofing slates in two thicknesses bedded in Portland cement, or of glazed earthenware such as that sold for the purpose, but the best appears to be a layer of asphalte about 1/2 inch thick through the thickness of the foundation walls. The ground floor of houses where there is no basement should be raised about two feet above the soil, and the space below well ventilated. Where there is a choice, dwellings should be so placed that as many of the rooms as possible may receive the sun's rays during some portion of the day. It is said that a south-eastern aspect is the best for the front of a house; it receives the morning sun. The north and east are usually undesirable aspects to select, owing to the cold winds which usually blow from those points. selection of an aspect, will, however, mainly depend on the climate, and the direction of the prevailing winds. The prospect from the windows of sitting rooms should be cheerful, whether the house be situated in the town or country, as a pleasantprospect assists considerably towards inducing a cheerful state of mind—a matter of no small importance to health.

LAYING FOUNDATIONS.—The foundation of a building, says Sir J. Gowans, is of primary importance, as, unless it is secure, the permanency of the structure cannot be maintained, however well built it may be. Before laying a stone the builder should be satisfied that the strata will give equal resistance to the pressure that may be put upon it. Strata that are hard and soft are very dangerous. Even clay if mixed with bowlders (which often happens) cannot be depended upon, unless they are removed, and means taken to equalize the ground on which the buildings are to be erected. Next to rock, no better foundation can be got than sand or gravel when dry. If wet, means should be taken to drain away the water; but, if this cannot be done, large flat bedded stones of sufficient area, fairly dressed in beds and joints and well put together, will, as the load increases, secure a foundation that anything can be built upon. In my own experience I have often tested this, and particularly when building a bridge on a railway contract I had many years ago. This was an under bridge of considerable span, the girders being in the form of an arch, in segments of cast iron, the security of which depended greatly on the permanent resistance of the abutments, or the bolts which held these segments together at their joints. In digging for a foundation it was found that the strata were very soft, being layers of sand and moss alternately, and to prevent failure I took the precaution to strengthen the foundation of the first abutment by driving piles to a depth of 30 to 40 feet, with horizontal planking, on which the foundation stones were bedded. Before building the second abutment, acting on the advice of a railway contractor who had had more experience than myself, I adopted a different plan-viz., to dig out the soft material to a depth and area as secured an outward resistance to meet the pressure of the large sized stones that were afterwards put into the foundation, course after course, until the load pressed out the water, and so secured a foundation which was equally as strong, if not stronger, than the first. stratum is unequal or not to be depended upon, I know of nothing better than a good bed of concrete not less than 3 ft thick, and no contractor should neglect this where there is the slightest doubt as to the sustaining character of the ground. This is always necessary in erections of different heights, and is particularly required in churches and other buildings where the spire, tower or \*ther elevation bears more beavily on the toundation than the walls which abut

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The best er. ata are also kely to form the surface of ground cold e sites to be other ground eposits have duced when borders of with respect osed valleys, nead or eno much imements pred from the and water uld be 8 or rains should alarious disng of it by of the inge quantity ass through i important the ground n the neighhen animal s and other to stagnate the woodervades the even those nay be its red imperso well as that made oncrete and ised which ging 2 or 3 ement, and

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upon them. And, in addition to this, and to make sure, I would have extra courses in the foundation of the higher and heavier portions, as in the hurry with which we build now-a-days every precaution is necessary. The same care should be taken with respect to the oriel windows or projections which do not go to the full heighth of the building, and consequently have not the same pressure on the foundation. The walls to which these lighter projections are attached should not only be well founded, but the tie or bond which unites the one wall to the other should be left free on the upper beds, so as to allow for the subsidence of the heavier wall without causing the fractures so often seen where this precaution is not taken.

THE STRENGTH OF A WALL.—The strength of a wall depends, of course, largely on the material used. A good, hard burned brick, well laid in cement mortar, makes a very strong wall. To tell a good brick, first examine the color; if it is very light, an orange red, the brick is apt to be soft. If the brick is easily carved with a knife, it is soft. If it can be crushed to a powder easily, it is soft. If two bricks are struck together sharply and the sound is dull, the bricks are poor; if the sound is clear, ringing, metallic, the bricks are good and hard. It brick shows a neat fracture, it is a good sign; a ragged fracture is generally a poor sign. The fracture also shows the evenness of the burning and fineness of the material. A brick that chips and cannot be cut easily is a good brick. darker the brick, the harder burned. This of course, does not hold good for artificially-colored bricks. The straight and more regular the brick, the softer it is (as a rule), as hard burning is apt to warp a brick. What has been said of the strength of brick holds good of terra cotta. The latter should be designed to be of same thickness, if possible, in all parts, and any hollows caused thereby must be filled in solid. It is best to fill in the hollows with bricks and mortar several days in advance, and let the filling set, so as to be sure it will not swell up afterwards and burst the terra-cotta.

CAPACITY OF CYLINDRICAL CISTERNS.—The following table shows the capacity in gallons for each foot in depth of cylindrical cisterns of any diameter:

Diameter.		Gallons.	Diameter.		Gallons
25 ft	• • • • • • • • • • • • • • • • • • • •	3,059			. 239
20 ft		1,958			
15 ft.		. 1,101			. 176
		200		• • • • • • • • • • • • • • • • • • • •	. 122
0		827			
12 ft		705			. 78
II ft		592			11
		1			
		. 396	2 ft.		. 19
8 ft		313			

Power of Walls to Resist Pressure.—Walls laid up of good, hard-burned bricks, in mortar composed of good lime and sharp sand, will resist a pressure of 1,500 pounds per square inch, or 216,000 pounds per square foot, at which figure it would require 1,600 feet height of twelve-inch wall to crush the bottom courses, allowing 135 pounds as the weight of each cubic foot. It also appears from accurate calculations and measurements that walls laid up in the same quality of brick and mortar, with one-third quantity of Portland cement added to the same, are capable of resisting some 2,500 pounds per square inch, or 360,000 pounds per square foot; this would require a height of wall 2,700 feet to crush the bottom bricks.

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ood, hardill resist a tre foot, at crush the . It also up in the d cement lare inch, vall 2,700 To Prevent Moisture from Penetrating Walls.—Sylvester's process for excluding moisture from external walls consists in using two washes or solutions for covering the surface of brick walls, one composed of soap and water and one of alum and water. The proportions are three-quarters of a pound of soap to one gallon of water and half a pound of alum to four gallons of water. Both substances must be perfectly dissolved in water before using. The walls should be perfectly clean and dry and the temperature of the air should not be below 50° Fahrenheit when the compositions are applied. The first, or soap wash, should be laid on when at boiling heat with a flat brush, taking care not to form a froth on the brick work. This wash should remain twenty-four hours, so as to become dry and hard before the second, or alum wash, is applied, which should be done in the same manner as the first.

The temperature of this wash when applied may be 60° or 70°, and it should also remain twenty-four hours before a second coat of the soap wash is put on, and these coats are to be repeated alternately until the walls are made impervious to water. The alum and soap thus combined form an insoluble compound, filling the pores of the masonry and entirely preventing the water from penetrat-

ing the walls. Four coatings will render bricks impenetrable.

HOW TO MAKE GOOD BLUE PRINTS.—The prime requisite for making good blue prints is a good tracing. The best material is tracing cloth. If tracing paper is used it should have a bluish but never a yellowish tint, neither should the paper be too thick, as otherwise light will slant through the tracing under The tracing should have no creases or wrinkles. All lines of the tracing, down to the very finest, should be absolutely black. This is imperative. When ink does not give sufficiently opaque lines, it can be improved by adding burnt sienna, burnt umber or gamboge; this detracts from its quality as a black drawing ink, but makes it better for tracings to be printed. To properly expose prints it is necessary to use a print frame, with strong springs, and an even, thick elastic cushion. To wash prints it is necessary to have water or developer, in a vessel (bath tray) as large as the tracing, and the bath should always be slightly tepid or at least not cold. The prepared paper should be kept and handled only in dim light, and be carefully protected from dampness. To print, the glass in the frame should be kept clean and free from dust. After removing the back of the frame and the cushion, place the tracing on the glass, with the inked side against the glass (or it will give a negative print,) place on that the prepared paper, the prepared side toward the tracing, and see that all lies smooth and is free from creases or wrinkles. Then put the felt cushion on smoothly, and close and lock the frame. Lack of contact produces blurred prints. Examine tracing and paper through the glass in the frame, and if there is lack of contact anywhere correct it by placing paper or pasteboard between cushion and back of frame. When looking at the print to determine time of exposure, open only part of the frame, and raise a corner of the paper. In carrying the print to the bath after exposure, roll it with the blank side out, to protect it from light, and be quick about it.

Experts in house building have suggested that grates in second stories are usually less safe than those below, as the narrower joists give little room for the boxing of the hearth. It is also urged that grates be examined carefully to determine whether the back of the flue is simply of four inch wall, which is always dangerous at the back of a grate in a frame house. This can be determined by measuring the distance the breasts extend out from the wall, and as sometimes the breast runs through flush with the face of the wall in the mext room, the calculation is to be made accordingly.

TO REMOVE EXUDATIONS FROM BRICK Walls—The simplest and least expensive method for removing salt-peter exudation from brick work, when the offlorescence is in position where the sun and wind do not have free access, is to wash it off with diluted hydrochloric or common muriatic acid of commerce. About half a pound of the acid is used with an ordinary pailful of water, the application being made with a sponge.

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Wood soaked for five days in a 7 per cent. solution of phosphate of soda, and after drying suspended for seven days in a 13 per cent. solution of chloride of barium stands the severest test—twelve months in moist earth near a manure pit—without sign of decay or mold.

TO PRESERVE PENCIL DRAWINGS.—Pencil drawings may be rendered ineffaceable by a very simple process. Slightly warm a sheet of ordinary drawing paper, then place it carefully on the surface of a solution of white resin in alcohol, leaving it there long enough to become thoroughly moistened. Afterwards dry in a current of warm air. Paper prepared in this way has a very smooth surface. In order to fix the drawing, the paper is simply to be warmed for a few moments. This process may prove useful for the preservation of plans or designs when want of time or any other cause will not allow of draughtsmen reproducing them in ink. A simpler plan than this is to brush over the back of the paper containing the pencil sketch with a weak solution of shellac in alcohol. Chalk and pencil drawings may be fixed (though not so thoroughly as by the above methods) by washing them with skimmed milk, or with water holding in solution a little isinglass or gum. When the first is used great care must be taken to deprive it of the whole of the cream, as the latter substance would cause the drawing to look streaky. An easy way of applying these fluids is to pour them into a shallow vessel, and to lay the drawings flat upon the surface of the liquid; after which it should be gently removed and placed on white blotting paper in an inclined position to drain and dry.

THE CONSTRUCTION OF CHIMNEYS.—Many able and scientific men have treated on this subject, but the result of their observations serves only to prove what is the result of every day's experience, namely, that rarefied air is lighter and less dense than cold air; and that it will ascend with a velocity proportionate to its rarefaction, unless obstructed by other bodies. Heat that is generated by the combustion of fuel, exists under two distinct forms, and is known by the names of combustible and radiant heat. Combustible heat partakes of smoke, and is carried off with it into the upper regions, while radiant heat is communicated to opposing bodies in contact with its rays. It is stated by some that combustible heat combined with air and smoke exists in the proportion of four to one, compared to radiant heat: but its correct proportion has perhaps never been ascertained. It is, however, certain that very little radiant heat will escape from a smothered combustion, while a dense smoke will very slowly ascend, and sometimes a portion of it is discharged into the room, and the chimney is pronounced smoky, while the epithets uttered against masons, on such occasions, would be more properly applied to the builders of the fire. As nature acts by certain laws, we may derive more profitable information by a proper observance of them, than from accidental occurrences. It is one of the laws of nature that rarefied air ascends, while cold or dense air descends. On the same principle, water discharges itself more copiously through a channel of a uniform and direct surface, on the same inclination. Therefore, channels that are obstructed by eddies and the discharge of other streams into them, are impeded, and the velocity of the water diminished, so as often to produce what is called back-water

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for a considerable distance, which, when removed, permits the water to flow with rapidity. Short bends and turnings also present obstacles to the current or flow of water, by which whirlpools are often seen in actual contact with the natural stream. The same observations may be applied to rarefied air or smoke. Hence those flues will carry smoke the best which arise perpendicularly in a uniform direction. Angles and turnings present obstacles to the progress of the smoke, and should be avoided as much as possible. Particular attention should be paid to the formation of the throat of the chimney, the dimensions of which should in no case exceed the number of square inches contained in a horizontal section of the flue. It has been contended by some that it should be smaller than this, while others have thought that it should be larger; but experience has shown both of these opinions to be erroneous. When the throat is smaller, the frequent rushes of cold air into it from the opening of doors, etc., sends a gush of smoke into the room, by obstructing the upward current of rarefied air. When the throats are larger, eddies are formed in them, and the smoke, becoming dense by the steam of the fuel, chokes the flue, and instead of ascending is puffed into the room. Experience has shown the best construction to be that where the throat contains as many square inches as a section of the flue. If the latter, for instance, is one hundred and forty-four inches wide, the throat should be four feet long, and three inches wide, nearly on a level with the mantle-bar, or at the top of the opening of the fire-place, and graduated to the regular dimensions of the flue.

GLUEING JOINTS.—In general, nothing more is necessary to glue a joint after the joint is made perfectly straight, or, in technical terms, out of winding, than to glue both edges while the glue is quite hot, and rub them lengthwise until it is nearly set. When the wood is spongy, or sucks up the glue another method must be adopted, one which strengthens the joint, while it does away with the necessity of using the glue too thick, which should always be avoided; for the less glue there is in contact with the joints, provided they touch, the better; and when the glue is thick, it chills quickly, and cannot be well rubbed out from between the joints. The method to which we refer is, to rub the joints on the edge with a piece of soft chalk, and, wiping it so as to take off any lumps, glue it in the usual manner; and it will be found, when the wood is porous, to hold much faster than if used without chalking.

THE NEUTRAL AXIS OF CAST-IRON BEAMS.—" It has long been known that under the existing theory of beams, which recognizes only two elements of strength-namely, the resistances to direct compression and extension-the strength of a bar of cast iron subjected to transverse strain cannot be reconciled with the results obtained from experiments on direct tension, if the neutral axis is in the centre of the bar. The experiments made both in the transverse and on the direct tensile strength of this material have been so numerous and so carefully conducted as to admit of no doubt of their accuracy; and it results from them, either that the neutral axis must be at or above the top of the beam, or there must mining the position of the neutral axis be made on such a scale and in such a manner as to place this question beyond doubt, and with this object two beams were cast, 7 feet long, 6 inches deep and 2 inches in thickness. Two were employed in order to avoid errors which might arise from accidental irregularities of the metal. Considering the very minute qualities which had to be measured and the numerous causes of disturbance to which observations of so much delicacy were liable, such as changes of temperature or want of perfect uniformity in the dimensions or texture of the beams, the results point out the

position of the neutral axis as the centre of the beam in a manner so decided as to remove all further doubt upon this subject not only in the smaller strains but in the larger ones also, which, in the case of the second beam, were carried to about three-fourths of the breaking weight."

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SETTING OUT STAIRS.—After determining the height of the riser from the "storey rod," the right proportion of tread must be found. Sometimes steps are arranged so that it is easier for a man to go up "two at a time" than to walk up in a proper manner. The reason is, both tread and riser are made small. When a riser is reduced the tread must be increased, and the contrary when the riser is increased, the tread must be reduced in width. Joiners do not often break this rule, but masons very often do, notably in steps leading to and from railway stations. A simple rule may be given for finding a suitable proportion. Take any suitable step as a standard step, that is to say, if you know of a staircase which is comfortable and easy to walk up, take it as a standard to guage others by. Suppose you have a riser given, and require the width of a suitable tread, make use of the following proportion: As the given riser: standard riser:: standard tread: required tread. If the tread is given and the riser required, then — as the given tread : standard tread :: standard riser : required To work out an example: Suppose 10 inch tread and 7 inch riser be taken as a suitable step, let 6 inch be the given riser; then by substituting the value of treads and risers, for the names we have — As 6": 7":: 10": required tread; this gives 70-6, or 11% for the size of the tread. Nicholson gives as a standard a tread of 12" to a riser of 51/2". Working out the example given by this proportion, we get II" instead of II2/3"; either of these sizes will be an agreeable step. The student will find it a good exercise to compare steps of different buildings with any assumed standard. A rough and ready rule, for the usual sizes of treads and risers, is to make two risers and one tread equal to 24 inches. The proper rule given above may be written for convenience as follows:

standard tread x standard riser

## given tread (or riser)

required riser (or tread, as the case may be.) Pitch boards should be made of hard wood, and should be tested occasionally, for differences in the temperature have a marked effect on the bevel and length of sides. In setting out strings do not depend upon the pitch board for giving the true lengths of the strings. Set a pair of compasses to the length of the hypothenuse of the pitch board, and mark off along the nosing line the number of steps. In this manner accuracy may be ensured. It is a good practice to mark on the strings and on the drawing the word UP after the number of steps which lands in each flight. This simple habit will prevent the not uncommon mistake of putting a step too many in a flight of stairs. In ramping strings to fit each other, it is necessary for the ramps to finish at right angles to the joints; if they do not, the mould on one string will not intersect with that on the other. The ends of the strings must be prepared to receive skirting before leaving the bench, and it is best generally for the joiner who makes the stairs to work sufficient skirting (to match his strings) to skirt the intermediate landing and the main landings to the nearest architrave. strings must be gauged, so that there may be a proper margin for plaster. well strings must be 1/8 inch below the treads or carriages, as the case may be and the wall string must be flush with them.

SETTING-OUT WINDERS AND NEWELS.—Winders require particular care in setting out; each window must be separately considered; for if not, the beginner will have great difficulty in fixing. When strings are tongued and grooved

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rogether, simply putting the tongue on one of them instead of the other will save a great deal of trouble in fixing. This is more particularly the case when the winders finish to a newel or into a solid well. Some times it is a good plan not to "glue up" the winders, for often these winders can be put in when not jointed to the riser. Too often winders are confined to newels at the narrow ends. When winders are set out by means of a falling mould, they are properly arranged and "eased" before reaching the springings. Now the winders round a newel should be similarly situated, although there is no wreath to take into consideration for the steps should begin to narrow gradually. If there is not room in the staircase to move the flyers back for this, the tread of the flyers must be reduced; a slight reduction will answer the purpose generally. It is a disavantage to have an easy flyer and a steep winder in the same flight of stairs; the pitch should be as uniform as possible.

PUTTING STAIRS TOGETHER.—Before wedging up both strings of a flight of stairs in which there are winders, or curved steps to newels, or solid wells, every winder or tread which cannot be fixed after the newel or well is on, must be in position. This is often forgotten by experienced hands and causes a great deal of trouble, such as the breaking of the joint of a winder and riser. If the flight has a cut string, each tread should be screwed to the riser of the next step before the treads are wedged to the wall string; if the steps are screwed up like this, they can hardly be wedged up "out of square." Strong pieces should be aramped against the nosings until the string board is fixed, blocked and set. The wall strings should be blocked to the treads and risers. Blocks hold the wall string much better than nails through the string into the treads.

SAFE HEIGHTS AND LENGTHS OF BRICK WALLS.—For first-class buildings (the workmanship being good) as a geneaal average the walls should not exceed a greater number of feet in height, than three times their thickness in inches, and the length should not exceed double the neight, without lateral support or triffening by pillasters, buttresses or wing walls say for.

Where the length must exceed these distances, as in depots, warehouses, etc., the thickness must be increased, or lateral braces provided (such as plasters or buttresses) and at as short intervals as practicable.

