to designers. The lettered dimensions given on Diagrams 6 and 7 are self-explanatory. The general problem is divided into two cases: Case I, symmetric reinforcement with no tension, Diagram-6; Case 2, symmetric reinforcement with tension, Diagrams 7 and 8 . In each case the thickness of insulation is taken as $d_{1}=D /$ ro, making $a=0.4 \mathrm{D}$. This will usually give good values, even for very large values of $D$. The reason for adopting this fixed relation is to simplify the formulae which could not otherwise be solved by curves owing to the extra variable $d_{1}$.

The curves cover any case of eccentrically applied loading, as for columns, piers or arch sections where $N=$ the resultant thrust normal to the section and $v=$ the eccentricity of this thrust.

Diagram 6 gives everything required for designing a section or for finding the stresses in a given design.

Diagram 7 gives all information for designing a section, while the additional diagram 8 is added to find the stresses in a given design.

## ALLOWABLE UNIT STRESSES AND BEAM FORMULAE.

TABLE II.
PHYSICAL CONSTANTS, CLASS A CONCRETE

|  | E |  | Ult. comp. | Working stresses, lbs. per sq. in. |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Concrete. | sq. in. ( 6 months). |  | $12^{\prime \prime}$ Cubes ( 6 months) | Compression. | Columns. | Tension. | Shear. | Bond. | $\mathrm{n}=\mathrm{E}_{\mathrm{s}} / \mathrm{E}_{\mathrm{c}}$ |
| I: $1.5: 3$ | $3,500,000$ | $0.000,0050$ | 3,400 | 650 | 550 | 0 | 45 | Smooth rod, 60 lb ., develop- | I 5 |
| I: $2: 4$ | $3,000,000$ | $0.000,0055$ | 3,200 | 600 | 500 | 0 | 40 | ed in 65 diam. | for average |
| 1:2.5:5 | 2,500,000 | $0.000,0060$ | 3,000 | 550 | 450 | 0 | 35 | bars, 150 lb , | working |
| $1: 3: 6$ | 2,000,000 | $0.000,0065$ | 2,800 | 500 | 400 | 0 | 30 | developed in 25 diam. |  |
| Steel | 30,000,000 | $0.000,0067$ | 65,000 | 15,000 | 10,000 | 15,000 | 10,000 | for $\cup$ bars. | conditions, |

Weight of plain concrete $=144 \mathrm{lbs}$. per $\mathrm{cu} . \mathrm{ft} . \quad \mathrm{I} \%$ reinforcement $=\mathrm{I}_{3} 2 \mathrm{lbs}$. per $\mathrm{cu} . \mathrm{yd} .=4.9 \mathrm{lbs}$. per $\mathrm{cu} . \mathrm{ft}$.

## Rectangular Beams in Simple Flexure, Neglecting Tension in the Concrete.



Moment of resistance of the concrete

$$
=M_{0}=\frac{1}{2} f_{0} k j b d^{2}=C j d=R_{0} b d^{2}
$$

Moment of resistance of the steel

$$
=M_{\mathrm{s}}=f_{\mathrm{s}} p j b d^{2}=T j d=R_{\mathrm{s}} b d^{2} .
$$

The smaller value of $M$ or $R$ governs the strength of the beam Steel ratio:
$p=\frac{A_{\mathrm{s}}}{b d} ; k=\sqrt{2 p n+(p n)^{2}}-p n ;$ and $j=1-\frac{k}{3}$.
Also, $A_{\mathrm{s}}=p b d ; f_{\mathrm{c}}=\frac{2 M}{k j b d^{2}}=\frac{2^{\prime} R_{\mathrm{c}}}{k j}$; and $f_{\mathrm{s}}=\frac{M}{p j b d^{2}}=\frac{M}{j d A_{\mathrm{s}}}=\frac{R_{\mathrm{s}}}{p j^{\prime}}$ where $M=$ moment of the external forces.

When $M_{\mathrm{o}}=M_{\mathrm{s}}$, then $R_{\mathrm{c}}=R_{\mathrm{s}}$ and $p=\frac{\mathrm{I}}{\frac{2 f_{\mathrm{s}}}{f_{\mathrm{c}}}\left(\frac{f_{\mathrm{s}}}{n f_{\mathrm{c}}}+\mathrm{I}\right)}$
Insulation $d_{1}=\frac{1}{2} \sqrt{D}$ about, and $D=d+d_{1} . \quad$ Min $d_{1}=\frac{9}{4}^{n}$
TABLE III.
WORKING VALUES FOR CLASS A CONCRETE.

| Mixture Concrete. | $\frac{f_{\mathrm{s}}}{f_{\mathrm{c}}}$ | $\begin{gathered} \quad \begin{array}{c} p \\ \text { for } n=15 \\ M_{\mathrm{s}}=M_{\mathrm{c}} \end{array} \end{gathered}$ | Steel per $\mathrm{cu} . \mathrm{ft}$. | $j$ | $k$ | $\begin{gathered} R_{\mathrm{c}}= \\ 1 / 8 f_{\mathrm{c}} k j \end{gathered}$ | $\begin{aligned} & R_{\mathrm{s}}= \\ & f_{\mathrm{s}} p j \end{aligned}$ | Moments of external loading. |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  | Nature of supports. | Uniform load. | Concentr load. |
| 1:1.5:3 | 23.1 | 0.0085 | 4.I7 lis. | 0.869 | 0.393 | 110.9 | I 10.9 |  |  | $M=P l / 4$ |
| 1:2:4 | 25 | 0.0075 | 3.68 lb . | 0.875 | 0.375 | 98.4 | $984$ | Simple beam, 2 supports | $M=p l^{2} / 8$ | $M=P / 1 / 5$ $M=P l / 5$ |
| 1:2.5:5 | $27 \cdot 3$ | 0.0065 | 3.19 lbs . | 0.882 | 0.355 | 85.0 74.5 | $\begin{aligned} & 86.0 \\ & 74.5 \end{aligned}$ | One end continuous Continuous beam | $\begin{aligned} & M=p l^{2} / 10 \\ & M=p l^{2} / 12 \end{aligned}$ | $M=P l / 6$ |
| $1: 3: 6$ | 30 | 0.0056 | 2.74 liss. | 0.888 | 0.335 | $74 \cdot 5$ | 745 |  |  |  |