To Make a Very Strong Glue.—An ounce of the best isinglass may be dissolved, by the application of a moderate heat, in a pint of water. Take this solution and strain it through a piece of cloth, and add to it a proportionate quantity of the best glue, which has been previously soaked for about four and twenty hours, and a gill of vinegar. After the whole of the materials have been brought into a solution, let it once boil up, and strain off the impurities. This glue is well adapted for any work which requires particular strength, and where the joints themselves do not contribute towarns the combination of the work, or in small fillets and mouldings, and carved pattern that are held on the surface by the glue

#### STRENGTH OF STONE MASONRY.

BY PLOF. I. O. BAKER.

The universal custom in determining the ability of stone to resist pressure is to test the compressive resistance of small cubes. The results obtained by testing small specimens of stone are very useful in determining the relative strength of different kinds of stone, but such results are of no value in determining the ultimate strength of the same stone when built into a masonry structure. strength of a mass of masonry depends on the strength of the stone, on the size of the blocks, on the accuracy of the dressing, on the proportion of headers to stretchers, and on the strength of the mortar. A variation in any one of these items may greatly change the strength of the masonry. The importance of the mortar as affecting the strength of masonry to resist direct compression, is generally overlooked. The mortar acts as a cushion between the blocks of stone, and if it has insufficient strength it will squeeze out lateraly and cause a tensile strain therefore weak mortar causes the stone to fail by tension instead of by compres Stone is several times stronger to resist compression than tension, and hence, where great strength is required it is necessary that the mortar should b of the best.

No experiments have ever been made, for obvious reasons, upon the strengt. of stone masonry under the conditions actually occurring in masonry structures; but experiments made upon brick piers 12 inches square and from 2 to 10 feet high, laid in mortar composed of one volume Portland cement and two of sand show that the strength per square inch of the masonry is only about one-sixtly of the strength of the brick. An increase of 50 per cent. in the strength of the brick produced no appreciable effect on the strength of the masonry; but the substitution of cement mortar—one part Portland cement and two sand—for lime mortar-one part lime and three parts sand-increased the strength of the masonry 70 per cent. The method of failure of these piers indicates that the mortar squeezed out of the joints and caused the brick to fail by tension. Since the mortar is the weakest element, the less mortar used the stronger the wall; therefore the thinner the joints and the larger the blocks, the stronger the masonry, provided the surfaces of the stone do not come in contact. It is generally stated that the working strain on stone masonry should not exceed onetwentieth to one-tenth of the strength of the stone; but it is clear from the experiments on brick piers referred to above, that the strength of the masonry depends on the strength of the stone only in a remote degree.

In a general way it may be said that the results obtained by testing small cubes may vary 50 per cent. from each other, or say 25 per cent. from the mean, owing to undetected differences in the material, cutting and manner of applying the pressure; and also that stones crack at half of their ultimate crushing strength. Hence, when the greatest care possible is exercised in selecting and bedding the stone, the safe working strength of the stone alone should not be regarded as more than one-fourth to three-eighths of the ultimate strength. A further allowance, depending upon the kind of structure, the quality of mortar, the closeness of the joints, etc., should be made to secure safety. Experiments upon comparatively large specimens are but little help in deciding this question; the only way is to determine the load carried by actual structures. The following are the greatest loads carried by stone masonry, that were discovered by an

extended search through engineering literature:

Early builders used much more massive masonry, proportional to the load to be carried, than is customary at present; experience and experiments have shown that such great strength is unnecessary. The load on the monolithic piers supporting the large churches in Europe does not exceed 30 tons per square foot

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e load to ve shown iers supiare foot (420 pounds per square inch) or about one-thirtieth of the ultimate strength of the stone alone. The stone-arch bridge of 140 feet span at Pont-y-tu-Prydd, over the Toff, in France, erected in 1750, is supposed to have a pressure of 20.7 tons per square foot (290 per square inch) on hard limestone rubble masonry laid in lime mortar. A former bridge at the same place failed with 64 tons per Rennie subjected good hard limestone rubble in columns 4 feet square to 22 tons per square foot, (300 pounds per square inch). The granite piers of the Saltash bridge (England) sustains a pressure of 9 tons per square foot (125 pounds per square inch). The maximum pressure on the granite masonry of the Brooklyn bridge is about 281/2 tons per square foot (about 400 pounds per square inch); the maximum pressure on the limestone masonry is about ten tons per square foot (125 pounds per square inch). The face stones ranged in cubical contents from ½ to 5 cubic yards; the stones of the granite backing averaged about 1½ cubic yards, and of the limestone about 1¼ cubic yards per pier. The mortar was I volume of Rosendale cement and 2 of sand. The stones were rough axed or pointed to half-inch bed joints, and half-inch vertical face-joints. These towers are very fine examples of the masons' art. pressure on the limestone piers of the St. Louis bridge was, before completion, 38 tons (527 pounds); after completion the pressure was 19 tons (273 pounds) on the piers and 15 tons (198 pounds) on the abutments. The limestone masonry in the towers of the Niagara Suspension bridge failed under 36 tons per square foot, and were taken down; however, the masonry was not executed. At the South Street bridge, Philadelphia, the pressure on the rubble masonry in the pneumatic piers is 15.7 tons per square foot (220 pounds) at the bottom and 12 tons at the top; this is unusually heavy, but there are no signs of weakness. The maximum pressure on the rubble masonry and cement mortar of some of the large masonry dams is from 10 to 14 tons per square foot. The proposed Quaker bridge dam, which is to impound water for New York City, and which is the largest in the world, is designed for a maximum pressure of 16% tons per square foot on massive rubble masonry in best hydraulic cement mortar.

#### HOW TO TEST THE DRYNESS OF WALLS.

A curious device for resisting the dryness of walls is described in the Wiener Banindustrie-Zuitung. The apparatus for the purpose consists simply in small sheets of gelatine, which are made by taking the sheet-gelatine of commerce, selecting the thinnest pieces, soaking them for a quarter of an hour in water until they are quite soft, spreading them out flat on a greased sheet of glass, and stretching them with the fingers until the folds and creases are smoothed out and the whole made as thin and uniform as possible. The sheets are then thoroughly dried in the air, the edges, which are rough and uneven, are trimmed off, and the whole cut into pieces about two inches wide and four inches long, for use in testing. If kept flat in a dry place, these gelatine strips are very sensitive to moist air. If a wall is suspected of being damp, a strip is moved slowly over it near its surface, but not touching it. If any damp spots exist, they are immediately shown by the curling of the gelatine as it passes near them. Although every one takes some interest in knowing whether his house is dry or not, this simple test is likely to be of more practical use to fresco-painters and paper hangers than to any one else. Both of these, to avoid disappointment and loss, need to know with certainty whether the walls and ceilings on which their art is to be exercised are dry or not, as their paper and colors will often change on damp plastering. In the case of a wall of m asonry, particularly, plastered on the brick work it is difficult to tell by ordinary inspection whether the moisture has dried out or not, and the gelatine sheets may give the desired information with such certainty and precision as to be of great service.

# GRAPHIC METHOD OF DETERMINING THE STABILITY OF A PIER OR BUTTRESS.

X C

Let A B C D represent a pier which sustains a given thrust T, at B.

Draw the indefinite line BX in the direction of the thrust through the centre of gravity of the pier (which in this case is at the centre of the pier), draw a vertical line until it intersects the line of the thrust at E. As a force may be considered to act anywhere in its line of direction, we may consider the thrust and the weight to act at the point E; and the resultant of these two forces can be obtained by laying off the thrust T from E on E X, and the weight of the pier W from E on the line E Y both to the same scale (pounds to the inch) completing the parallelogram and drawing the diagonal. If this diagonal prolonged cuts the base upon the outer edge, the pier will be unstable and its dimensions must

be changed. The stability of a pier may be increased by adding to its weight by placing some heavy material on top or by increasing its width at the base by means of "set-offs."

[Kidder.]

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#### BONDING WITH HOOP IRON.

Hoop iron is in narrow strips about an inch in breadth, and for bonding is generally dipped in hot tar before being built into a wall to preserve it from rust. It is often laid in the centre of a thin wall, but there should be one hoop iron to every half brick in thickness of wall. Two courses of hoop iron should be built into every story, one below the window sill and one above the head, but where the expense can be permitted it may be placed in every few courses of brick. The ends of the iron lengthways should overlap and be hooked together, and at angles of wall the iron should also be hooked together.

#### WATERPROOFING STONEWORK.

Coal tar is recommended for waterproofing masonry. For exposed surfaces apply from one to three coats boiling hot. By adding a small quantity of India rubber dissolved in benzine the coating will last longer. To whiten the color, dust with plaster of Paris before dry. For surfaces to be covered by earth a single coating of tar made thick by blazing is preferable; two or three gallons should be boiled and lighted when boiling. While blazing stir continually till volume is reduced and becomes pasty in cooling. Spread over stone rapidly with large flat brush.

#### BOILER SETTING.

Brickwork for boiler setting is very different from ordinary brickwork. The joints should be very thin and both inside and out must be very carefully executed. Kaolin or prepared fire clay should be used as mortar for the fire brick and mixed so thin that it must be put on with iron spoons instead of trowels. The fire brick should be dipped in water before laying so that it will take up the water in the cement. Every sixth course beginning with the grates should be headers well bonded into the rear work; they must be well bonded into the setting to hold upper part of wall in position as the lower courses of fire brick burn away. This admits of replacing of fire bricks without rebuilding the wall

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#### LIMESTONES.

Limstones are for many reasons eminently suitable for constructive purposes, being cheap and easily worked, but they readily absorb moisture. This moisture usually contains carbonic acid and gradually dissolves the stone, and in winter serious injury is often caused by the freezing of the moisture and its subsequent expansion. Recent experiments prove that metallic fluosilicates, more especially those of aluminium, magnesium and zinc, are successful preservatives. The surfaces should be brushed over with a solution of salt chlorine, causing on first application an abundant froth, due to the liberation of carbonic acid gas. When dry, wash over again once or twice according to the quality of the stone; on an average, 1.7 pounds of solution to 40° Beaume are required per cubic yard. The process is completed in 24 hours. This treatment admits of polishing of the stone, and by a suitable choice fluosilicate used, different colors can be communicated to the stone. The process is cheap and good alike for mortars and cement containing lime.

#### SAND FOR CONCRETE OR MORTAR.

Sand when rubbed in the hand should give a dry crackling sound and be prickly to the skin. Clean and sharp sand if wetted and taken up in the hand, after being tightly held will not, on being loosed, retain the shape nor soil the hand.

[T. M. Clarke]

#### INVERTED ARCHES.

A foundation or basement of piers constructed with badly formed inverted arches is vorse than if the arches had been omitted altogether. They are introduced for the purpose of distributing the superincumbent weight and are intended to obviate a solid wall. By means of the inverted arch the whole weight of the building is distributed over the whole foundation, and it is necessary that the outer arch at each end of the wall should have a sufficient abutment to prevent the arch pushing out the final piers. The inverted arch, when used, should be the full thickness of the piers against which it abuts. Semi-ellipse is a good form, but it should be built of particularly good brick set in quick setting cement, the cement joints being very fine. The rims of the arch should not be more than half brick in height, and nothing should be built upon the intrados. If it is necessary to build solid above the arch a stone should be inserted above the abutment of the arch, and the upper work should rest on it. Great care must be taken to form the abutment, which must be above the top course of the footings of the piers.

#### ARCHES.

In constructing arches of wide spans, an important factor of safety is the thickness of the material from the intrados to the extrados, especially at the crown or keystone. Several rules for determining the depth of the keystone have been devised, but there is a very wide difference between them. Prof. Rankine's rule, however, seems to answer for most purposes: "Take the mean proportional between the inside radius at the crown and 0.12 of a foot for a single arch, and 0.17 of a foot for an arch forming one of a series.

Formulæ:

Depth of keystone for single arch in feet =  $\sqrt{(0.12 \times \text{radius at crown.})}$ Depth of keystone for an arch of a series in feet =  $\sqrt{(0.17 \times \text{radius at crown.})}$ 

#### LAMINATION IN STONE.

All sandstones may be grouped in three ways: (1) those that are very hardvery compact, very fine in grain, and generally speaking of a pale color; (2) those that are durable but costly; (3) those that are hard and laminated. That is the character that belongs to almost all rocks that have been found in water, but in some it shows itself more markedly than in others. In stones that are completely laminated the appearance is like sheets of paper placed one over the other, and you can almost separate them. These stones are easily perishable. There are, however, laminated stones of good quality. These are generally not of fine grain, although they may be so. They are most frequently of a mixed grain made up of particles of sand and small pebbles of different sizes. Very often they have reddish lints of color owing to the presence of iron, and they are often very irregular in their character. There are some soft stones which are laminated and generally red, but soft and bad. These are the three kinds of sand stones. They are subject to decay in this way: First, from lamination. Having been formed in water, they have been deposited in beds one over the other and never become entirely free from water, and when exposed to the air are liable to give off the water by evaporation and take it again by absorption when rain comes or when the air is damp. After this if a change of temperature follows and a severe cold sets in, the temperature of the stone passing below the point of the extreme density of water, the water begins to expand. Then again, water contains foreign substances floating in the atmosphere which are soluble in water. These substances include a large number of gases; acid gases for the most part, but some others. For example, they include carbonic acid gas, and carbonic acid dissolves in water; they include also sulphuric acid, and this is taken up by the water; also sulphate of ammonia, all these substances being produced in the atmosphere of large towns. These substances entering into the body of the stone begin to act upon the cementing medium. If the cementing medium is easily acted upon chemically by these substances, it is, of course, very soon removed. If it is not easily affected by them, then the stone remains unaltered; but generally speaking, it is the case that sandstones that have either lime or clay as their cementing medium are more or less affected by foreign substances entering into them through the atmosphere. There is then a cause of decay in the sandstones, and the sandstones, when they are very absorbent, generally become readily disintegrated in this manner.

# TABLE SHOWING PERMISSIBLE LOADS UPON VARIOUS KINDS OF FOUNDATION BEDS, PER SQUARE FOOT.

Rock foundations, 4,000 to 40,000 lbs., aver	20,000 lbs.
Coarse gravel and sand2,500 to	3,500 lbs.
Clay	4,000 lbs.
Concrete	8,000 lbs.
Piles in artificial soil, for each pile	4,000 lbs.
Piles in firm soil, for each pile30,000 to 1.	40,000 lbs.
[Kidder	s Pocket Book.]

#### THICKNESS OF WALLS IN DWELLING HOUSES—BRICK.

e very hard Length up to 45 feet. Length up to 80 feet. Length unlimited. Height | e color; (2) up to ated. That Two stories 21 1/2 in. One story 30 100 Two stories 26 in. nd in water, Three 11 171/2 in. feet. 21½ in. Two stories 26 in. 11 hes that are Remainder 13 in. 21 ½ in. 17½ in. 11 one over the Remainder 13 in. 17½ in. perishable. Remainder 13 in. enerally not of a mixed sizes. Very Height Length up to 45 feet. Length up to 70 feet. Length unlimited. and they are s which are up to Two stories 211/2 in. One story 30 One story 26 90 in. in. ee kinds of feet. Two stories 26 Two stories 21 ½ in. in. lamination. 17 ½ in. Remainder 13 in. 17½ in. One story 21 1/2 in. ne over the Remainder 13 in. Two stories 17 1/2 in. d to the air Remainder 13 emperature g below the Length unlimited. Height Length up to 40 feet. Length up to 60 feet. are soluble up to Two stories 21 1/2 in. 85 One story 21 1/2 in. One story 26 in. feet. Two stories 27 1/2 in. Two stories 21 1/2 in. 17 ½ in. Remainder 13 in. and this is Remainder 13 17½ in. Remainder 13 , of course, Height Length up to 40 feet. Length up to 55 feet. Length unlimited. ne remains up to have either Two stories 17 1/2 in. One story 26 One story 21 1/2 in. 70 by foreign feet. Remainder 13 in. Two stories 171/2 in. Two stories 21 1/2 in. e is then Remainder 13 in. One story 17 in. re very ab-Remainder 13 Height Length up to 30 feet. Length up to 50 feet. Length unlimited. up to Two stories 171/2 in. 60 One story 171/2 in. One story 211/2 in. KINDS feet. Remainder 13 in. Remainder 13 in. Two stories 17 1/2 in. Remainder 13 in. S. Height Length up to 30 feet. Length up to 45 feet. Length unlimited. up to 50 Wall below the top One story 171/2 in. One story 21 1/2 in. feet. Rest of wall below 17 1/2 in. story 13 in. S. Top story 8½ in. Remainder 13 in. top story 13 in. S. Remainder 81/2 in. Top story 81/2 in.

Remainder

8 1/2 in.

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Book.]

# TABLE SHOWING NUMBER OF BRICKS IN WALLS OF VARIOUS THICKNESSES.

Super- ficies				тніс	KNES	s of	WALL	S.		
of wall	4½ in. o ½ bric	r 9 in. o k i brick	r 13 in. oi 1½ brick	18 in. o 2 brick	r 22 in. o 2½ brich	r 26 in. o k 3 briel	or 30 in. o	r 35 in. c k 4 bricl	or 39 in. o k 4½ brick	r 44 in. or 5 brick
ft in					١,				1.	
0.6	1	7	10		17		24			
1.0		14	21	28	35	42	49			70
1.6	-	21	31 1/2		52		73		94	-1
2.0		28	42	56	70	84	98	112	126	140
2.6		35	521	70 84	87	105	122	140	157	
3.0	$\begin{array}{c c} 21 \\ 24\frac{1}{2} \end{array}$	42	63		105		147		189 220	210
3.6		49 56	73½ 84	112	1222	147	196	224	252	245 280
4.6		63	$94\frac{1}{2}$	,	1571	1	220		2831	1
5.0		70	105	140	175	210	245	280	315	315
5.6		77	1151		1925	1	269		346	
6.0		84	126	168	210	252	294	336	378	420
6.6		91	1361	182	227		318	364	4092	
7.0		98	147	196	245	294	343	392	441	490
7.6		105	1572	210	2623		367\frac{1}{2}		4722	
8.0		112	168	224	280	336	392	448	504	560
8.6		119	1783	238	$297\frac{1}{2}$	357	416	476	5352	595
9.0		126	189	252	315	378	441	504	567	630
9.6	$66\frac{1}{2}$	133	$199\frac{1}{2}$	266	$332\frac{1}{2}$	399	465		5982	665
10.0	70	140	210	280	350	420	490	550	630	700
15.0	105	210	315	420	525	630	735	840	945	1050
20.0	140	280	420	560	700	840	980	1120	1260	1400
30.0	210	420	630	840	1050	1260	1470	1680	1890	2100
40.0	280	560	840	1120	1400	1680	1960	2240	2520	2800
50.0	350	700	1050	1400	1750	2100	2450	2800	3150	3500
60.0		840	1260	1680	2100	2520	2940	3360	3780	4200
70.0	490	980	1470	1960	2450	2940	3430	3920	4410	4900
80.0	560	1120	1680	2240	2800	3360	3920	4480	5040	5600
90.0	630	1260	1890	2520	3150	3780	4410	5040	5670	6300
100.0	1	1400	2100	2800	3500	4200	4900	5600	6300	7000
200.0		2800	4200	5600	7000	8400	9800	11200 16800	12600	14000
300.0 400.0		4200 5600	6300 8400	8400	10500	12600	14700 19600			21000 28000
		7000		11200 14000		21000		22400 28000	-	
500.0	0.0	8400		4		25200	24.500	33600		35000
600.0 700.0						29400		39200		42000 49000
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# TABLE OF KEYSTONES FOR ARCHES OF FIRST-CLASS CUT STONE.

(From Trautwine's C. E. Handbook.)

For second-class cut stone add about one-eighth part. For good rubble or brick add about one-fourth part.

Span		RISE	E IN PA	RTS OF	THE S	PAN.	
feet.	1/2	1/3	1/4	1/5	1/6	1/8	1/10
	Key ft.	Key ft					
2	0.55	0.56	0.58	0.60	0.61	0.64	0.68
4	0.70	0.72	0.74	0.76	0.79	0.83	0.88
4 6	081	0.83	0.86	0.89	0.92	0.97	1.03
8	0.91	0.93	0.96	1.00	1.03	1.09	1.16
10	0.99	10.1	1.04	1.07	1.11	1.18	1.26
15	1.17	1.19	1.22	1.26	1.30	1.40	1.50
20	1.32	1.35	1.38	1.43	1.48	1.59	1.70
25	1.45	1.48	1.53	1.58	1.64	1.76	1.88
30	1.57	1.60	1.65	1.71	1.78	1.91	2.04
35.	1.68	1.70	1.76	1.83	1.90	2.04	2.19
40	1.78	18.1	1.88	1.95	2.03	2.18	2.33
50	1.97	2.00	2.08	2.16	2.25	2.41	2.58
60	2.14	2.18	2.26	2.35	2.44	2.62	2.80
80	2.44	2.49	2.58	2.68	2.78	2.98	3.18
100	2.70	2.75	2.86	2.97	3.09	3.32	3.55
120	2.94	2.99	3.10	3.22	3.35	3.61	3.88
140	3.16	3.21	3.33	3.46	3.60	3 87	4.15
160	3.36	3.44	3.58	3.72	3.87	4.17	
180	3.56	3.63	3.75	3.90	4.06	4.38	
200	3.74	3.81	3.95	4.12	4.29		
220	3.91	4.00	4.13	4.30	4.48		
240	4.07	4.15	4.30	4.48			
260	4.23	4.31	4.47	4.66			
280	4.38	4.46	4.63				
300	4.53	4.62	4.80	1	I	1	1

#### CRUSHING STRENGTH OF BRICKWORK.

.[F. E. Kidder.]

Piers, uniform size, 8" x 12" x 22 ½" high, common mortar, good average quality, Cement, pure Portland, ½ inch thick under and on top of each pier. Age of piers, 4 months 26 days, exposed to air only, not in water.

Ultii strength  Strength  Pressure in. whe shewed of yiel Tons pe	Pressur in. w shewe of yield porne first y
lbs. lbs. tor	s. lbs. tons.
Lime mortar, plain	52 833 59
Lime mortar 3 parts, Rosendale cement, 1 part 245,000 2,552 1,354 9	2 1,354 97
Lime mortar 3 parts, Roman cement 1 part. 195,000 2,030 1,041 7	30 1,041 75
Portland cement 1 part, sand 2 parts 240,000 2,500 1,302 9	O 1,302 93
Newark and Rosendale cement I part, sand	
	35 708 51
Roman cement 1 part, sand 2 parts	7 1,770 127

Results of from 30 per cent. to 50 per cent. lower have been obtained by previous competent authorities.

Bricks used for the Indiana Statehouse (F. W. Vogdes' Sup.) specimens having been rubbed to parallel faces and averaging 4" x 8" x 2 ½" = 32" sup., were tested separately without mortar and bore a pressure of 55 tons before yielding, crushing under 80 tons; this was the lowest grade admitted, the pressure being equal to 5,600 lbs. per square inch. Of the specimens tested, the results varied from crushing under 40 tons to remaining uninjured while bearing a pressure of 97½ tons (equal to 6,825 lbs. per sq. in.) for 15 minutes. Of Philadelphia facebrick (best) used in Municipal Buildings, Philadelphia, the tests gave for one specimen 15,240 lbs. per square inch and another 10,240 lbs. per square inch.

[Vogdes' "Pocket Companion."]

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#### TO PREDETERMINE EFFLORESCENCE ON BRICKWORK.

It is possible to determine in advance of their use whether bricks will effloresce. Knowing this, architects and builders should permit the use of none that will not stand the test. At the Royal testing station of building material in Berlin bricks are gradually heated to the boiling point in a water bath and are then suddenly immersed in cold water. They are boiled for one hour in a 15 per cent. solution of common salt and frequently cooled as before. They are again boiled half an hour in 5 per cent. soda lye. They are further boiled half an hour in the same solution with the addition of I per cent. of ammonium sulphate. They are then boiled half an hour in a solution containing 2 per cent. blue vitriol and 10 per cent. common salt. Fragments of the brick are placed for 75 hours in 3 per cent. hydrochloric acid, and for 50 hours more in 5 per cent. hydrochloric acid. further treatment of the fragments with pure 4 per cent. hydrochloric acid a fluid clear as water is formed, which when treated with barium salts should not show the presence of sulphates, which are the cause of efforescence. These tests determine the quality of the bricks as well, and none which fail to stand the test should be used. It may be only necessary to test a few samples of brick from each brick yard when the general quality of the clay from which the bricks are made will be ascertained.

Table Showing Diameter and Height of Chimney for any Boiler.

(Kidder's Hand Book.)

		1			
Horse power of boiler,	Height of chimney in feet.	Interior diameter at top.	Horse power of boiler.	Height of chimney in feet.	Interior diameter at top.
10 12	60 75	14 inches.	70 80	120 120	30 inches.
- 16 20	90 99	16 11	120 160	135 150	38 11 43 11
30 50	105	2T 11 26 11	200 250	165 180	47 11 52 11
60	120	27 "	380	195	57 11

#### GENERAL RULES FOR BRICK CHIMNEYS.

(Molesworth.)

The diameter at the base should be not less than one-tenth of the height. Batter of chimneys 0.3 inch to the foot.

THICKNESS OF BRICKWORK:

One brick, from top to 25 feet from top.

Brick and a half, from 25 feet from top to 50 feet from top. Increase thickness by half a brick for each 25 feet from top.

If the inside diameter at top exceeds 4 feet 6 inches the top length should be a brick and a half.

AREA OF CHIMNEYS:

Q=Quantity of coal consumed per hour in lbs.

H = Height of chimney in feet.

P=Indicated horse power of engine.

A=Area of chimney at top in square inches.

 $A = \frac{15 \text{ Q}}{1.00 \text{ P}} = \frac{150 \text{ P}}{1.00 \text{ P}}$ 

The area for entrance of air to ash pit should be ¼ the area of grate, 2 feet 6 inches is sufficient depth. The grate bars inclining downwards 1 in. per foot, not more than ¾ in. thick and ¾ to ½ in. spaces between. The furnace should have 3 cubic feet of space above each superficial foot of grate bar surface.

[Yones & Laughlin.]

erage quality,
of each pier,
ter.

shewed first sign of yielding.	Tons per sq. feet borne when piers first yielded.
bs.	tons.
333	<b>5</b> 9
354	97 75 93
04 I	75
302	93
'o8	51
'08 '70	51 127

1,500 lbs.

216,000 lbs.

2,500 lbs.

360,000 lbs. ned by pre-

specimens
" sup., were
re yielding,
ssure being
sults varied
pressure of
elphia faceve for one
are inch.

tpanion."]

# TABLE OF SAFE LOADS,

UNIFORMLY DISTRIBUTED, FOR WHITE PINE BEAMS, SUPPORTED AT BOTH ENDS IN TONS OF 2,000 POUNDS.

		7 in.	2.0.4 4.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0
EEP.	ن ا	6 in.	2.8. 2.4.4.6. 2.4.4.6. 2.4.7.6. 3.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.
BEAMS 7 INCHES DEEP.	Breadth.	5 in.	6.8 3.4.5 2.2.2 2.2.2 2.1.1 1.1.2 1.1.1 1.2.1 1.3.1 1.
MS 7 IN		4 in.	4.6. 4.6.
BEA		3 in.	2.7. 2.1. 1.6. 1.6. 1.2. 1.3. 88.  7.9  7.9  7.5  7.5  7.5  7.5  7.5  7.5  7.5 7.5
	Clear span in	feet.	2 & 4 200 5 8 9 0 1 1 1 4 5 8
di di		6 in.	6. 33. 2.4 2.4 2.4 2.4 2.4 2.4 2.4 2.4 2.4 2.
S DEE	Breadth.	5 in.	3.3 2.5 2.5 1.1 1.1 1.1 1.2 1.3 1.4 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5
5 INCHI	BRE	4 in.	4
BEAMS 6 INCHES DEEP		3 in.	3. 1.5. 1.2. 1.2. 1.5. 2.5. 2.5. 2.5. 2.5
	Clear <b>s</b> pan in	feet.	2 & 4 20 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1

DEEP.  DEEP.  17.1 A. A. THI.  18.1 A. A. THI.  18.2 A. A. THI.  18.3 A. THI.  18.4 A. THI.  18.4 A. THI.  18.5 A.	DEEP.    A. Deep.	DEEP.   BEANS	DEELP.   BEANS 97 200	DEEP.   BEAMS 9" DEEP.	DEELP.   BEANS 97 200	- b 7	8,45ç	1025	0 4 4 0 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Span 3" 4	BEAD
P. 65 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	F. 6544 553 7	P. BEAMS    Clear   Span   Spa	P. BEANS 97 P. Clear Span Span Span Span Span Span Span Span	P.   Clear   BEANS 9" DEEP,   Span   3"   4"   5"   6"   7"   7"   8"   in th.   3"   4"   5"   6"   7"   15.3   4.1   5.2   3.6   4.5   4.5   5.6   6.7   7"   15.5   5.6   5.7   5.6   5.7   5.6   5.7   5.6   5.7   5.6   5.7   5	P.   DEANS OF 2000 bs.   P.   BEANS OF 2000 bs.   BEANS OF DEEP.   BEANS O	16 or 16 or	1545	1615	0 5. <del>1.</del> 6	4"   5"   6	supported BEAMS 8" DE
Clear   RE   Span   S	Clear BEANS (	35 0 4 5 5 7 5 8 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	7 200 1 200	BEANS 9" DEEP.  BEANS 9" DEEP.  BEANS 9" DEEP.  BEANTH.  4" 5" 6" 7"  4 " 5" 6" 7"  4 " 5" 6" 174  4 5 56 57 74  5 6 57 74  5 6 57 74  5 7 7 7 7  5 7 7 7  5 7 7 7  5 8 7 7 7  5 8 7 7 7  5 8 7 7 7  5 8 7 7 7  5 8 7 7 7  5 8 7 7 7  5 8 7 7  6 8 7 7  6 8 7 7  6 8 7 7  6 8 7 7  6 8 7 7  6 8 7 7  6 8 7 7  6 8 7 7  6 8 7 7  6 8 7 7  7 8 8 7  6 8 7 7  7 8 8 7  6 8 7 7  7 8 8 7  6 8 7 7  7 8 8 7  8 8 7 8 7  8 8 7 8 7  8 8 7 8 7	BEANS 9" DEEP,		12228	4468 6691	• Į	1 1~	
5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	#EAMS of # 112222 # 4 # # # # # # # # # # # # # # # #	25 0 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	72 200 1 200	BEANS 9" DEEP.  BEANS 9" DEEP.  BEANS 9" DEEP.  BEANTH.  4"   5"   6"   7"    4"   5"   6"   7"    4   5"   6"   7"    5   6"   7"    5   6"   7"    6   7"   7"    6   7"   7"    7   7   7"    8   7   7"    8   7   7"    9   1   1   1   1    10   1   1   1    11   1   1   1    11   1	10 of 2000 bs.  10 of 2000 bs.  10 of 2000 bs.  10 of 2000 bs.  11 of 2000 bs.  12 of 2000 bs.  13 of 2000 bs.  14 of 2000 bs.  15 of 2000 bs.  16 of 2000 bs.  16 of 2000 bs.  17 of 2000 bs.  18 of 2000 bs.	8822	1222			Clear span in ft.	ends, i
	AMA	BH 57 6 120 121 121 121 121 121 121 121 121 121	SAMS 9" DEE BREADT 5 6 67 1 15 6 67 1 15 25 25 25 25 25 25 25 25 25 25 25 25 25	bs. EEF. 177. 177. 177. 177. 177. 177. 177. 17	bs.   cr.		.g=6				BI

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	EEAMS 10" DEEP.		_	BEAMS	dS 11	-	DEEP		
Clear	BREADTH.	Clear			H.	BEAUTH	YIII.		
in ft.	4"   3"   6"   7"   8"   9"   10"	in ft.	Üţ.	6"	-1	8	9	10"	11"
- 1	9.7 11.1 12.4	4.17	7.4	» (C.	92.7	13.4	15.1	16.7	18.4
<b>_</b>	4.6 5.5 6.4 7.3	0,0	5.6	6.7	· .	8.9	5		12.2
	3.9 4.7 5.5 62 -7.1	-1	4.8	2,	6.	6	8.6		10.5
	3.4 4.1 4.8 5.5 6.	200	4:	0	000	6.6	:5		9.9
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	1.6 2 2.3 2.6 29	6	.2	24	2.8	3	3.6		4.4
18	1.4 1.7 2 23 2.6	18	1.7	2.1	2.4	2.8	3.2		ده 00
3	1.3 1.5 1.8 2   2.3	8	1.5	1.9	2.9	2.5	2.8		3.
23	1.3 1.6 1.8 2	23	1.4	1.7	9	22	2.5		ယ
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Name and Address of the Owner, where the Party of the Owner, where the Party of the Owner, where the Owner, which is the O		
86212211119876	Clear span in ft.	Safe
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	4"	Safe Load, unifo
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ಗೆ ನೆನೆ ನೆನೆ ಬೆಳೆ ಬೆಳೆಯ ಪ್ರತಿಗಳ ಕ್ರಮಿಸಿದ್ದರು. ಬೆಳೆ ಬೆಳೆ ಬೆಳೆ ಬೆಳೆಯ ಬೆಳೆಯ ಪ್ರತಿಗಳ	6"	at both
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2010 2010 2010 2010 2010 2010 2010 2010	8"	13.
128011778181848888888888888888888888888888	9"	for White Tons of 2
ET ext. 1-0044000000000000000000000000000000000	10"	or White Pine B. Tons of 2000 lbs
######################################	11"	Beams, sup- lbs.
22222222222222222222222222222222222222	12"	sup-

iii ft.         3"         4"         5"         6"         7"         8"         9"         100           6         4.4         5.4         6.7         8.1         10.9         12.1         13.4           7         4.4         5.4         6.7         8.1         10.7         12.1         13.4           8         3.5         4.6         5.8         6.7         8.1         9.2         10.4         11.8           9         3.1         4.1         5.1         5.2         6.2         10.4         11.8         11.8         11.8         12.2         9.2         10.4         11.8         11.8         11.8         11.8         12.2         9.2         10.4         11.8	Safe Clear span	Loc	Safe Load, unifo		1.00	Str	ted, s, in 13"	EEL	of 200		te Pine Bu 2000 ibs.	ine Beams, O ibs.
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1.5 2.2 2.5 3. 3.4 3.9 4.4 1.5 1.5 2.2 2.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3	5	1.7	2.2	2.8	0.4	3.9	4	O1	5.6		-	6.9
1.2 1.4 2.9 2.5 3.9 1.1 1.4 1.8 2.1 2.5 3.9 1.1 1.5 1.6 1.9 2.2 2.5 2.8 3.9 1. 1.8 1.6 1.9 2.2 2.5 2.9	8	1.5	:2	2.5	ça	3.4	3.9	4.4	4	-	_	5.4
1.1 1.4 1.8 2.1 2.5 2.8 3.0 1.1 1.3 1.6 1.9 2.9 2.6 2.9	3	3	1.7	9.9	2.6	<u>ي</u>	င္ပေ	3.9	4.4			4.00
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1.   1.3   1.6   1.9   2.2   2.6   2.9	12	1.1	1.4	00	2.1	2.5	300	ço i o	3,5		-	3.9
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	Cleur span in ft.	safe
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8662131688888888888888888888888888888888	Clear span in ft.	Safe
01544888888881111111 013164164164	ಚ್ಚ	
&	* .	Load, po
10.4 8.7.7 7.7.7 6.8 6.8 5.1 5.1 5.1 5.1 5.1 5.1 5.1 5.1 5.1 5.1	5"	ported
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3844556 799 1000 1000 1000 1000	BREADTIL	5" 1
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Strength of	White	Pine	Struts	or
	Pilla	ra.		
				_

engt i feet						of 20		
Ä.E	4×5	4×0	4×7	4×8	4×9	$4 \times 10$	4×11	$4 \times 12$
6	4.1	5.0	5.8	6.6	7.4	8.3	9.1	9.9
7	38	4.51	5.3	6.0	6.8	7.5	8.3	9.0
8	3.53	4.2	4.9	5.6	6.3	7.0	7.7	8.4
9	3.2	3.9	4.6	5.2	5.9	6.5	7.2	7.8
10 .	2.9	3.5	4.0	4.6	5.2	5.8	6.3	6.9
11	2.6	3.2	3.7	4.2	4.7	5.3	5.8	6.3
12	2.4	2.9	3.3	3.8	4.3	4.7	5.2	5.7
13	2.0	2.4	2.8	3.2	3.6.	4.0	4.4	4.8
14	1.8	2.1	2.5	2.8	3.2	3.5	3.9	4.3
15	1.4	1.7	1.9	2.2	2.5	2.8	3.0	3.3
16	1.1	1.4	1.6	1.8	2.0	2.3	2.5	2.7
17	1.0	1.2	1.4	1.6	11.8	2.0	2.2	2.4
18	0.9	1.1	1.2	1.4	1.6	1.8	1.9	2.1

Safe Load, uniformly distributed, for White Pine Beams, supported at both ends, in tons of 2000 lbs.

BEAMS 15" DEEP.

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n Ins.	
14×12	ı
9.9	
9.0	
7.8	
6.9	1
6.3	
5.7 4.8	1

ST	EFN		OF V			NE S	TRUTE	OR
Length	Di					Section of 200	ns in 0 lbs.	Ins.
3.5	5×5	5×6	5×7	3×8	5×3	5×10	5×11	5×12
8	5.0	6.0	7.0	8.0	9.0	10.0	11.0	120
9	4.7	5.6	6.6	7.5	8.5	9.4	10.3	11.3
10	4.4	5.3	6.2	7.0	7.9	8.8	9.7	10.6
11	4.1	4.9	5.8	6.6	7.4	8.2	9.0	9.8
12	3.5	4.6	5.3	6.1 5.6	6.8 6.4	7.6	8.4	9.1
13	3.2	4.2 3.8	4.5	5.1	K Q	2 4 1	7.7	7.7
iš	2.9	3.5	ii	4.5	5.2	5.8	6.4	7.0
16	26	3.1	3.6	4.2	4.7	5.2	5.7	6.2
17.	2.3	2.8	3.2	3.7	4.1	4.6	3.1	5.5
18	21	2.5	2.9	3.4	3.8	4.2	4.6	5.0
19	1.8	2.2	2.5	2.9	3.2	3.6	1.0	4.8
20	1.5	1.8	2.1	24	2.7	3.0	3.2	3.6
						120		0.0
ngth feet.	Di			of C			ons in	===
Length in feet.		Sa	fe loo	of C	tons	Section 200	ons in	Ins.
I Length	6×5 5.9	5at 6×6 7.1	6 x 7 8.3	of C d in 6×8	6×9 10.7	Section 200 6×10 11.8	ons in 0 lbs.	Ins.
11	6×5 5.9 5.6	5a 6×6 7.1 6.7	6×7 8.3 7.8	of C d in 6×8 9.5 8.9	6×9 10.7 10.0	Section 2000 6×10 11.8 11.2	ons in 0 lbs. 6×11 13.0 12.3	Ins. 6×12 14.2 13.4
11 12	6×5 5.9 5.6 5.3	5a 6×6 7.1 6.7 6.3	6×7 8.3 7.8 7.4	of C d in 6×8 9.5 8.9 8.4	6×9 10.7 10.0 9.5	Section 200 6×10 11.8 11.2 10,5	ons in 0 lbs. 6×11 13.0 12.3 11.5	Ins. 6×12 14.2 13.4 12.6
10 11 12 13	6×5 5.9 5.6 5.3 5.0	5a 6×6 7.1 6.7 6.3 5.9	6×7 8.3 7.8 7.4 6.9	of C d in 6×8 9.5 8.9 8.4 7.9	6×9 10.7 10.0 9.5 8.8	Section 200 6×10 11.8 11.2 10,5 9.8	0 lbs. 6×11 13.0 12.3 11.5	Ins. 6×12 14.2 13.4 12.6 11.8
11 12 13 14	5.9 5.6 5.3 5.0 4.7	5a 7.1 6.7 6.3 5.9 5.6	6×7 8.3 7.8 7.4 6.9 6.5	of Cod in 6×8 9.5 8.9 8.4 7.9 7.5	6×9 10.7 10.0 9.5 8.8 8.4	Section 200 6×10 11.8 11.2 10.5 9.8 9.3	0 lbs. 6×11 13.0 12.3 11.5 10.8	Inc. 6×12 14.2 13.4 12.6 11.8 11.2
10 11 12 13 14 15	6×5 5.9 5.6 5.3 5.0 4.7 4.4	5a 6×6 7.1 6.7 6.3 5.9 5.6 5.3	6×7 8.3 7.8 7.4 6.9 6.5 6.2	of C d in 6×8 9.5 8.9 8.4 7.9 7.5	6×9 10.7 10.0 9.5 8.8 8.4 7.9	Section 200 11.8 11.2 10,5 9.8 9.3 8.8	0 lbs. 6×11 13.0 12.3 11.5 10.8 10.3	Ins. 6×12 14.2 13.4 12.6 11.8 11.2 10.6
11 12 13 14 15	5.9 5.6 5.3 5.0 4.7 4.4 4.1	5a 7.1 6.7 6.3 5.9 5.6 5.3 4.9	6×7 8.3 7.8 7.4 6.9 6.5 6.2 5.7	of C d in 6×8 9.5 8.9 8.4 7.9 7.5 7.1 6.5	6x9 10.7 10.0 9.5 8.8 8.4 7.9	-Section 200   6×10   11.8   11.2   10,5   9.8   9.3   8.8   8.2	0 lbs. 6×11 13.0 12.3 11.5 10.8 10.3	1ns. 6×12 14.2 13.4 12.6 11.8 11.2 10.6 9.8
10 11 12 13 14 15 16 17 18	5.9 5.6 5.3 5.0 4.7 4.4 4.1 3.8 3.5	54 6.7 6.3 5.9 5.6 5.3 4.9 4.5 4.2	6 los 6 x 7 8 3 7 . 8 7 . 4 6 . 9 6 . 5 6 . 2 5 . 7 5 . 3 4 . 9	of Cd in   6×8   9.5   8.9   7.5   7.1   6.3   5.6	6×9 10.7 10.0 9.5 8.8 8.4 7.9 7.3 6.8	Section 200 11.8 11.2 10.5 9.8 9.3 8.8 8.2 7.5 7.0	0 lbs. 6×11 13.0 12.3 11.5 10.8 10.3 9.7 9.0 8.3	Ins. 6×12 14.2 13.4 12.6 11.8 11.2 10.6 9.8 9.0 6.4
11 12 13 14 15 16 17 18	5.9 5.6 5.3 5.0 4.7 4.4 4.1 3.8 3.5 3.2	5a 6x6 7.1 6.7 6.3 5.9 5.6 5.3 4.9 4.5 4.2 3.8	6 los 6 x 7 8 .3 7 .8 7 .4 6 .9 6 .5 6 .2 5 .7 5 .3 4 .9 4 .4	6×8 9.5 8.9 8.4 7.9 7.5 7.1 6.3 5.9 5.6	6x9 10.7 10.0 9.5 8.8 7.9 7.3 6.8 5.7	Section 200  6×10  11.8  11.2  10.5  9.8  9.3  8.8  8.2  7.5  7.0  6.4	0 lbs. 6×11 13.0 12.3 11.5 10.8 10.3 9.7 9.0 8.3 7.7	11.2 11.2 11.2 11.8 11.2 10.6 9.8 9.0 8.4 7.6
11 12 13 14 15 16 17 18 19 20	5.9 5.6 5.3 5.0 4.7 4.4 4.1 3.8 3.5 3.2	Sai 6×6 7.1 6.7 6.3 5.9 5.6 5.3 4.9 4.5 4.2 3.8 3.5	6 loss 6	6×8 9.5 8.9 8.4 7.9 7.3 7.1 6.3 5.9 5.1 4.7	6x9 10.7 10.0 9.5 8.8 8.4 7.9 7.3 6.8 6.3 5.7	Section 200  6×10  11.8  11.2  10.5  9.8  9.3  8.8  2.7  7.0  6.4  8.8	0 lbs. 6×11 13.0 12.3 11.5 10.8 10.3 9.7 9.0 8.3 7.7 7.0	1ns. 6×12 14.2 13.4 12.6 11.8 11.2 10.6 9.8 9.0 8.4 7.6
11 12 13 14 15 16 17 18	5.9 5.6 5.3 5.0 4.7 4.4 4.1 3.8 3.5 3.2	5a 6x6 7.1 6.7 6.3 5.9 5.6 5.3 4.9 4.5 4.2 3.8	6 los 6 x 7 8 .3 7 .8 7 .4 6 .9 6 .5 6 .2 5 .7 5 .3 4 .9 4 .4	6×8 9.5 8.9 8.4 7.9 7.3 7.1 6.3 5.9 5.1 4.7	6×9 10.7 10.0 9.5 8.8 8.4 7.9 7.3 6.8 6.3 5.7 5.2 4.7	Section 200 6×10 11.8 11.2 10.5 9.8 9.3 8.8 8.2 7.5 7.9 6.4 8.5 5.2	0 lbs. 6×11 13.0 12.3 11.5 10.8 10.3 9.7 9.0 8.3 7.7	Ins. 6×12 14.2 13.4 12.6 11.8 11.2 10.6 9.6 9.6 8.4 7.6

ЗТІ	RENC		OP V			NR SI		OR
Length in feet.	Di					Section 1900		Ins.
<b>₩</b> .5	7×5	7×6	7×7	7×8	7×9	7×10	$7 \times 11$	7×12
10	7.7	9.2	10.8	12.3	13.9	15.4	16.9	18.4
11	7.2	8.6	10.1	11.5	13.0	14.4	15.8	17.2
12	0.8	8.0	9.5	10.9	12.2	13.6	15.0	16.0
13	6.5	7.7	9.0	10.3		13.0	14.2	15.4
14 15	6.2 5.9	7.4	8.6			12.4	13.5	14.8
16	5.6	6.7	7.8	9.4 8.9	10.5 10.0	11.8	12.5	14.0
17	5.3	6.4	7.4	8.5	9.5	10.6		12.8
18	5.0	6.0	7.0			10.0	11.0	12.0
19	4.7	.6	6.6			9.4	10.3	11.2
20	4.4	3.2	6.1	7.0			9.6	10.4
21	4.1	4.9	5.7	6.5				9.8
22	3.8	4.6	5.3	6.1	0.8	7.6	8.4	9.2
유나	Di					Section		Ins.
fee		Sai	e loa	d in	tons o	of 200	O lbs.	
Length in feet.	8×5	8×6	8×7	-	8×9	8×10	8×11	8×12
: 10	9.6	11.5		15.3	17.2	19.2	21.0	23.0
11	9.0	10.8	12.6	14.4	16.2	18.0	19.8	21.6
12	8.5	10.2	11.9		15.3	17.0	18.7	20.4
13	8.1	9.7		12.9	14.5	16.2	17.7	19.4
14	7.7	9.2		12.3	13.9 13.1	15.4	16.9	18.4
15 16	7.8 7.0	8.8 8.4		$\frac{11.7}{11.2}$	12.6	14,6 14.0	16.1 15.4	17.6 16.8
17	6.7	8.0	9.4	10.7	12.1	13.4	14.7	16.0
18	6.4	7.7	9.0	10.2	11.5	12.8	14.1	15.4
19	6.1	7.4		9.7	10.9	12.2	13.3	
20	5.8	7.0	8.1	9.3	10.4	11.6	12.8	14.0
21	5.5	6.6	7.7	8.8	9.9	11.0	12.1	13.2
22	5.3	6.3	7.4	8.4	9.5	10.6	11.6	12.6

					ntinue		
Length in feet.		Safe	load	In tor	s of 20		
3.5	9×5	9×6 <sub>1</sub> 9	×7 9:	K8 9X	9  9×1	$0 9\times11$	0×12
10					.5 23.		
11	11.0				.7 22.		
12				3.4 18	.5 /20.	6 22.7	
13	9.8		3.7 1		.5 49.	6 21.5	23.4
14					0.7 18. 0 17.	6 20.8 8 19.6	
16	8.5				.3 17		
17	8.2				7 16		
18	7.9						
19	7.6			2.0 13	3.7 15	2 16,7	
20	7.3			1.6 13	3.11 14.	6 16.0	
21	7.0				.6 14.		
22	6.8	8.1	9.5 1	0.8 12	2.11 13.	.6  14.9	18.2
Length in feet.	Dir				s-Sect	iotis in	Inc.
84							10. 11.
<del>1</del> .=	-		_	-	$10 \times 10$		10×12
	17.5	20.4	23.4	26.3	29.2	32.1	35.0
10		18.9	21.6	24.3	27.0	29.7	32.4
111	16.2				25.1	27.6	30.2
11	15.1	17.6	20.0	22.6			
11 12 13	15.1 14.2	16.6	19,0	21.3	23.7	26,1	28.4
11 12 13 14	15.1 14.2 13.5	16.6 15.8	19.0 18.0	21.3 20.3	23.7 22.5	26,1 24,8	28.4 27.0
11 12 13 14 15	15.1 14.2 13.5 12.9	16.6 15.8 15.1	19.0 18.0 17.2	21.3 20.3 19.4	23.7 22.5 21.5	26,1 24,8 23,7	28.4 27.0 25.8
11 12 13 14 15 16	15.1 14.2 13.5	16.6 15.8	19.0 18.0	21.3 20.3 19.4 18.5	23.7 22.5	26,1 24,8 23,7 22,6	28.4 27.0 25.8
11 12 13 14 15 16	15.1 14.2 13.5 12.9 12.3 11.8 11.3	16.6 15.8 15.1 14.4	19.0 18.0 17.2 16.4	21.3 20.3 19.4	23.7 22.5 21.5 20.5	26,1 24,8 23,7	28.4 27.0 25.8 24.6
11 12 13 14 15 16 17 18	15.1 14.2 13.5 12.9 12.3 11.8 11.3 10.9	16.6 15.8 15.1 14.4 13.7 13.1 12.7	19.0 18.0 17.2 16.4 15.6 15.2 14.6	21.3 20.3 19.4 18.5 17.6 17.0 16.4	28.7 22.5 21.5 20.5 10.6 18.9 18.2	26,1 24,8 25,7 22,6 21,6 20,8 20,0	28.4 27.0 25.8 24.6 23.6 22.6 21.8
11 12 13 14 15 16	15.1 14.2 13.5 12.9 12.3 11.8 11.3	16.6 15.8 15.1 14.4 13.7 13.1	19.0 18.0 17.2 16.4 15.6 15.2	21.3 20.3 19.4 18.5 17.6 17.0	28.7 22.5 21.5 20.5 10.6 18.9	26,1 24,8 25,7 22,6 21,6 20,8	28.4 27.0 25.8 24.6 23.6 22.6

STI	RENG		F WH		PINE !		вов
Length in feet.	Din				s-Sect is of 20		
3.5	11×6	11×7	11×8	11×9	11×10	lixii	11×12
12	18.0	21.0	24.0	27.0	30.0	83.0	36.0
13	16.9	19.7	22.6	25.4	98.2	31.0	33,8
14	16.0	187	91.0	24.0 23.0	26.8 25.6	20.4 28.1	32.0
15 16	15.4	17.9 17.2	20.4 19.6	23.0	24.6	26.9	29.4
17	14.2	18.5	18.8	21.2	23.6	25.9	28.4
18	13.5	15.8	18.0	20.3	22.6	24.9	27.0
19	13.0	15.2	17.4	19.5	21.8	23.9	26.0
20	12.5	14.6	16.8	18.8	21.0	23.0	25.0
21	12.0	14.0	16.0	18.0	20.0	22.0	24.0
22	11.6	13.5	15.4	17.4	.19.4	21.2	23.2 .
23	11.2	13.0	14.8	16.7	.18.6	. 20.5	22.4
24	10.8	12.6	14.4	16.2	18.0	19.8	21.0
Length in feet.	Dir	nensi Safe	ons of	Cros	s-Sect	ions ir	Ins.
3.5	12×6	12×7	12×8	12×9	12×10	12×11	12×12
12	21.0	24.5	28.0	31.5	35.0	38.5	42.0
13	19.9	23.2	26.4	29.8	33.2	36.4	39.7
14	18.8	21.0	25.0	28.1	31.4	34.4	37.6
15	17.9	20.9	23.8	26.8	29.8	32.8	85.8
16	17.1	20.0	22.8	25.7	28.6	31.4 30.0	34.2
17 18	16.4	19.1 18.3	21.8	24.6 23.6	27.4 26.2	28.8	31.4
19	15.7	17.6	20.2	22.7	25.2	27.7	30.2
20	14.6	17.0	19.4	21.9	24.4	26.7	29.2
21	14.1	16.5	18.8	21.2	23.6	25.8.	28.2
22	13.6	15.9	18.2	20.5	22.8	25.0	27.2
23	13.1	15.3	17.4	19.6	21.8	24.0	26.2
24	1 12.6	14.7	16.8	18.0	21.0	23.1	25.2

#### TABLE OF BOARD MEASURE.

EXPLANATION.—The length of the board is given, in feet, in the left-hand column; the width is given, in inches, in the upper row of figures; and the contents are given under the width, and opposite the length. Thus, the contents of a board 13 feet long and 7 inches wide will be found under 7, and opposite 13, and is 7 feet 7 inches.

Length,

wa I

gth,							W	IDI	'H, I	n I	NCH)	ES.					
Length, in feet.	6	5	7		8		9		I	)	I	ı	12	I,	3	I	4
1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 19 20 21 22 23 24 25 26 27 28 29	ft. 0 1 1 2 2 3 3 4 4 5 5 5 6 6 7 7 8 8 9 9 10 10 11 11 12 12 13 13 14 14	In. 6 0 6 0 6 0 6 0 6 0 6 0 6 0 6 0 6 0 6	ft. 0 1 1 2 2 3 4 4 4 5 5 5 6 7 7 8 8 8 9 9 10 11 11 12 12 13 14 14 15 16 16	in. 7 2 9 4 11 6 1 8 3 10 5 0 7 2 9 4 11 6 1 8 3 10 5 0 7 2 9 4 11	ft. 0 1 2 2 3 4 4 4 5 5 6 6 7 8 8 8 9 10 11 12 12 13 14 14 15 16 16 17 18 18 19	in. 8 4 0 8 4 0 8 4 0 8 4 0 8 8 4 0 8 8 4 0 8 8 4 0 8 8 4 0 8 8 4 0 8 8 4 0 8 8 4 0 8 8 4 0 8 8 4 0 8 8 4 0 8 8 4 0 8 8 4 0 8 8 4 0 8 8 4 0 8 8 8 8	ft. 0 1 2 3 3 4 5 5 6 6 7 8 9 9 10 11 12 12 13 14 15 16 17 18 18 19 20 21 21	in. 96 30 96	ft. 0 1 2 3 4 4 5 5 6 6 7 8 9 10 11 12 13 14 15 15 16 17 18 19 20 21 22 23 24	in. 10 8 6 4 2 0 10 8 6 4 2 0 10 8 6 4 2 0 10 8 6 4 2 0 10 8 6 4 2 0 10 8 6 6 4 2 0 10 8 6 6 4 2 0 10 8 6 6 4 2 0 10 8 6 6 6 4 2 0 10 8 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	ft. 0 1 2 3 4 5 6 7 8 9 10 11 11 12 13 14 15 16 17 18 19 20 21 22 22 24 25 26	in. 11 10 9 8 7 6 5 4 3 2 1 0 11 10 9 8 7 6 5 4 3 2 1 0 11 10 9 8 7	feet.  1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29	ft. 1 2 3 4 5 6 7 8 9 10 11 13 14 15 16 17 18 19 20 21 22 23 24 26 27 28 29 30 31	in. 1 2 3 4 5 6 7 8 9 10 1 2 3 4 5 6 7 8 9 10 1 1 2 3 4 5 6 6	ft. 1 2 3 4 5 7 8 9 10 11 12 14 15 16 17 18 19 21 22 23 24 25 26 28 29 30 31 32 33	in. 2 4 6 8 10 0 2 4 6 6 8 10 0 2 4 6 6 8 10 0 2 4 6 6 8 10 0 0 2 4 6 6 8 10 0 0 2 1 10 10 10 10 10 10 10 10 10 10 10 10 1
30	I 5 I 5	6	17 18	6 1	20 20	o 8	22 23	6	25 25	0	27 28	6 5	30 31	32 33	7	35 36	2

#### TABLE OF BOARD MEASURE—(Continued.)

WIDTH, IN INCHES.

the left-; and the contents

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33 35 36

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posite 13,	್ಯಾತ	-			— ī		1												
osite 13,	Lengt in fee	1	5	10	5	I	7	18	3	19		20	o	2	ı	2	2	2	3
ft. in.  1 2 2 4 3 6 4 8 5 10 7 0 8 2 9 4 10 6 11 8 12 10 14 0 15 2 16 4 17 6 18 8 19 10 21 0 21 0 22 2 23 4 24 6 25 8 26 10 28 0 29 2 30 4 31 6	17 18 19 20 21 22 23 24 25 26 27 28 29 30	ft. I 2 3 5 6 7 8 10 11 12 13 15 16 17 18 20 21 22 23 25 26 27 28 30 31 32 33 33 33 33 33 33 33 33 33 33 33 33	n. 36 90 36 90 36 90 36 90 36 90 36 90 36 90 36 90	ft. I 2 4 5 6 8 9 10 12 13 14 16 17 18 20 22 24 25 26 28 29 30 32 33 34 36 37 38 40 41	in. 48 0 48 0 48 0 48 0 48 0 48 0 48 0 48	ft. I 2 4 5 7 8 9 11 12 14 15 17 18 19 21 22 24 25 36 38 39 41 42 43	in. 5 10 3 8 1 6 11 4 9 2 7 0 5 10 3 8 1 6 11 4 9 2 7 0 5 10 3 8 1 6 11	ft. I 3 4 6 7 9 10 12 13 15 16 18 19 21 22 24 25 27 28 30 31 33 34 36 37 40 42 43 45 46	in. 6 0 6 0 6 0 6 0 6 0 6 0 6 0 6 0 6 0 6	ft. 1 3 4 6 7 9 11 12 14 15 17 19 20 22 23 33 34 36 38 39 41 42 44 45 7 49	in. 7 2 9 4 11 6 1 8 3 10 5 0 7 2 9 4 11 6 1 8 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1	ft. I 3 5 6 8 10 11 13 15 16 18 20 21 23 25 26 28 30 31 33 35 36 38 40 44 44 44 45 46 49 50 51	in. 8 4 0 8 8 4 0 8 8 0 8 8 0 8 0	ft. 1 3 5 7 8 10 12 14 15 17 19 21 22 24 26 28 29 31 33 35 36 38 40 42 43 45 50 52 54	in. 96 30 96 30 96 30 96 30 96 30 96 3	it 1 3 5 7 9 11 12 14 16 18 20 22 23 33 34 36 38 40 44 45 47 49 51 53 55 56	In. IO 8 6 4 2 0 IO 8 6 4 2 0 IO 8 6 4 2 0 IO 8 6 6 4 2 0 IO 8 6 6 4 2 0 IO 8 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	ft. I 3 5 7 9 11 13 15 17 19 21 23 24 26 28 30 32 24 44 46 47 49 51 55 57 59	in. 111 100 98 8 76 6 54 4 33 22 1 1 100 98 8 76 6 55 4 4 3 3 2 1 100 98 8 76 6 5 5 6 6 5 6 6 5 6 6 6 6 6 6 6 6 6

FLOORS FOR DANCING must be springy and elastic; joists may be 18 inches apart or even more; the boards should be in narrow widths, of pine, well beeswaxed, and should follow as much as possible the round of the room. Allow I cwt. per foot superficial as safe load.

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### RULE FOR COMPUTING THE NUMBER OF SLATES IN A SQUARE.

Subtract three inches, or the amount of head-cover, from the length of the slate, multiply the remainder by the width, and divide by two. This will give the number of square inches covered per slate; divide 14,400 (the number of square inches in a square) by the number so found, and the result will be the number of slates required.

The following table gives the number of slates per square for the usual sizes,

allowing three inches for head-cover :-

#### NUMBER OF SLATES PER SOUARE.

Size, in inches.	Pieces per square.	Size, in inches.	Pieces per square.	Size, in inches.	Pieces per square.
6 x 12	533	8 x 16	277	12 X 20	141
7 x 12	457	9 x 16	246	14 x 20	121
8 x 12	400	10 x 16	221	11 X 22	1 37
9 x 12	355	9 x 18	213	12 x 22	126
7 x 14	374	10 x 18	192	14 x 22	108
8 x 14	327	12 x 18	160	12 x 24	118
9 x 14	291	10 x 20	169	14 x 24	94 86
10 x 14	261	II X 20	154	16 x 24	86

The weight of slate per cubic foot is about 174 pounds, or, per square foct of various thicknesses, as follows:—

The weight of slating laid per square foot of surface covered will, of course, depend on the size used. The weight of 10 by 18 slate, three-sixteenths of an inch thick, for example, per square foot of roof, would be 5.86 pounds.

An experienced roofer will lay, on an average, two squares of slate in ten hours. Ordinary roofing-paper weighs about fifteen pounds per square, and averages about fifty pounds in a roll.

At the present time the additional cost of laying slate in elastic cement varies from thirteen to fifteen per cent.

#### SHINGLES.

The average width of a shingle is four inches: hence, when shingles are laid four inches to the weather, each shingle averages sixteen square inches, and 900 are required for a square of roofing.

If 4½ inches to the weather, 800 will cover a square.

5	66	6*	66	720	"	66
51/2	44	66	66	655	66	44
51/2	44	44	46	720 655 600	66	66

This is for common gable-roofs. In hip-roofs, where the shingles are cut more or less to fit the roof, add five per cent. to above figures.

A carpenter will carry up and lay on the roof from fifteen hundred to two thousand shingles per day, or two squares to two squares and a half of plain gable-roofing.

One thousand shingles laid four inches to the weather will require five pounds of shingle-nails to fasten them on. Six pounds of fourpenny nails will lay one thousand split pine shingles.

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#### CRUSHING AND TENSILE STRENGTH, IN LBS., PER SQ. INCH OF NATURAL AND ARTIFICIAL STONES.

DESCRIPTION.	Weight per Cubic ft., in lbs.	Crushing Force. Lbs. per sq. inch.
Aberdeen Blue Granite	164	8,400 to 10,914
Quincy Granite	166	15,300
Freestone, Belleville		3,522
Freestone, Caen		1,088
Freestone, Connecticut		3,319
Sandstone, Acquia Creek, used for Capitol		
Washington		5,340
Limestone, Magnesian, Grafton, Ill		17,000
Marble, Hastings, N. Y		18,941
Marble, Italian		12,624
Marbie, Stockbridge, City Hall, N. Y		10,382
Marble, Statuary		3,216
Marble, Veined		9,681
Slate		9,300
Brick, Red	135.5	808
Brick, Pale Red	130.3	562
Brick, Common		800 to 4,000
Brick, Machine Pressed		6,222 to 14,216
Brick, Stock		2,177
Brick-work, set in Cement, bricks not very		
hard		521
Brick, Masonry, Common		500 to 800
Cement, Portland		1,000 to 8,300
Cement, Portland, Cement 1, Sand 1		1,280
Cement, Roman		342
Mortar		120 to 240
Crown Glass		31,000
		TENSION.
Portland Cement		427 to 711
Portland Cement, with Sand		92 to 284
Glass, Plate		9,420
Mortar		50
Plaster of Paris		72
Slate		11,000

# PROPERTIES OF TIMBER.

DESCRIPTION.	Weight per Cubic Foot in Ibs.	Weight per foot B. M. in lbs., average.	Tensile strengta per sq. in., in lbs.	Crushing strength per sq. inch, in lbs.	Relative strength for cross breaking White Pine=100.	Relative strength Shearing strength for cross breaking with the grain. White Pine=100. lbs., per sq. inch.	Pressure in lbs. per sq. inch, to indent 1-20'
Ash	43 to 55.8	4.1	11,000 to 17,207	4,400 to 9,363	120 to 180	458 to 700	t,800 to 1,850
Beech	43 to 53.4	3.9	II,500 to 18,000	5,800 to 9,363	100 to 104		
Cedar	50 to 56.8	4.5	10,300 to 11,400	5,600 to 6,000	55 to 63		
Cherry					130		
Chestnut	33	2.75	10,500	5,350 to 5,600	96 to 123		
Elm	34 to 36.7	2.9	13,400 to 13,489	6,831 to 10,331	96		
Hemlock		•	8,700	5,700	88 to 95		
Hickory			12,800 to 18,000	8,925	150 to 210		0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Locust.	4	3.7	20.500 to 24,800	9,113 to 11,700	132 to 227		0
Maple	49	4.1	10,500 to 10,584	8,150	122 to 220	367 to 647	1,700 to 1,900
Oak, White	45 to 54.5	4.1	10,253 to 19,500	4,684 to 9,509	130 to 177	752 to 966	2,300 to 3,550
Oak, Live	20	8 15		6,850	155 to 189		
Pine, White	30	2.5	000,21 C1 000,01	5,000 to 6,650	100	225 to 423	875 to 1,160
Pine, Yellow.	28.8 to 33	5.6	12,600 to 19,200	5,400 to 9,500	98 to 170	286 to 415	1,900
Spruce			10,000 to 19,500	5,050 to 7,850	86 to 110	253 to 374	875 to 1,025
Walnut, Black	42	3.5	9,286 to 16,000	7,500	0	•	2,200 to 2,600

Th tilled inch

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M Co D G D H K M Q M

# 875 to 1,025 2,200 to 2,600

253 to 374

86 to 110

5,050 to 7,850 7,500

10,000 to 19,500 9,286 to 16,000

42 3.5

#### AVOIRDUPOIS WEIGHT.

The standard avoirdupois pound is the weight of 27.7015 cubic inches of distilled water, weighed in the air, at 39.83 degrees Fahr., barometer at thirty inches.

Ounces		Pounds.		Quarters.		Cwts.		Ton.
1. 16.	****	.0625 1.	=	.00223	==	.000558	=	.000028
448.		28.		I.		.25		.0125
1792.		I 12.		4.		Ι.		.05
358 10.	:	2240.		80.		20.		1.

A drachm = 27.343 grains. A stone = 14 pounds. A quintal = 100 kilogrammes.

7000 grains = 1 avoir. pound = 1.21528 troy pounds. 5760 grains = 1 troy pound = .82285 avoir. pound.

Kilos p. sq. centim. × 14.22 = pounds p. sq. inch. Pounds p. sq. inch × .0703 = Kilos p. sq. centim.

#### FRENCH WEIGHTS.

EQUIVALENT TO AVOIRDUPOIS

	Grains.	Ounces.	Pounds.
Milligramme	.015433	000252	.000022
Centigramme	.154331	.000352	.000220
Decigramme	1.54331	.003527	.002204
Gramme	15.4331	.035275	.022047
Decogramme	154.331	.352758	**
Hectogramme	1543.31	3.52758	.220473
Kilogramme	15433.1	35.2758	2.20473
Myriogramme		352.758	22.0473
Ouintal		3527.58	220.473
Millier or Tonne		35275.8	2204.73

#### AVOIRDUPOIS WEIGHT.

#### (Canadian.)

Squ Squ Ro Squ Ro Ac

> Ya Ra Cl Fi M

> > M

27 1/3 grains	= 1  drachm = 27.34375  grains.
16 drachms	= 1 ounce = 437.5 grains.
16 ounces	= 1 pound = 7000 grains.
25 pounds	= 1 quarter.
4 quarters	= 1 hundredweight.
20 hundredweight	= I ton or 2000 lbs.

#### CIRCLES.

The diameter of a circle is 0.31831 times the circumference.

The circumference is .31416 times the diameter.

The diameter multiplied by 0.8862 equals the side of a square of the same area.

The inside of a square + 1.128 equals the diameter of a circle of the same area.

#### WEIGHT OF VARIOUS LOADS ON ROOFS.

100 FT. SPAN OR LESS.

Covered with corrugated iron:—			
Laid on purlins	= 8	lbs. per	sq. ft.
board			11
Covered with slate:—			
Laid on purlins	=13	11	11
boards			11
Covered with shingles or laths	=10	11	11
	10	11	11
For iron constructionadd	4	11	11
For snow and vertical component of			
wind forceadd	30	11	11

#### SAFE HEIGHT OF PILLARS OF STONE OR BRICK.

No pillar or support of brick or stone should, as a rule, exceed in height 12 times its least thickness at the base. When longer there is a considerable falling off in strength. A height of 24 times the thickness reduces the strength from 10 to 7. When increased to 30 times the strength is reduced one-half, and when increased to 40 times the strength is reduced to one-third.

#### TEST OF WHITE PINE.

The strength of white pine varies widely in different samples. Some careful tests made recently by J. W. Woodman, Building Inspector, Minneapolis, gave the following results, all but two samples being taken from different boards and from different trees. Thirteen pieces 1"×1", 12" between bearings and loaded at centre, broke as follows:

1. 420 lbs.	5. 610	lbs.	9.	295	lbs.
2. 580 11	6. 530	11	10.	540	11
3. 430 11	7. 395		11.	270	t'e
4. 465 11	8. 280	19	12.	420	11
	13. 210	11			

#### SQUARE OR SURFACE MEASURE.

	Inches.	Feet.	Yards.	Poles.	Chains.	Roods.
Square foot	144	1				
Square yard	1,296	9	I			
Rod, pole or perch	39,204	2721/4	301/	1		
Square chain	627,264	4,356	484	16	1	
Rood			1,210	40	$2\frac{1}{2}$	I
Acre	6,272,640		4,840	160	10	4

1 square mile=640 acres=2560 roods=6400 chains

= 102,400 rods (poies or perches)

== 3,097'600 square yards.

1 square acre=209 feet (nearly) or 12¾ rods on each side.

#### CUBIC OR SCLID MEASURE.

1728 C	ubic fee	hes t	t cubic yard.
40	11	of rough timber of hewn timber	t ton or load.
50	11	of hewn timber	
42	11	of timber	shipping ton.
108	11		1 stack of wood.
128	11	4×4×8	1 cord of wood.
40	11	merchandise	I ton shipping.

#### MEASURES OF LENGTH.

	Inches.	Feet.	Yards.	Poles.	Chairs.	Furlongs,
Foot	12	1				
Yard	36	3	I			
Rod, pole or perch	198	3 16½	5 1/2	1		
Chain		66	22	4	I	
Furlong	7,920	660	220	40	10	I
Mile	63,360	5,280		320	89	8
Mile, geographical		6,082.66	Ó			

#### PARTICULAR MEASURES OF LENGTH.

- I Hand=4 inches. I Link=7.92 inches. I Cubit=18 inches.
- 1 Military pace=2 feet 6 inches. 1 Pace geometrical=5 feet.
- I Fathom=6 feet. I Cable's length=120 fathoms.
- I League = 3 miles. I Admiralty knot=6080 feet.
  I Degree=691/8 miles=60 nautical knots or geometrical miles.

#### WEIGHT OF WATER.

I cubic foot of water	=64.425 lbs.
I cubic inch of water	= .03612 lbs.
r gailon	= 10 lbs.
t cwt = 1.8 cubic feet	= 11.2 gallons.
1 ton = 35.9 cubic feet	= 224 gallons.
r cubic foot of sea water	= 64.11  lbs.

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height 12 erable falle strength e-half, and

ne careful polis, gave poards and loaded at

# CONVERTING SUPERFICIAL MEASURE INTO BOARD MEASURE FOR FLOORS.

2 x 2 2 x 4 2 x 5 1.200 2 x 8 1.600 2 x 10 2	0 0.333 0 0.667 0 1.000 1.333	centres	centres centres	entres	centres	centres	centres	centres	centres	centres
2 x 4 0.80 2 x 5 1.20 2 x 8 1.60 2 x 10 2.00 2 x 12 2.40		0.286	0.250	0.222	0.200	0.182	0.167	0.155	0.143	0.133
2 x 5 2 x 8 1.60 2 x 10 2 x 10 2 x 12 2 x 12 2 x 12		0.571	0.500	0.444	0.400	0.364	0.333	0.300	0.286	0.267
2 x 8 1.60 2 x 10 2.00 2 x 12 2.40		0.857	0.750	0.667	0.600	0.545	0.500	0.464	0.429	0.400
2 x 10 2.00 2 x 12 2.40		1.143	1.000	0.889	0.800	0.727	0.667	0.618	0.572	0.533
2 x I2 2.40	_	1.429	1.250	I.III	1.000	0.900	0.833	0.773	0.714	0.667
	_	1.714	1.500	1.333	1.200	160.1	1.000	0.928	0.857	0.800
2 x 14 2.80	_	2.000	1.750	1.556	1.400	1.273	1.167	1.082	000.1	0.933
2 x 16 3.20		2.286	2.000	1.778	1.600	1.455	1.333	1.237	1.143	1.067
2 x 18 3.60		2.571	2.250	2.000	1.800	1.636	1.500	1.392	1.286	1.200
2 x 20 4.00		2.857	2.500	2.222	2.000	1.818	1.667	1.546	1.429	I.333
2 x 22 4.40		3.143	2.750	2.444	2.200	2.000	1.883	1.701	1.572	1.467
2 x 24 4.80		3.429	3.000	2.667	2.400	2.182	2.000	1.855	1.714	1.600
3 x 2 0.60	_	0.429	0.375	0.333	0.300	0.273	0.250	0.232	0.214	0.200
3 x 4 I.20		0.857	0.750	0.667	0.600	0.545	0.500	0.464	0.429	0.400
3 x 6 I.80		1.226	1.125	1.000	o 0 0 0 0	0.818	0.750	0.696	0.643	0.600
3 x 8 2.40	_	1.714	1.500	1.333	1.200	1.091	000.1	0.928	0.857	0.800
3 x 10 3.00	_	2.143	1.875	1.667	1.500	1.363	1.250	1.160	1.071	1.000
3 x 12   3.60	_	2.571	2.250	2.000	1.800	1.636	1.500	1.392	1.286	1.200
3 x 14 4.20		3.000	2.625	2.333	2.100	1.999	1.750	1.623	1.500	1.400
3 x 16 4.80	_	3.429	3.000	2.667	2.400	2.182	2.000	1.855	1.714	009.1
3 x 18 5.40		3.857	3.375	3.000	2.700	2.455	2.250	2.087	1.929	1.800
3 x 20 6.00	_	4.286	3.750	3.333	3.000	2.727	2.500	2.319	2.143	2.000
3 x 22 6.60	_	4.714	4.125	3.667	3.300	3.000	2.750	2.551	2.357	2.200
3 x 24 7,200		5.143	4.500	4.000	3.600	3.273	3.000	2.683	2.571	2.400

#### CARD

3.000

3,600

6.000 5.143 4.500

7,200

3 x 24

#### STRENGTH OF SOLID TIMBER AND PLANK FLOORS,

i. e., floors of large beams at 8 ft. centres covered with planks.

(From Kidder. By C. J. H. Woodbury.)

WEIGH	ΓPER SQ	. FT. OF	FLOOR.	DIMENS	SIONS OF	BEAMS.	Thickness
Super- ficial load.	Weight of beam in lbs.	Weight of floor plank.	Total.	Depth in inches.	Breadth in inches.	Span in feet.	of floor plank in inches.
50 {	3.00 4.08 5.33	6.07	59.07 60.15 61.40	12 14 16	6 7 8	20.95 26.16 31.63	2.43
75 {	3.00 4.08 5.33	7.40	85.40 86.48 87.73	12 14 16	6 7 8	17.42 21.82 26.46	2.96
100	3.00 4.08 5.33	8.55	111.55 111.63 113.88	12 14 16	6 7 8	15.25 19.12 23.23	3.42
125 {	3.00 4.08 5.33	9.55	137.55 138.63 139.88	12 14 16	6 7 8	13.73 17.23 20.96	3.82
150 {	3.00 4.08 5.33	10.45	163.45 164.53 165.78	12 14 16	6 7 8	12.59 15.82 19.25	4.18
175 {	3.00 4.08 5.33	11.26	189.26 190.34 191.59	12 14 16	6 7 8	11.71 14.70 17.91	4.51
200 {	3.00 4.08 5.33	12.05	215.05 216.13 217.38	12 14 16	6 7 8	10.98 13.80 16.81	4.82
225 {	3.00 4.08 5.33	12.75	240.75 241.83 243.08	12 14 16	6 7 8	10,38 13.06 15.90	5.11
250 {	3.00 4.08 5.33	3.45	266.45 267.53 268.78	12 14 16	6 7 8	9.86 12.40 15.08	5.38
275 {	3.00 4.08 5.33	3.55	291.55 292.63 293.88	12 14 16	6 7 8	9.43 11.86 14.46	5.62
300 {	3.00 4.08 5.33	}14.72 {	317.72 318.80 320.05	12 14 16	6 7 8	9.03 11.36 13.85	5.89

#### SOLID BUILT BEAMS.

In the construction of "built" beams for wide spans concerning the keys used to prevent sliding of the timbers, Tredgold says, "the breadth of the key should be twice its depth," and the sum of the depths should be equal to once and a third the total depth of the beam." The bolts and keys may with great advantage be placed at an angle of 45 degrees with the axis of the beam, those on the left half sloping one way, those on the right, the reverse. Keys are made in two pieces with a wedge between left projecting when first driven in tight so as to admit of tightening up in the event of shrinkage. When the depth of the beam is restricted by circumstances so that keys cannot be used, the beams should be notched on the sides that touch, the notches or indentations corresponding exactly. The two timbers are then held together by means of straps or bolts, but this method does not admit of wedges by which the beams may be tightened up. Beams of several thicknesses should "break joint."

#### DRY ROT.

To prevent dry rot, good ser soning of the timber before using and good ventilation for it when in place in a building are essential. Charring and coal tar are recommended. To cure dry rot, a solution of corrosive sublimate in water (an ounce to a gallon used hot) or a solution of sulphate of copper (half a lb. to a gallon of water used hot) are good washes. Where dry rot results from want of ventilation no cure will be effective short of supplying the necessary air. The best cure is to substitute new timbers for rotten ones, clear away every particle of fungus from adjoining walls and timbers, afterwards apply some of the washes given for the preservation of timber. Coal tar will effect the same purpose, or a weak solution of vitriolic acid with water will generally stop the rot if it has not gone too far, and pyroligneous acid is recommended to prevent the spread of dry rot. When lincleum or kamptulicon are fastened down to wood floors dry rot is almost inevitable; free ventilation under the boards will prevent it but that is generally impracticable, or when practicable it is difficult to obtain sufficient ventilation.

#### STRENGTH OF TIMBER.

The following figures give the transverse strengths of several woods in common use as compared with cast iron. The test piece in each case being a bar I in square in cross section, one foot long between supports:

Materials.	Breaking weight in pounds.	Weight carried with safety.
Hickory (seasoned)	270	90
White oak	240	90 80
Ash (seasoned)	175	55
Chesnut II	170	54
Yellow pine 11		50
White pine "	135	45
Cast iron	5781	1927

A titie line Thu

Size

2X 2X 2X 2X 2X 2X 2X

> 3× 3× 3× 3× 4× 4×

4: 4: 4: 6: 6: 6: 6: 8:

8: 10: 10: 12: 12:

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#### TABLE OF LUMBER MEASURE.

A table of lumber measure is convenient to have when making bills of quantities. In the table here given the length of the timber is set off in the upper line ranging from 12 to 30 ft., and the size is indicated in the left hand column. Thus a stick 6x6 in. and 26 ft. long contains 78 ft., board measure.

Size in inches	12	14	16	18	20	22	24	26	28	30
2x3	6	7	8	9	10	11	12	13	14	15
2x4	8	9	ΙΙ	12	13	15	16	17	19	20
2x6	12	14	16	18	20	22	24	26	28	30
2x8	16	19	21	24	27	29	32	35	37	40
2xIo	20	23	27	30	33	37	40	43	47	50
2x I 2	24	28	32	36	40	44	48	52	56	60
2x14	28	33	37	42 18	47	51	56	61	65	70
3x4	12	14	16		20	22	24	26	28	30
3x6	18	21	24	27	30	33	36	39	42	45
3x8	24	28	32	36	40	44	48	52	56	60
3×10	30	35	40 48	45	50	55 66	60	65	70	75
3x12	36	42	48	54	60		72	78	84	90
3x14	42	49	56	63	70	77	84	91	98	105
4x4	16	19	21	24	27	<b>2</b> 9	32 48	35	37	40
4x6	24	28	32	36	40	44	48	52	56	60
4x8	32	37	43	48	53	59	64	69	75	80
4x10	40	47	53	60	67	73	80	87	93	100
4x12	48	56	64	72	80	88	96	104	112	120
6x6	36	42	48	54	60	66	72	78	84	90
6x8	48	56	64	72	80	88	96	104	112	120
6x10	60	70	80	90	100	110	120	130	140	150
6x12	72	84	96	108	120	132	144	156	168	180
8x8	64	75	85	96	107	117	128	139	149	160
8x10	80	93	107	120	133	147	165	173	187	200
8x13	96	112	128	144	160	176	192	208	224	240
OIXOI	100	117	133	150	167	183	200	217	233	250
IOXI2	120	140	160	180	200	220	240	260	280	300
12X12	144	168	192	216	240	264	288	312	336	360
12x14	168	196	224	252	280	308	336	364	392	420
14x14	196	229	261	294	327	359	392	425	457	490

#### STRENGTH OF MATERIALS.

Beams decrease in strength much faster than the length is increased; for instance, if a beam of any given size 20 ft. long will sustain a load of 100 lbs. per foot, a beam of the same size 40 ft. long will only sustain 25 lbs. per foot, and that with much more deflection, while the same beam cut down to 10 ft. long would carry 400 lbs. to each foot in length.

With posts the ratio of strengths to their lengths differs somewhat with different proportions, but roughly speaking, posts of sizes in common use diminish in strength as they increase in length in a ratio of about 1/3 to 2, that is, if a post of a given size and 10 ft. long is capable of supporting 12 tons, one of the same

material and size but 20 ft. long will support but 4 tons.

The comparative strength of rods sustaining loads by suspension is not materially affected by their length. A few examples are given as a basis of calculations.

A mill has to be constructed 50 ft. wide of three stories each 10 ft. high, centre posts and beams 8 ft. between centres, making beams 25 feet long from posts to walls. To carry safely the ordinary load of 200 lbs. per square foot the beam should be of Southern pine 12x16 with posts in lowest stories, round, 11 inches diameter, or square, 10 inches diameter; second story posts 9x9 or 10 inches diameter, and in the third story 8x8 or 9 inches diameter to carry roof. A 15 inch I beam 150 lbs. per yard 25 ft. long would be about the same strength as a 12x16 beam.

A 6-inch round wrought iron column of %-inch shell or a 6-inch cast iron column free from flaws, with ½-inch shell would be the same strength as the loxio posts, cast iron being stronger in columns than wrought iron, except where they are very slender.

Iron will not resist heat so long as wood, wrought iron becoming soft and

pliable and cast iron cracking with heat and water.

If a brick pier is to carry a load of 10x10 posts it should be well built, 2 ft. square, or at least 20x24 inches, and it will resist the action of heat longer than any other of the materials mentioned.

A 134 inch rod will safely support by suspension the same amount of floor

surface of a single floor as is carried by one of these posts.

It the mill is to be used as a warehouse, filled with barrels of flour, the weight should be calculated for 400 lbs. to the square foot, and it would require another row of posts between the centre posts and the wall, thus making the beams about 12 feet between the bearings; the beams remaining the same size and the posts increased a little, the wood about 1 inch, the 100 ½ inch.

#### BEAMS AND GIRDERS.

It is often necessary to decide quickly what sort of beam or girder shall be used across any wide span. To ascertain the required strength estimates must be made of the following: the amount of walling, brick or stone, which the beam will have to carry, the beam being able to support twice as much when the load is evenly distributed along its length than when concentrated in the centre. When there is a central pier between windows, the heaviest part will be in the centre and must be allowed for. Allow I cwt. per cubic foot as weight of brick or stone, the share of flooring or roof that the beam will carry either resting on itself or in the wall above. Half the entire weight upon any such floor will be carried as a

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er shall be mates must h the beam the load is re. When the centre ck or stone, itself or in arried as a distributed load upon the beam. Hurst's Handbook gives the following weights per square foot which floors should be calculated to sustain, including the floor itself:

 Ordinary dwelling house floors
 1½ cwt.

 Public buildings, etc
 1½ cwt.

 Warehouses, factories, etc
 ½ cwt. to 4 cwt.

These are high, but they allow for part being moving weights. For the roofing 40 lbs. per square foot may ordinarily be allowed, which includes timbers, covering, etc.

#### WEIGHT OF CROWDS.

Authorities differ to an extraordinary degree as to the weight of crowds, some giving as a correct weight per square foot as much as four times that given by others. The lowest calculation (given by Trautwine and Stoney) of 41 lbs. per square foot has been shown by Trautwine himself to be insufficient. Tredgold gives 120 lbs. per square foot, a result obtained by actual experiment with men packed together as closely as possible on a platform of 20 ft. diameter, conducted by Mr. Nash, the architect of Buckingham Palace. Prof. Kernot obtained a result of 143 lbs. per square foot, while Mr. Stoney found by experimenting with 58 men at one time and 76 at another he obtained a weight of 147.4 lbs. per square foot may be considered the actual weight of a crowd of stationary human beings, but for bridges or any place where the crowd is moving a greater weight must be allowed for in addition to calculations for the resistance to strains caused by movement.

Mr. Kidder (Boston, 1885) states that for dwelling houses it is not necessary to allow for more than 40 lbs. per square foot, and in most cases 80 lbs. per square foot for assemblages of people will be sufficient. He gives the following table of weights in addition to the weight of the floors:

	street bridges for general traffic		per	square foot.
	floors of dwellings		11	11
11	churches, theatres, ball-rooms80 to	120	11	11
11	schools	8o	11	11
11	hay lofts	8o	11	11
11	storage of grain	100	11 -	11
11	warehouses and general merchandise	250	11	11
11	factories100 to	400	11	11

Special calculations should be made for weights of particular loads on floors.

#### USES OF RESIN.

There are many useful purposes to which resin can be applied outside those of general practise. As a non-conductor of heat it is used as a protector of water pipes, particularly in the crossing of bridges, where the pipe is laid in a long box and the whole filled with melted resin. Resin is also used in supporting basement floors in machine shops which may be laid over some dry material, as spent moulding sand, which is carefully levelled off and the planking laid upon temporary supports separating it about 2 inches above the sand. Numerous holes about 2 inches in diam, being bored through these planks, melted resin is forced through them by means of funnels until the whole space is solidly filled and then the upper flooring is laid upon these planks. In case the floor is subjected to shocks sufficient to break the resin it rapidly joins together again in much the same manner as the relegation of ice. Resin is also used to form water proof paper for use in butcher's shops, fish markets, and also for building purposes, and strange to say, this improvement reduces the cost of the paper. All methods of applying resin in solution after the paper is finished adds heavily to its cost, and also renders it very brittle; but if the resin is dissolved in potash and mixed with the pulp in the beating engine and this alkali afterwards treated with alum, it becomes neutralized and washed away, leaving the finely diffused resin throughout the whole mass. It is also used for protecting the coarser manufactured products, such as agricultural implements, against just, by mixing it with a solution of benzine. This is applied as a varnish, and the benzine rapidly dries away leaving a coat which protects the material until it goes to the severe service of actual use.

#### LIGHTNING CONDUCTORS.

The immunity of private houses from injury by lightning, when unfurnished with regular conductors is much marked, and it is said to be due to the fact of the metal work on the roofs being connected with the eaves troughs and down pipes whereby no doubt the electric fluid finds a conductor and so is dispersed in the drain. In planning the roof covering it is as well to bear this in mind and to arrange for a continuation of metal whereby without going to the expense of a regular copper conductor, a conductor may be formed with the material that has to be used.

#### PANTILES.

A curved tile about 13½ inches long, 7 inches wide, ½ inch thick, rather more than half the weight of plain tiling, less secure in rough weather. To find the number of pantiles of the above dimensions to cover a roof, the weathering being 10 inches, multiply the area in superficial feet by 1.80. To find the weight in tons, multiply the area in superficial feet by .00377.

#### PUTTY FOR REPAIRING BROKEN WALLS.

Equal parts of whiting and plaster of paris walls may be coloured immediately after the application of putty. Whiting and size is not a good mixture, as it rises above the surface of the walls and shows patches. Lime must not be used as it destroys colour,

T and utside those protector of is laid in a in supportne dry mahe planking e the sand. ese planks, hole space s. In case ly joins tosin is also ts, and also the cost of is finished esin is disthis alkali vay, leaving ed for promplements, d as a var-

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Nominal Size.	Outside Dia. Standard.	Inside Dia., Standard.	Weight per foot. Lbs.	Threads to in. of Screw.	Inside area.
**************************************	.40 .54 .67 1.05 1.31 1.66 1.90 2.37 2.87 6.50 5.60 5.60 7.62 7.62 9.68	.27 .36 .49 .62 1.04 1.38 1.61 2.06 2.46 3.06 4.02 4.50 6.06 7.02 7.98	24 .42 .56 .85 1.12 1.67 2.25 2.69 3.06 5.77. 7.54 10.72 12.49 14.56 18.77 23.41 28.35 34.07 40.64	27 18 18 14 11 11 11 11 11 11 11 11 11 11 11 11	.0672 .1078 .1078 .3919 .5281 .84957 2.0358 8.3329 4.7529 7.1529 9.8423 12.0924 12.0924 15.9043 19.9504 63.6174 63.6174

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#### FORMULAE FOR CAST AND WROUGHT IRON PILLARS.

These formulæ are intended to apply to pillars with flat ends, of any length and any form of section.

w = breaking load in lbs. per square inch.

 $\frac{w}{6}$  = safe load for cast iron.

w = safe load for wrought iron.

1 = length of pillar in inches.

h = exterior diameter of round, or least exterior dimension in inches if otherwise shaped.

f = a co-efficient of the material in respect to compression.

a = a co-efficient of the material in respect to flexure.

 $f = \begin{cases} 36.000 & \text{for wrought iron.} \\ 80.000 & \text{for cast iron.} \end{cases}$   $a = \begin{cases} \frac{I}{4500} & \text{for wrought iron.} \\ \frac{I}{400} & \text{for cast iron.} \end{cases}$ 

$$\mathbf{W} = \frac{\mathbf{f}}{\mathbf{I} + \mathbf{a} \left(\frac{1}{\mathbf{h}}\right)^2} = \begin{cases} \frac{36.000}{\mathbf{I} + \frac{\mathbf{I}}{4500} \left(\frac{1}{\mathbf{h}}\right)^2} & \text{For wrought iron.} \\ \frac{80.000}{\mathbf{I} + \frac{\mathbf{I}}{400} \left(\frac{1}{\mathbf{h}}\right)^2} & \text{For cast iron.} \end{cases}$$

#### RULES FOR OBTAINING APPROXIMATE WEIGHT OF WROUGHT IRON.

For round bars.—Multiply the square of the diameter in inches by the length in feet and that product by 2.6. The product will be the weight in lbs., nearly.

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For square and flat bars.—Multiply the area of the end of the bar in inches by the length in feet and that by 3.32. The product will be the weight in lbs., nearly.

Wrought iron usually assumed:	Specific gravities:
1 cubic foot = 480 lbs. 1 sq. foot 1 in. thick = 40 lbs. a bar 1 in. sq., 1 ft. long = 3½ lbs. 11 yd. long = 10 lbs.	Cast iron
Shrinkage in castings:	
Pipes $\dots = \frac{1}{8}$ in. in I ft.	Zinc = $/_{16}$ in. in 1 foot.
Girders, beams, etc = $\frac{1}{8}$ in 15 ins.	Lead = 11 11
Thin brass = 1/8 in. in 9 ins.	Copper = $\frac{1}{16}$
Thick brass $\dots = 11$ 10 ins.	$Tin \dots = \frac{1}{4}$

TO REMOVE RUST FROM STEEL.— $\frac{1}{2}$  oz. cyanide of potassium,  $\frac{1}{2}$  oz. castile soap, 1 oz. whiting and water, to make a paste. Brush the rusted parts with this compound.

TO PRESERVE STEEL FROM RUST.—I caoutchouc, 16 turpentine; dissolve with a gentle heat, and add 8 parts boiled oil. Mix by bringing them to a temperature of 212° Fahr., and lay on with a brush.

## TABLE SHOWING WEIGHT SUSTAINED WITH SAFETY BY A COLUMN OF CAST IRON.

Length or height in feet.	8	10	12	14	16	18	20	22	24
Diameter in inches.			\	VEIGH	IT IN	CWTS	).		
21/2	91	77	65	55	47 84	40	34	29	25
3 3 2	145	128	111	97		73	64	56	49
35	214	191	172	156	135	119	106	94	83
4	288	266	242	220	195	178	160	144	130
41/2	379	354	327	301	275	251	229	208	189
5	479	452	427	394	365	337	310	285	262
6	573	550	525	497	469	440	413	386	360
7 8	989	959	924	887	848	808	765	725	686
	1289	1259	1224	1185	1142	1097	1052	1005	959
9	1672	1640	1603	1561	1515	1467	1416	1364	1311
10	2077	2045	2007	1964	1916	1865	1811	1755	1697
11	2520	2490	2450	2410	2358	2305	2248	2189	2127
12	3020	2970	2930	2900	2830	2780	2730	2670	2600

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#### STRENGTH OF STRUCTURAL IRON AND STEEL.

The greatest strength of cast iron is resistance to crushing, hence it is applicable for columns. Its strength as a girder is greater than wrought iron, but its comparatively brittle character makes it inapplicable for this purpose, where it would be subject to jarring. Its most important element is probably its stiffness.

In general cast iron should be used wherever its strength can be made so far in excess of any strain that can be put upon it that there is no necessity of applying calculations to determine the strength. The only exception to this is its

ase in columns supporting a perfectly dead load.

Wrought iron is strongest under tension, not so strong as a girder, and weakest under compression. Its extreme between the strongest and the weakest is not so great as in cast iron, consequently it may be used in any position, but its strength and stiffness under compression are so much less than cast iron that except for special reasons it is not used in compression.

Wrought steel may be said in general to have the same characteristics as wrought iron slightly exaggerated, and is therefore suitable for the same purposes. Its use is recommended in place of wrought iron where extra strength is

required without increase of size.

Cast steel, except those grades for tools, has the same characteristics as cast iron, but is stronger in every way and not so brittle.

#### A PLAN FOR CUTTING IRON RAILS.

An ingenious method is followed in some German steel works to secure rails of exactly the same length. During the process of cutting it often happens that even with the same gauge one rail will be longer than the others owing to the different heat at which they enter the saws. Those which were the hottest when cut are shortest when cold, having contracted more than the others after cutting. In the German mills the workmen look at the heated rail through a dark glass, so tinted that when the metal has cooled to a certain temperature the rails cannot be seen at all. A dark blue or orange yellow glass will make a red hot rail invisible. It may be considered a fact that any two rails looked at through the same pair of glasses will disappear at the same temperature. If every rail is allowed to cool until it is just invisible through a certain pair of glasses all will be of the same temperature and their lengths will be the same.

#### EXAMINING IRON CASTINGS.

Strike the edges with a light hammer. If the blow makes a slight impression the iron is probably of good quality provided it be uniform throughout. If fragments fly off and no sensible indentation is made, the iron is hard and brittle. Air bubbles are a common and dangerous source of weakness. They should be searched for by tapping the casting all over with a hammer. Bubbles or flaws filled in with sand cause a dulness in the sound which leads to their detection. The exterior surface of the metal should be smooth and clear, and edges sharp and perfect. The surface of a fracture should be of a uniform bluish grey color and high metallic lustre.

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#### NOTES CONCERNING THE SPECIFICATIONS OF QUALITY FOR IRON.

The tensile strength of iron is properly determined by ascertaining the load under which permanent set takes place, and the amount of stretch under the proof load, rather than from the ultimate load that causes the fracture of the bar. In other words, the elastic limit rather than the breaking strain should be regarded as the measure of quality in a bar, and working loads should be proportioned with reference to the elastic limit instead of to the so-called ultimate strength.

Tough, sinewy iron is what is required in a tension bar, and although a hard, unyielding iron may show greater ultimate strength under a gradually applied strain, yet it is not suitable for use under tension for the reason that a sudden shock may cause it to snap under a weight that it ought to carry with entire safety.

Good bar iron should be of uniform character and possess a limit of elasticity of not less than 25,000 pounds per square inch. The ultimate resistance of prepared test-bars having a sectional area of about one square inch for a length of 10 inches should be not less than 50,000 pounds per square inch when the test-bars have been prepared from full-sized bars having not more than 4 square inches of sectional area. For each additional square inch of full-sized bar area above 4 square inches a reduction of 500 pounds per square inch may be allowed down to a minimum ultimate resistance of 46,000 pounds. The amount of stretch under the breaking load should be not less than 15 per cent. in 10 inches of the test-bar.

Bars that are to be used in tension should stand, without cracking, a coal bending test to 90 degrees to a cuvature the radius of which is about the thickness of the bar under test, and at least one-third of the lot should stand bending to 180 degrees under the same conditions.

A round bar, one inch in diameter, should bend double, cold, without signs of fracture. A square bar of the same quality may show cracks on the edges under such a test.

Under a breaking pull the reduction of area should be not less than 25 per cent. of the original section.

The shape of a bar has much influence in determining the breaking-strain. The ultimate strength of round bars is, for this reason, considerably greater than that of flat bars, but in either case the elastic limit will be found to occur at about the same point for equally good qualities of iron.

Within the elastic limit the extension of iron may, for all practical purposes, be stated as follows:

Wrought iron, one ten-thousandth of its length per ton per square inch.

Cast iron, one five-thousandth of its length per ton per square inch.

The compression of wrought iron within the limits of elasticity follows the same law, and the amount of shortening under pressule will be in direct proportion to the weight applied. But with cast iron the amount of compression does not follow a constant ratio, the compression per ton becoming greater with the increase of the weight. Thus, a cast iron bar, one square inch in section was compressed one fifty-nine-hundredths of its length by a load of one ton; but under a load of 17 tons, instead of being compressed seventeen fifty-nine-hundredths, it was compressed twenty fifty-nine-hundredths.

THE MODULUS OF ELASTICITY is a term used to designate such a weight as would extend a bar through a space equal to its original length, supposing the elasticity of the bar to be perfect. Or, the modulus of elasticity of any given material in feet is the height in feet of a column of this material, the weight of which would extend a bar of any determinate length through a space equal to this length. Thus, if one ton extends an inch bar of wrought iron one tenthousandth of its length, it is evident that, upon the supposition that the bar is

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veight as sing the sy given veight of equal to one tenbar is perfectly elastic, 10,000 tons would extend it to twice its original length. Hence, on this assumption, 10,000 tons, or 22,400,000 pounds, will be the modulus of elasticity of the wrought iron stated in weight. But an inch bar of wrought iron to weigh 22,400,000 pounds, at 31/3 pounds per foot, would be 6,720,000 feet long, and this would express the modulus of elasticity in feet.

The modulus of elasticity will of course, vary according to the character of the material tested, being much higher in the better than it is in the lower grades of iron, but it forms a very useful and convenient standard of comparison in determining quality.

#### THE WAY PORTLAND CEMENT IS MADE.

Portland cement is an artificial product, chemically proportioned by the proper selection of the material entering into its composition. These whether chalk or clay, as in England, marl or clay as in Germany, or hydraulic limestones, ar in this country, are in every case reduced to the finest powder by either wet os dry grinding, and this powder moistened merely in the dry process, or in the form of paste in the wet process, becomes practically, either by drying in large tanks or by being moulded into bricks, eggs, or other form, a new stone, into which all the elements are brought in close contact and are in perfect chemical proportion. The artificially made new stone, burnt, as it is, at high heat in close kilns, has every element chemically active, and the clinker represents practically, when preperly proportioned chemically, a composition of bi-basic silicate of lime and aluminate of lime. This ground clinker is the Portland cement of commerce, a fixed uniform product, sold under a warranty of its strength and firmness, and depends upon the controllable elements of skillful manufacture for its character and uniformity.

#### PRESERVING STONE.

About a year ago I watched with great interest the operation of the workmen building devoted to the manufacture of candy in this city. The building beau 5" ck, and the men were engaged in holding little charcoal furnaces about a too. Quare against the outside walls. They went over every square foot of this building -- and it was five stories high by about seventy-five feet deep—holding the glowing coals against the brick sides. My curiosity on the subject was somewhat aroused, and upon inquiry I found that it was a new process for preserving brick and making it impervious to the storms of winter. Now I find that the same process is being used on obelisk, and that these little charcoal furnaces are burning the sides of Cleopatra's Needle from the base to the apex. This famous stone, that has stood the storms of centuries in Egypt, was begining to succumb to the curious climate of the United States, and fears were entertained that a few more winters would make serious inroads upon it; but someone suggested burning it with paraffine, and it is now being done. The heat is so intense that it is burned in for a full inch; then when the stone cools again it is as hard as adamant.—New York Letter.

Soapstone incorporated with oil, after the manner of a paint, is said to be superior to any kind of paint as a preservation. Soapstone is to be had in an exceedingly fine powder, mixes readily with prepared oils for paint use, covers well surfaces of iron, stee!, or stone, and is an effectual remedy against rust. It has been known to protect some stonework, such as obelisks, in China for ages past.

#### HINTS FOR PLASTERERS

#### THE MAKING OF MORTAR.

**HE** making of mortar, comprehends the slacking of lime and the mixture of ingredients worked up with it. As we have already seen, both the former process and the nature of the latter differ, according to the nature of the lime to be dealt with. It is, however, an universal rule, in contradiction to the slovenly practice of some builders, that all limes, of what nature soever, should be reduced to a paste before being mixed with the other ingredients. People who have not studied the actions of the hydrates in a scientific and consecutive manner, oppose the introduction of the previous manipulation of the lime on the score of the extra expense, and on the pretence that the lime loses in strength thereby. As to the objection of the expense, that must of course be estimated by the importance of the work. The second objection is to be met by observing that the rich limes require to be for a long time exposed to the air to enable them to take up the carbonic acid gas, and that, therefore, so far from losing, they gain by exposure; and, moreover, the hydraulic limes being very difficult to slack it is necessary that all their particles should be put in contact with the water. If the lime be not previously reduced into the state of a perfect hydrate, it is always exposed to blister, and to disintegrate, in a manner depending upon the comminution of its particles before being employed; for it is evident that if the lime be ground, the more inactive particles are in a more favorable condition for the absorption of the water. The degree of consistence of this paste should vary with the nature of the extraneous matter. It should be stiff whenever it is intended to form a guage for substances whose particles are hard and palpable, and which are capable of preserving sensible distances from one another. It should be more liquid when the substances to be mixed with it are pulverulent, of impalpable and fine grains, presenting an homogeneous appearance, and in which it is impossible to distinguish the separate elements, such as the puozzolanos, &c. To secure a proper state of the hydrate, it is of very great importance, however, not to use too much water in slacking the lime. So much should be used, and only so much, as is necessary to cause the quicklime to fall to powder. It is also equally important not to mix up into the state of paste more lime than is immediately required to be used; for although, upon being reworked, the hydrates, which begin to carbonite, give off the water they had rendered latent, as a were, yet a portion of their force must be lost by their doing so in proportion to the degree of advancement of the process. In France, whenever great care is required in the fabrication of the mortars, the lime is worked up into a paste in a mill, consisting of two vertical stones working in a The lime, after going through this operation, is then mingled with the sand in a pug-mill or by hand, upon a floor. If the dimensions of the con structions should be such as to justify the expense, it should be made a neces

mixture of he former lime to be slovenly uld be reople who tive manme on the strength estimated y observto enable m losing, difficult to with the t hydrate. ding upon nt that if le condihis paste iff whenhard and one anth it are appear-, such as ery great So much ne to fall of paste eing reney had by their France, lime is ng in a ith the ne con

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sary condition that mechanical means be employed, for even with the greatest possible care the mixture by hand is never perfectly effected. The quantities of sand to be used vary, as might be expected, according to the nature of the limes, and also of this sand. Within certain limits, if they do not gair, by the mixture, at least their effect is not sensibly diminished. Thus we find that, for the rich limes, the resistance is rather increased if the sand be in the proportions varying from 50 to 240 per cent of the paste measured in bulk in the state of a firm paste. Beyond that point the resistance decreases. The resistance of hydraulic limes increases, if the sand be mixed in the proportion of 50 to 180 per cent of the paste; from thence it decreases. The much greater proportion of sand the rich limes are able to support, may perhaps account for the partiality of the builders in their favor. If it be required to mix common lime and puozzolanos, the best proportions according to General Treussart, are I of lime in powder to 2 of trass; or 1 of lime to 1 of sand, and 1 of puozzolano or trass. The best hydraulic limes, as we have seen, lose much of their qualities if long exposed to the air; it is therefore advisable to work them only for the time absolutely necessary to ensure, firstly, their perfect reduction to the state of hydrates; and secondly, the intimate mixture of the lime and sand. The rich limes, however, as we have before said, inasmuch as they absorb the carbonic acid gas with difficulty, gain by being exposed for a longer period to the contact of the atmosphere. As far as such a proceeding is consistent with economy, it is advisable then, to protract the operation of their manipulation as much as possible; it is even advisable to work up large quantities of such mortar beforehand, which are subsequently rendered fit for use by a second manipulation. Some of Vicate's experiments show that all limes lose two-fifths of their strength if mixed with too much water. It is then better to wet the material to be used, and to employ a stiff mortar than to follow the course usually adopted by masons and bricklayers of using very fluid soft mortar. The system of grouting is more than questionable in its results; the lime suspended in it is nearly destroyed, the extra quantity of water is but an addition to the difficulties of setting opposed to the mortar already in place. There are conditions of the atmospheric state which affect the goodness of the mortars, about whose actions the best authorities are not decided. For instance those made in summer are always worse than those made in winter. It has been supposed by some that this fact is accounted for by the too rapid desication of the mortar; and Vicate even asserts that they lose four-fifths of their strength if allowed to dry very rapidly. He recommends, in consequence, that the masonry be watered during the summer months, in all constructions of importance, to guard against this danger. Probably the the hydrates are not in a favorable condition to absorb the carbonic acid gas, if they be allowed to dry rapidly; the presence of the water being necessary for the combination of the lime and the carbon. The tradom of the water from carbonic acid gas in solution is also a necessary condition of the successful use of the hydraulic limes. Their success depends, in fact, upon the slow gradual manner in which they take up that gas from the atmosphere, and crystallized about the nuclei offered to their actions. Some engineers prescribe that the water should be deprived of such impurities by boiling, and although the precaution be rather exaggerated, it is certainly of a useful tendency. As the lime reduced into a paste does not fill up the voids of the materials it is mixed with, there is necessarily a very considerable diminution of bulk upon the quantities of the respective substances taken separately. The exact amount of this diminution varies of course with the limes or sands employed; but as a general rule it may be taken as about three-fourths of their collective volumes. To state this in a convenient formula; is a = the bulk of the lime, b = the bulk of the sand; then  $(a+b) \times 0.75 =$  the bulk of the mortar they will produce

position in which a mortar of any description is to be used, also modifies the proportions of sand which it is desirable to mix with it. Under ground, in the water, and in damp positions, less is and should be employed than in the open air, where it is exposed to the changes of the atmosphere. It is often a matter of importance to know the power of resistance of mortars, but, as they differ within a very large range, it is not easy to state it very precisely. The best experiments, however, show that we may safely calculate for all practical purposes upon a resistance of 14 lbs. avordupois per inch superficial, to a force acting in a direction to tear asunder—an effort of longitudinal traction—of 42 lbs. to a crushing force; and of 5¾ lbs. per inch superficial to a force tending to make the particles slide upon one another. It would not be safe to expose new works to greater efforts than those which could be classed under the above heads.

Plastering is always measured by the square yard for all plain work, and by the foot superficial for all cornices of plain members, and by foot lineal for enriched or carved mouldings in cornices.

By plain work is meant straight surfaces (like ordinary walls and ceilings,) without regard to the style or quality of finish put upon the job. Any panneled work, whether on walls or ceilings, run with a mould, would be rated by the foot superficial.

Different methods of valueing plastering find favor in different portions of the country. The following general rules are believed to be equitable and just to

all parties

Rule 1.—Measure on all walls and ceilings the surface actually plastered, without deducting any grounds, or any openings of less extent than seven super-

ficial vards.

Rule 2.—Returns of chimney breasts, pilasters, and all strips of plastering, less than 12 in. ir width, measure as 12 inches wide; and where the plastering is finished down upon the washboard surbase or wainscoting, add 6 inches to height of walls.

Rule 3.—In closets, add one-half to the measurement; or if shelves are put up before plastering, charge double measurement. Raking ceilings and soffits of stairs, add one-half to the measurement. Circular or elliptical work, charge

two prices; domes or groined ceilings, three prices.

Rule 4.—For each 12 feet interior work is done further from the ground than the first 12 feet, add five per cent. For outside work, add one per cent. for each

foot the work is done, above the first 20 feet.

Rule 5.—Round corners measure per foot lineal extra. Arrisses (other than chimney breasts) measure per foot lineal. All joinings of *new* plastering to old measure lineal by 1 ft. in width extra. *This* does not apply to patching or repairing, which should be done at an agreed price.

Any furring or straightening of joist or studding to be charged for by day's time, but the owner or main contractor should be notified of the necessity of

such work before it is done by the plasterer, or at his expense.

## AMOUNT OF MATERIALS REQUIRED FOR PLASTERING 100 SQUARE YARDS (AVERAGE), "TWO COAT" OR "COAT AND SKIM" WORK.

	*
Sand, ordinary work	40 cubic ft
Lime, " with common sand	6 bushels.
Hair, ½ in. wide, nailed ¾ in. apart	1440
Nails, 3rd fine, studding or joist 16 in. apart	10 lbs.
Nails, ard fine, studding or joist 12 in, apart	12 lbs.

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### HINTS FOR PAINTERS AND DECORATORS.

PAINTING is measured by the superficial yard, girting every part of the work that is covered by paint, and allowing an ad 'ition to the actual surface for the difficulty of covering deep quirks or mouldings, carved or enriched surfaces, as in iron railings, and for "cutting in" as in sash and shelving, or where there is a change of color on same work. Allowances are frequently made for distance from the ground, as in cornices, balconies, dormers, etc. Charges are usually made for each coat of paint put on, at a certain price per yard sup. and per coat. Graining and Marbling (imitations of wood and stone,) are classed differently, and rated at different prices from plain work; and so also with Varnishing.

The following Rules of Measurement, which are based upon, and a fair average of those in use in many sections of the country, are suggested as being equitable and just to both employer and employed, and a reliable standard of valuation.

Rule 1.—All surfaces less than 6 inches wide, or girt, to be measured as 6 inches; and over 6 inches and under 12 inches, to be measured as 12 inches; over 12 inches, to be taken sup. nett.

Rule 2.—All openings to be deducted nett, and all jambs and reveals to be measured as per Rule 1.

Rule 3.—Window sash, when there are more than two lights, to be measured square (as if solid.) Two, and single-light sash to be measured as per Rule 1.

Rule 4.—In measuring doors, panneled shutters, and plain pannel work, girt in and out of pannels both ways and add one edge to each side of door or shutter. Measure sash-doors solid. For batten doors, girt over battens for height; and if beaded, add I inch in width for each bead.

Rule 5.—Measure Venetian blinds or shutters as if square plain pannels, and add one hundred per cent. for labor of working in the slats, etc.

Rule 6.—Measure all architraves, casings, jamos, base, cornices, and similar moulded work, by girting every part of the work covered; and in carved or enriched work add per ft. sup. nett for amount of such work. Girt dentil work twice.

Rule 7.—For consoles, modillions, brackets, cantilevers, ornamental iron work, balusters, lattice work, and paling or balustrade fences, girt in each direction, and add one hundred per cent. to prices of plain work.

Rule 8.—All "picked out" work to be valued by the measurer according to amount of labor performed; and all work not specified in preceding Rules to be rated at an average of rates for other work.

Rule 9.—For "knotting," puttying and cleaning off each coat with sand-paper, add five per cent. to prices of plain work (not graining or marbling). For cutting down with pumice-stone and water, add ten per cent. more.

Rule 10.—For all work done above level of ground, if interior work, add five per cent, for each story of 12 feet or less, above first story. For exterior work,

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add one per cent. for each foot of height above the first 12 feet. For exterior

walls, take half the whole height as the average height.

Rule 11.—For change of colors on pannel work, cornices, washboard, etc., add one-fifth for each tint employer. On paling or picket fences, if the tops are painted different colors from the rest of the fence, add six inches to height of fence.

#### ABOUT PAPER HANGING.

"An eight or ten inch bristle smoothing brush, a six to eight inch paste brush, a fourteen to sixteen inch pair of shears, a paper knife, seam roller, plumb-bob, chalk line, paste pail, size kettle, paper boards, trestles and step-ladder, these are the necessary tools for general work, though it is necessary to save time and trouble to have besides these tools a good sized kalsomine brush and a double width putty knife.

The next we need to proceed with is a bucket of paste. Use flour paste, except on very rare occasions when the tints are very delicate. The best patent flour is the most adhesive, and retains its consistency much better than starch.

Take a common patent pail and put in one-half gallon of flour. Stir in enough cold water to mix a flour batter; work out all the lumps thoroughly; have ready three gallons of absolutely boiling water, and stir this until you have enough to cook it. Now pour a little cold water ever the top to prevent skinning over until you are ready to use it. You can thin it down at pleasure.

If the paper put on is not a metallic ground put in four or five ounces of alum in the paste. This will prevent its turning sour and hasten drying. Should the paper be metallic, use a little carbolic acid in place of alum, as alum is liable to

turn gilt dark.

If the room you are going to paper is a hard finish, and not very badly smoked up, all that is necessary to prepare the walls is to brush them with a broom. Should they be smoked and dirty, it is best to go over them with a weak solution of glue and alum. If the walls have been papered before it is necessary to go over them again with a putty knife and cut the old paper off, pull out all nails, and with some plaster of Paris mixed with paste heal all the bad places.

If the walls have been whitewashed, doctor them with a strong solution of vinegar. Having the walls ready, lay a roll of paper on the boards, and with your straight-edge, which should be six feet long, measure the height to where the border will come and about an inch below the baseboards, and cut the strips off. Match the next strip to the top of the previous one and cut enough strips

to cover the room.

To ascertain the number of strips required, take a roll of paper and count the number around the room. Now turn the paper over if you have trimmed it; put the trimmed edges towards you and pull the first over, so that it covers the other strips. This is to prevent the paste from forming on the trimmed edge and making bad work.

Some paper-hangers never trim the paper till after it is pasted. This procedure has some good features and some bad ones, which we will not discuss here. Beginners will find it better to have the paper trimmed beforehand. Commence at one end of the room and hang the slips as nearly perpendicular

as you can.

Always brush the paper from the centre down and at either side. If you have wrinkles in the paper pull it off to where the wrinkle is and brush it out from the centre. Run the shears along the paper at the top of baseboards, and cut off nice and even. Never allow the paper to look haggled or uneven around the base, and cut it close down, but not overlapping.

When you come to an opening let the paper overlap, and trim with the paper

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knize as you work a saw. You will next need short strips, but do not run them beyond the opening unless the piece cut out of the other side will fill up the opposite; rather lap back again to be sure of a perfect match.

When you come to a corner, never lap the paper around it unless the space is very narrow. Fold up the strips at both ends measure the distance with your ruled shears, lay the straight-edge on the paper, mark the distance just a little beyond the corner, and cut. In this manner you will always have corners that

will be square and stay in position.

Use the same roller, and roll the seams nicely as you go. Cut the border in such lengths as can be easily reached to put on, paste and double up at both ends, so that the lines meet, exactly and cut."

#### USEFUL INFORMATION.

A gallon of water (U. S. Standard) weighs 8½ pounds and contains 231 cubic inches.

A cubic foot of water weighs 62½ pounds, and contains 1,728 cubic inches, or 7½ gallons.

Each nominal horse-power of boilers requires one cubic foot of water per hour.

In calculating horse-power of boilers, consider for tubular or flue boilers 15 square feet of heating surface, equivalent to one horse-power.

Condensing engines require 20 to 25 gallons of water to condense the steam

evaporated from one gallon of water.

To find the pressure in pounds per square inch of a column of water, multiply the height of the column in feet by .434. (Approximately, every foot elevation is called equal to one-half pound pressure per square inch.)

To find the capacity of a cylinder in gallons. Multiplying the area in inches by the length in stroke in inches will give the total number of cubic inches; divide this amount by 231 (which is the cubical contents of a gallon in inches), and the product is the capacity in gallons.

Ordinary speed to run pumps is 100 feet of piston per minute.

To find quantity of water elevated in one minute running at 100 feet of piston per minute. Square the diameter of water cylinder in inches and multiply by 4. Example: capacity of a five-inch cylinder is desired: the square of the diameter (5 inches) is 25, which, multiplied by 4, gives 100, which is gallons per minute, (approximately).

To find the diameter of a pump cylinder to move a given quantity of water per minute (100 feet of piston being the speed), divide the number of gallons by 4, then extract the square root, and the result will be the diameter in inches.

To find the velocity in feet per minute necessary to discharge a given volume of water in a given time, multiply the number of cubic feet of water by 144 and

divide the product by the area of the pipe in inches.

To find the area of a required pipe, the volume and velocity of water being given, multiply the number of cubic feet of water by 144, and divide the product by the velocity in feet per minute. The area being found, it is easy to get the diameter of pipe necessary.

The area of the steam piston, multiplied by the steam pressure, gives the total amount of pressure exerted. The area of the water piston, multiplied by the pressure of water per square inch gives the resistance. A margin must be made between the power and resistance, to move the pistons at the required speed; usually reckoned at about 50 per cent.

#### PIGMENTS.

### PIGMENTS AFFECTED BY EXPOSURE TO LIGHT AND THE NORMAL ATMOSPHERE.

RED—Pure scarlet, carmine, crimson lake, scarlet lake, Indian lake, dragons blood.

YELLOW—King's yellow, citron yellow, stronian yellow, yellow lake, Italian pink, gamboge, extract of gamboge, gallstone, Indian yellow.

GREEN—Chrome green, Hooker's green, Prussian green, sap green. BLUE—Prussian blue, Antwerp blue, cyanine blue, indigo, intense blue.

PURPLE—Purple lake, burnt carmine, burnt lake, violet carmine, Indian purple.

BROWN—Bone brown.

CITKINE—Brown pink.

OLIVE—Olive lake, olive green.

GRAY—Neutral tint, Payne's gray.

### PIGMENTS AFFECTED BY AN ATMOSPHERE CONTAINING SULPHURETTED HYDE.OGEN.

WHITE-Flake white, cremnitz, blanc d'argent.

RED-Pure scarlet, red chrome.

ORANGE-Orange chrome.

YELLOW-Deep chrome yellow, pale chrome yellow, Naples yellow.

GREEN—Chrome green, emerald green, malachite green, verdigris.

Blue—Cerulean blue, cobalt blue, smalt, cyanine blue.

PURPLE—Indian purple.

### PIGMENTS WHICH SUFFER CHANGE BY ADMIXTURE WITH WHITE LEAD AND OTHER LEAD COMPOUNDS.

RED—Pure scarlet, carmine, crimson lake, madder carmine, rose madder, scarlet lake, pink madder, madder lake, Indian lake, dragon's blood.

YELLOW—King's yellow, yellow lake, Italian pink, gamboge, extract of gamboge, Indian yellow, gallstone.

GREEN—Sap green.

BLUE—Indigo, intense blue.

PURPLE—Purple lake, burnt carmine, burnt lake, Indian purple, violet carmine.

CITRINE—Brown pink.

OLIVE—Olive lake, olive green.

### PIGMENTS WHICH ARE DECOMPOSED BY ADMIXTURE WITH OCHRES AND OTHER FERRUGINOUS SUBSTANCES.

RED—Pure scarlet, carmine, crimson lake, scarlet lake, madder carmine, rose madder, pink madder, madder lake.

YELLOW-King's yellow.

GREEN-Emerald green, malachite green, verdigris.

BLUE-Indigo, intense blue.

PURPLE—Purple lake, burnt carmine, burnt lake, Indian purple, violet carmine.

CITRINE—Brown pink.

OLIVE—Olive lake, olive green,

#### PERMANENT PIGMENTS.

Pigments which withstand the action of light, of atmospheric oxygen and moisture, of sulphuretted hydrogen, and which may be safely mixed with compounds of iron and lead:

WHITE—Zinc white, Chinese white, permanent white.

RED—The vermilions, Mars red, light red, Venetian red, Indian red, red ochre.

Grange—Cadmium orange, Mars orange, burnt sienna, burnt Roman ochre, neutral orange.

Yellow—Aureolin, cadmium yellows, lemon yellows, Mars yellow, raw sienna, yellow ochre, Roman ochre, transparent gold ochre, brown ochre.

GREEN—Oxide of chromium, transparent oxide of chromium, viridian, terre verte, cobalt green.

BLUE—Genuine ultramarine, artificial ultramarine, new blue, permanent blue.

Purple—Purple madder, Mars violet.

Brown—Brown madder, Ruben's madder, bistre, Prussian brown, burnt umber, Verona brown, Vandyke brown, Caledonian brown, Cappah brown, asphaltum, Cologne earth, mummy, sepia, warm sepia, Roman sepia.

CITRINE—Raw umber, Mars brown. GRAY—Ultramarine ash, mineral gray.

BLACK—Ivory black, lamp black, blue black, cork black, Indian ink, black lead.

[Field's Chromatography.]

### TINTS FOR PAINTING AND DECORATING, WITH THEIR COMPOUNDING PIGMENTS.

The following is taken from *Painting and Decorating*, with the note that some pigments being so much stronger than others, it is impossible to give the exact quantities of each required. The pigments are, however, mentioned in the order of their importance in the mixture:

SHRIMP PINK—White lead, Venetian red and burnt sienna. A little pale English vermilion will enhance its richness.

BUTTERCUP YELLOW—White lead, lemon chrome yellow.

SPRUCE YELLOW—Frence ochre, white lead, with a small touch of Venetian red.

PEACOCK BLUE—Ultramarine blue, extra light chrome green and white lead. COTRINE—White lead, orange chrome yellow and lamp black.

RUSSET—White lead, small quantity of lamp black, orange chrome yellow. SLATE—White lead, raw umber, ultramarine blue and a trifle of lamp black. MYRTLE—Dark chrome green, ultramare, lightened up with a small quantity of white lead.

MASTIC—White lead, French ochre, Venetian red, a trifle of lamp black.

TURQUOISE BLUE—White lead, cobalt blue, Paris green or extra light chrome green.

TAN—White lead, burnt sienna; add a trifle of lamp black.

MAUVE-Yellow ochre, Venetian red, lamp black, a little white lead.

SALMON—White lead, French ochre, burnt sienna, with a touch of English vermilion (pale).

PRIMROSE—White lead, lemon or medium chrome yellow (according to the shade desired).

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ECRU—White lead, French ochre, burnt sienna, lamp black. This tint varies greatly. It means raw. It is intended to show the tint of raw flax or hempen fabrics.

ASHES OF ROSES-Light Tuscan red, lamp black.

QUAKER DRAB—White lead, French ochre, lamp black and burnt sienna.

LEAF BUD-White lead, orange chrome yellow, light chrome green.

DREGS OF WINE—Dark Tuscan red, lamp black, to which add a trifle of white lead.

POMPEHAN RED—Venetian red. If a richer tone is desired, use half and half American vermilion and Venetian red.

BROWN STONE—Orange chrome yellow, dark Tuscan red, lamp black; lighten up to suit with white lead.

LONDON SMOKE—Yellow ochre, ultramarine blue, lamp black; lighten up to suit with white lead.

BISMARCK BROWN—Burnt sienna, burnt umber, orange chrome yellow, lightened up with white lead.

AMBER BROWN—Burnt sienna, orange chrome yellow, burnt umber, lamp black; lighten up to suit with white lead.

SCARLET—Pale English vermilion, or the various scarlet reds, such as the new Idria, etc.

PURPLE BROWN—Dark Indian red, ultramarine blue, lamp black; lighten with white lead to suit.

YELLOW BRONZE—Lemon or medium chrome yellow, French ochre, a trifle of burnt umber.

CRIMSON—Dark English vermilion or the scarlet reds (deep shades); add some carmine, or, better, glaze with it.

EMERALD GREEN—Paris green. A good imitation can be had, answering most purposes, with extra light chrome green.

HAY COLOR—White lead, orange chrome yellow, light chrome green, Tuscan or Indian red.

ANTIQUE BRONZE—Orange chrome yellow, ivory black.

GAZELLE—Dark Tuscan red, Venetian red, lamp black; lighten up with white lead.

APPLE GREEN—White lead, light chrome green, orange chrome yellow.

RUSSIAN GRAY—White lead, ultramarine blue, pale Indian red, lamp black. GOLDEN BROWN—French ochre, orange chrome yellow, lamp black; lighten up to suit with white lead.

GRAY GREEN—White lead, ultramarine blue, lemon chrome yellow, lamp black.

ELECTRIC BLUE—Ultramarine blue, white lead, raw sienna.

[F. Maire.]

SUBSTITUTE FOR PLASTER OF PARIS.—Best whiting 2 lbs., glue 1 lb., linseed oil 1 lb. Heat all together and stir thoroughly. Let compound cool, and then lay it on a stone covered with powdered whiting and heat it well till it becomes of a tough and firm consistence; then put it by for use, covering it with wet cloths to keep it fresh. When wanted for use it must be cut in pieces adapted to the size of the mould, into which it is forced by a screw press. The ornament may be fixed to a wall, picture frame, etc., with glue or white lead. It becomes in time as hard as stone.

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#### HOW TO CLEAN VARIOUS SUBSTANCES, ETC.

Brass or Copper—I oz. oxalic acid, 6 oz. rotten stone,  $\frac{1}{2}$  oz. gum arabic finely powdered, add I oz. sweet oil, and water to make a paste. Apply with flannel or wash leather.

Bronze—To remove fly specks—Lavender oil, I drachm; alcohol, I oz.; water, I 2 oz. Apply with soft sponge but rub as little as possible.

Bronze statuary—Weak soap suds and aqua ammonia.

Brushes, paint—Turpentine—press out all particles of color and hang brushes

in water, not letting them touch the bottom of the vessel.

Engravings—Expose to the fumes of muriatic acid and wash with water. To remove ink spots, apply aqua fortis, diluting application with water as soon as any effect is observed. Dry off with blotting paper and repeat process. Dip in water in which a little potash has been stirred.

Floors—(a) Ink stains may be removed by rinsing with strong vinegar.

(b) Parafin oil, by applying a strong hot solution of oxalic acid, and scrubbing afterwards.

Gilt frames-Soap and water. Boil common size in water sufficient to cover

it, strain through muslin and apply with camel's hair brush.

Glass, paint off—Take as much off with a knife as can be removed without scratching glass. Mix oil turps and pumice stone, and apply; clean off with a rag and soap wash, and polish with cotton tag.

Grease spots, before painting—Wash with saltpeire in solution or very thin lime whitewash; soap suds, it used must be well washed off or paint will not dry.

Grease from stone—Pour strong soda or boiling water over spot; lay over it fuller's earth made into a paste with boiling water; let it remain some hours.

Iron and steel—Saturate a spongy piece of fig wood tree with a mixture of sweet oil and finely powdered emery, and rub.

Ivory or bone.—Brush with a thick paste of common whiting; wash off with water; dry gently near fire and brush again with one drop of alcohol. To remove smoke stains, dip in benzine.

Paint, to remove—4 lbs. Irish moss, 3 lbs. methylated spirits, 30 lbs. water, boiled; add solution of 16 lbs. caustic potash in 28 lbs. water; stir till cold, when it will be a gelatinous mass; apply with brush and allow it to stand for 24 hours; wash off thoroughly.

Varnish brushes—That have dropped, and so got dirty while in use—Clean out well in varnish; fill brush with varnish and place in keeper, dust will gradually sink; by cleaning with turpentine which is volatile dust and dirt are drawn up to the tin of the brush and will work out when brush is used again.

Wall paper, grease from-Lay blotting paper of several thicknesses over spots

and press a hot iron against it.

Whitewash, to remove—A thick paste of wheat flour, with alum added in considerable quantity, applied with whitewash brush; shut door and windows and let it stand over night.

GOLD LACQUER FOR METALS.

1. Shellac       100 parts         Alcohol       895 "         Boric acid       5 "         Picric       enough to colour         2. Shellac       120 parts         Gamboge       30 "         Mastic       30 "         Sandarac       60 "         Aloes       10 "         Venice turpentine       30 "	3. Dragon's blood Gamboge	40 30 30 72 20 20	n n n n n n n n n n n n n n n n n n n
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### Points to Plumbers.

THE following pages on the hydraulics of plumbing are taken directly from the fifth edition (1884) of an excellent work on "House Drainage and Water Service," by James C. Bayles.

Water is practically an incompressible liquid, weighing, at the average temperature of sixty degrees F., about 62.3 pounds to the cubic foot, and 8.3 pounds to the gallon. These figures are subject to slight variations incident to changes

in temperature.

A column of water 12 inches high exerts a downward pressure of about 0.43 of a pound to the square inch. A column two feet high exerts a pressure of about 0.86 of a pound, or just twice that exerted by a column one foot high. This pressure per square inch, due to head, is irrespective of volume, or any thing else except vertical height of column. With these figures in mind, the calculation of the pressure per square inch due to any head is a simple matter. The following rules will be found valuable for reference:—

TO FIND PRESSURE IN POUNDS PER SQUARE INCH EXERTED BY A COLUMN

OF WATER.—Multiply the height of the column, in feet, by 0.43.

To FIND THE HEAD.—Multiply the pressure, in pounds per square inch, by 2.31.

PRESSURE OF WATER.—The weight of water or of other liquids is as the

quantity, but the pressure exerted is as the vertical height.

Fluids press equally in all directions: hence any vessel or conduit containing a fluid sustains a pressure on the bottom equal to as many times the weight of the column of greatest height of that fluid as the area or the vessel is to the sectional area of the column.

LATERAL PRESSURE.—The lateral pressure of a fluid on the sides of the vessel or conduit in which it is contained is equal to the product of the length multiplied by half the square of the depth and by the weight of the fluid in cubic unit of dimensions. The following formula is simple and satisfactory: Multiply the submerged area in inches by the pressure due to one-half the depth. By "submerged area" is meant the surface upon which the water presses; for example, to find the lateral pressure upon the sides of a tank 12 feet long by 12 feet deep: 144 × 144 equals 20736 inches of side. The pressure at the bottom will be 12 × 0.43 equals 5.16 pounds, while the pressure at the top is 0, giving us, say, 2.6 pounds as the average; therefore 20736 × 2.6 equals 53914 pounds.

DISCHARGE OF WATER.—The quantity of water discharged during a given time from a given orifice, under different heads, is nearly as the square roots of the corresponding heights of the water in the reservoir or containing vessel above

the surface of the orifice.

If a cylindrical horizontal tube through which water is discharged be of greater length than its diameter, the discharge is much increased. It can be lengthened with advantage to four times the diameter of the orifice.

TO FIND THE NUMBER OF UNITED-STATES GALLONS CONTAINED IN A FOOT OF PIPE OF ANY DIAMETER.—Square the diameter of the pipe in inches,

and multiply the square by 0.0408.

TO FIND THE NUMBER OF GALLONS DISCHARGED, WHEN THE HEAD LENGTH OF PIPE AND ITS DIAMETER, ARE KNOWN.—Divide the head of water in feet by the given length in yards, and the nearest number thereto in the table under the diameter will be found opposite the required number of gallons.

VELOCITY OF FLOW OF WATER,-Water which has a chance to flow down-

ward does so with a velocity in exact proportion to its head. The following table gives the velocity of flow of water due to heads of from one to forty feet:

Velocity in Feet per Second due to Heads of from 1 to 40 Feet.

Head.	Velocity.	Head.	Velocity.	Head.	Velocity.	Head.	Velocity.
0.5	5 67	10.5	25.98	20.5	36.31	30.5	44.29
1.0	8.02	11.0	26.60	21.0	36.75	31.0	44.65
1.5	9.82	11.5	27.19	21.5	37.18	31.5	45.01
2.0	11.34	120	27.78	22.0	37.61	32.0	45.37
2.5	12.68	12.5	28.35	22.5	38.04	32.5	45.72
3.0	13.89	13.0	28.91	23.0	38.46	33.0	46.07
3.5	15.00	13.5	29.46	23.5	38.88	33 5	46.42
4.0	16.04	14.0	30.00	24.0	39.29	34.0	46.76
4.5	17.01	14.5	30.54	24.5	39.69	34.5	47.10
5.0	17.93	15.0	31.06	25.0	40.10	35.0	47.44
5.5	18.81	15.5	31.57	25.5	40.50	35.5	47.78
6.0	19.64	16.0	32.08	26.0	40.89	36.0	48.12
6.5	20.44	16.5	32.58	26.5	41.28	36.5	48.45
7.0	21.22	17.0	33.06	27.0	41.67	37.0	48.78
7.5	21.96	17.5	33.55	27.5	42.05	37.5	49.11
8.0	22.68	18.0	34.02	28.0	42.44	38.0	49.44
8.5	23.38	18.5	34.49	28.5	42.81	38.5	49.76 .
9.0	24.06	190	34.96	<b>29.0</b>	43.19	39.0	50.08
9. <b>5</b>	24.72	19.5	35.41	29.5	43.56	39.5	50.40
10.0	25.36	20.0	35.86	30.0	43.92	40.0	50.72

In plumbing-work we cannot secure this velocity in the flow of water through pipes, because of the friction which constantly tends to diminish it. The longesthe pipe, the greater the friction and consequent retardation of the flow.

TO FIND THE HEAD OF WATER, WHEN DIAMETER AND LENGTH OF PIPE, AND NUMBER OF GALLONS DISCHARGED PER MINUTE, ARE KNOWN.—In the above table the head due to a length of one yard is found opposite the number of gallons. Multiply that number by the given length in yards, and we have the required head in feet. Thus, to find the head necessary to deliver 130 gallons per minute by a pipe 4 inches in diameter, 500 yards long; opposite 130 gallons in the table, and under 4 inches in diameter, is 0.679, which, multiplied by 500, gives 339.5 feet, the head sought.

TO FIND THE DIAMETER OF THE PIPE, WHEN HEAD, LENGTH OF PIPE, AND THE NUMBER OF GALLONS DISCHARGED PER MINUTE ARE KNOWN.—Divide the head of water in feet by the length of the pipe in yards, and the number nearest to this in the table opposite the number of gallons will be found under the required diameter.

TO FIND THE LENGTH, WHEN THE HEAD, NUMBER OF GALLONS PER MIN-UTE, AND DIAMETER OF PIPE ARE KNOWN.—Divide the given head by the head for one yard, found in the table under the given diameter and opposite the given number of gallons, and the result is the required length.

The discharge of small pipes may be calculated with sufficient accuracy for practical purposes from the following convenient table, showing the quantity of water that will flow through a pipe 500 feet long in 24 hours, with a pressure due to a head of ten feet:

3/8-inch bore				576 gallons.	34-inch bore			3,200 g	allons
1/2-inch		•			-1-inch "			6,624	**
W-inch "		•	•	2,040 "	1 1/4 -inch "	٠	۰	10,000	и

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#### CONTENTS OF EARTHWORK.

In estimating the number of cubic yards in an excavation or embankment, the solid contents of the earth before breaking up is taken or "measured in place," as it is commonly termed. If measurements are to be taken from a loose heap, a deduction must be made according to the nature of the soil. In ordinary soils it is the usual practice to deduct one-third or one-fourth.

SHRINKAGE OF EMBANKMENT.—[From Trautwine's Handbook.]—"Earthwork when first dug and loosely thrown out swells about 1-5 part, so that a cubic yard in place averages about 1 1-5 or 1.2 cubic yards when dug, or 1 cubic yard dug is equal 5-6 of a cubic yard in place. When made into an embankment it gradually subsides and settles or shrinks into a less bulk than it occupied before being dug."

The following are approximate averages of the shrinkage, or in other words, the earth measures in place in a cut will, when made into embankment, occupy a bulk less than before by about the following proportions: Gravel or sand, about 8 per cent., or 1 in 12½ less; clay about 10 per cent., or 1 in 10 less; loam about 12 per cent., or 1 in 8½ less; loose vegetable surface soil, about 15 per cent., or 1 in 6½ less; puddled clay about 25 per cent., or 1 in 4 less.

Trautwine further says, from trials of his own, that one cubic yard of any hard rock in place will make from 13/3 to 13/4 cubic yards of embankment; say, on an average, 1.7 cubic yards, or that one cubic yard of rock embankment requires .5882 of a cubic yard in place. He found that a solid cubic yard when broken into fragments made about as follows:

	OF WHICH THERE WERE				
CUBIC YARDS.  In loose heap	SOLID. 52.6% 57.0% 63.0% 67.0% 80.0%	VOIDS. 47.4% 43.0% 37.0% 33.0% 20.0%			

Excavation is measured by the cubic yard. To ascertain the number of cubic yards of excavation made, take the length and multiply the same by the width and the average height; the result will give the number of cubic feet, which divided by 27. will be the amount in cubic yards. Example:—How many yards of excavation in a cellar 15 feet wide by 18 feet long and 7 feet high? Answer: 15 times 18 times 7 are 1,86c cubic feet; divided by 27 are 70 cubic yards Trenches and pier holes double measurements are usually allowed.

#### TESTS FOR SI-ATE.

A German trade journal advocates the following method for testing the quality of roof slates: The samples of the slate to be tested should be carefully weighed, and then put into boiling water for a quarter of an hour. The water must, however, be fairly free from lime, saltpetre and ammonia. The slates are then reweighed, and those that show the greatest increase of weight are those most capable of resisting deterioration.

Cracks in floors, around the skirting board or other parts of a room, may be neatly and permanently filled by thoroughly soaking newspapers in paste made of one pound of flour, three quarts of water and a tablespoonful of alum, thoroughly boiled and mixed. The mixture will be about as thick as putty, and should be forced into the cracks with a bent knife or other handy tool. When dry it will be harder than the boards.

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